

STATE OF MAINE
DEPARTMENT OF TRANSPORTATION



CHESTERVILLE-FARMINGTON
FRANKLIN COUNTY
FARMINGTON FALLS BRIDGE
OVER
SANDY RIVER
ROUTE 41/156

FEDERAL AID PROJECT No. 2229600
PROJECT LENGTH 0.24 mi.
BRIDGE NO. 2273

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SPECIFICATIONS

Design: Load and Resistance Factor Design per AASHTO LRFD Bridge Design Specifications, Eighth Edition 2017.

DESIGN LOADING

Live Load HL - 93 Modified for Strength 1

TRAFFIC DATA

| | |
|--------------------------------|------|
| Current (2020) AADT | 4270 |
| Future (2040) AADT | 5120 |
| DHV - % of AADT | 12% |
| Design Hour Volume | 614 |
| % Heavy Trucks (AADT) | 9% |
| % Heavy Trucks (DHV) | 5% |
| Directional Distribution (DHV) | 60% |
| 18 kip Equivalent P 2.0 | 355 |
| 18 kip Equivalent P 2.5 | 338 |
| Design Speed (mph) | 30 |

HYDROLOGIC DATA

| | |
|----------------------------|-------------|
| Drainage Area | 425.2 sq mi |
| Design Discharge (Q50) | 34,250 cfs |
| Check Discharge (Q100) | 39,200 cfs |
| Headwater Elevation (Q50) | 340.61 ft |
| Headwater Elevation (Q100) | 341.99 ft |
| Discharge Velocity (Q50) | 6.14 fps |
| Discharge Velocity (Q100) | 6.61 fps |
| Headwater Elevation (Q1.1) | 325.77 ft |
| Discharge Velocity (Q1.1) | 3.32 fps |
| Headwater Elevation (Q25) | 337.78 ft |

MATERIALS

| | |
|--------------------------------------|-----------------------------------|
| Concrete: | |
| Curb, Sidewalk & Transition Barriers | Class "LP" |
| All Other | Class "A" |
| Reinforcing: | |
| Plain Reinforcing Steel | ASTM A 615/A 615M, Grade 60 |
| Stainless Reinforcing Steel | ASTM A 955, Grade 75 |
| Structural Steel: | |
| All Materials (except as noted) | ASTM A 709, Grade 50W (unpainted) |
| High Strength Bolts | ASTM F3125, Grade A325 |

BASIC DESIGN STRESSES

| | |
|------------------------------|-------------------|
| Class A Concrete | f 'c = 4,000 psi |
| Class LP Concrete | f 'c = 5,000 psi |
| Plain Reinforcing Steel | fy = 60,000 psi |
| Stainless Reinforcing Steel | fy = 75,000 psi |
| Structural Steel: | |
| ASTM A 709/A 709M, Grade 50W | F y = 50,000 psi |
| ASTM A 709/A 709M, Grade 36 | F y = 36,000 psi |
| ASTM A 325 | F μ = 120,000 psi |

UTILITIES

Central Maine Power Company
Consolidated Communications
Farmington Falls Water District

MAINTENANCE OF TRAFFIC

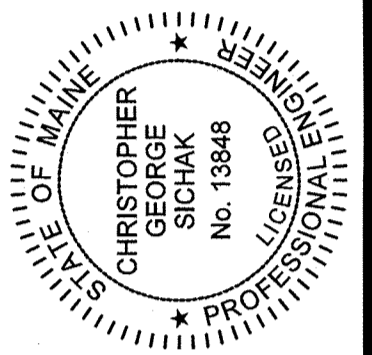
Maintain alternating one-way traffic using temporary one-lane bridge with temporary signals

| | |
|-------------------|---|
| PROJECT LOCATION: | Farmington Falls Bridge #2273 carries State Route 41 over Sandy River. Lat./Long. 44°-37'-12" N, 70°-04'-29" W |
| PROGRAM AREA: | Bridge Program |
| OUTLINE OF WORK: | Bridge Replacement |

ERDMAN
ANTHONY



| | | |
|--|--------------------|---------|
| STATE OF MAINE DEPARTMENT OF TRANSPORTATION | APPROVED | DATE |
| COMMISSIONER: <i>[Signature]</i> | <i>[Signature]</i> | 7-30-21 |
| CHIEF ENGINEER: <i>[Signature]</i> | <i>[Signature]</i> | 7-28-21 |



| | | | | | |
|--------------------|-----------|-------|-------------|------------|------|
| <i>[Signature]</i> | SIGNATURE | 13848 | P.E. NUMBER | 06/29/2021 | DATE |
|--------------------|-----------|-------|-------------|------------|------|

| | | | | | | | |
|---------------------|----------------|-----------------|--------------|----------------|------------------|------------|-------------------------|
| PROJECT INFORMATION | BRIDGE PROGRAM | PROJECT MANAGER | DESIGNER | CONSULTANT | PROJECT RESIDENT | CONTRACTOR | PROJECT COMPLETION DATE |
| | | MICHAEL WIGHT | CHRIS SICHAK | ERDMAN ANTHONY | | | |

| | |
|--|-------------|
| CHESTERVILLE-FARMINGTON FARMINGTON FALLS BRIDGE | TITLE SHEET |
|--|-------------|

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| SHEET NUMBER |
| 1 |
| OF 76 |

Date:6/29/2021

Username: Lindo T

Division: BRIDGE

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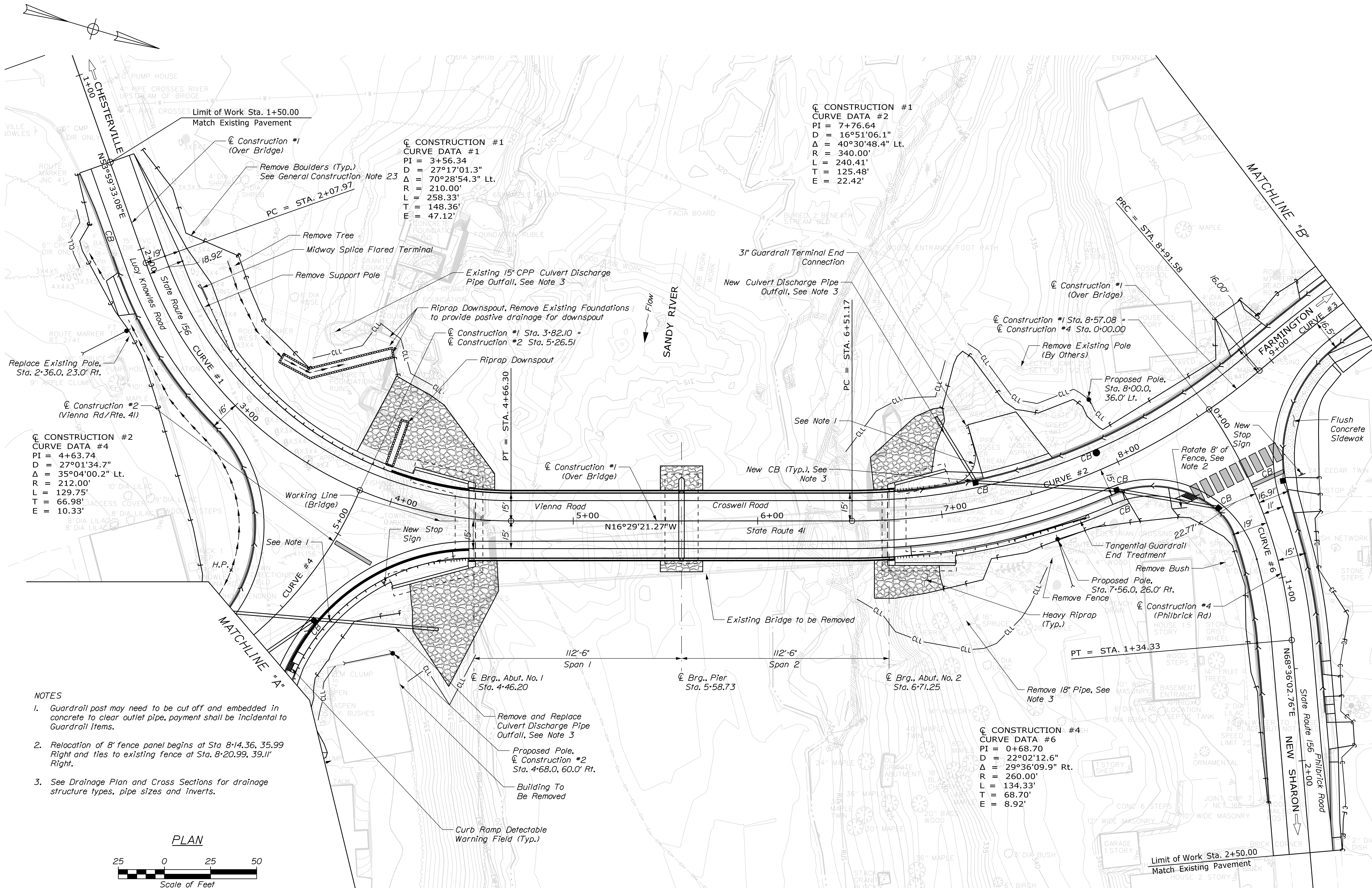
2229600 WIN 22296.00

Date: 6/29/2021

Username: LindoT

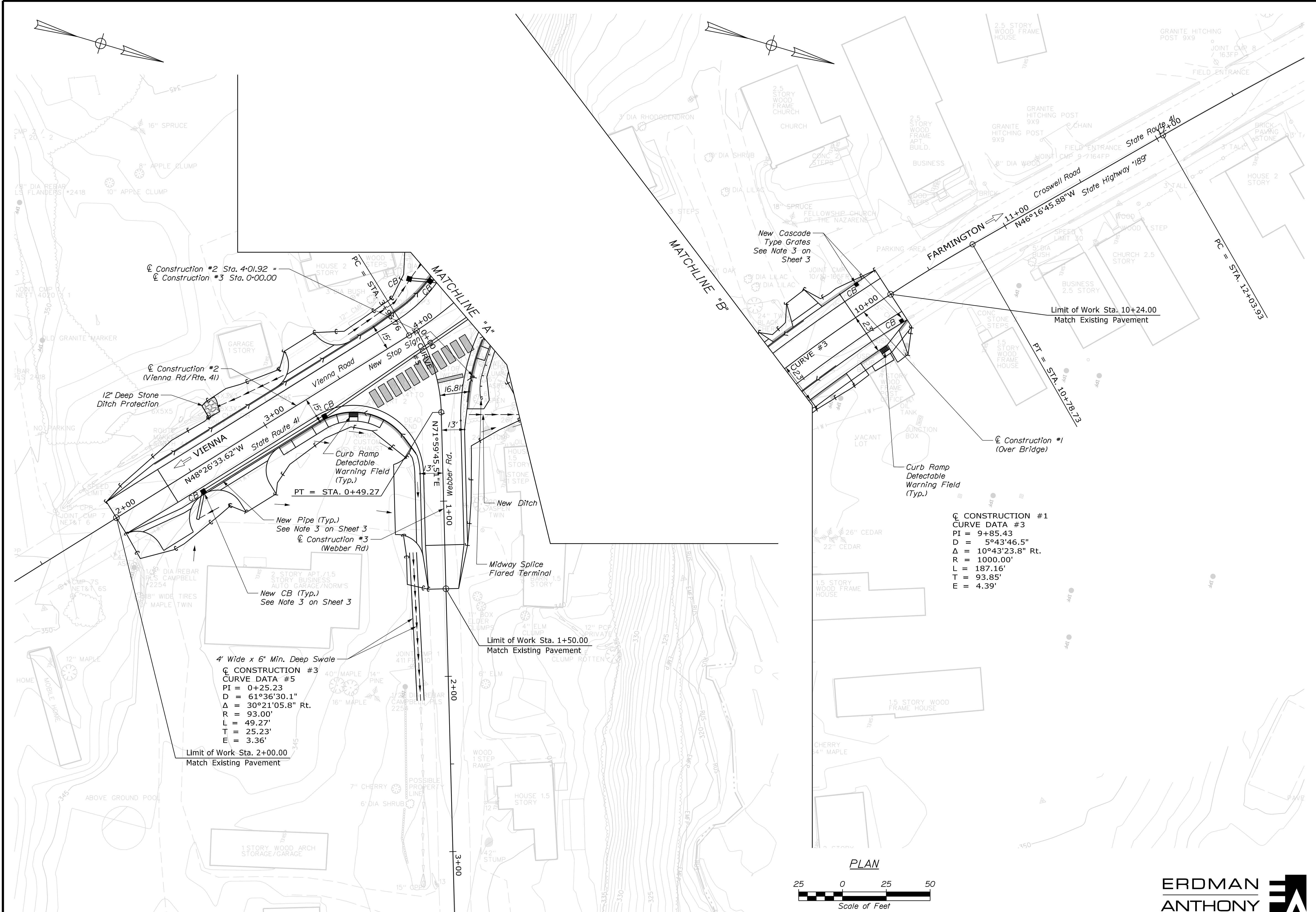
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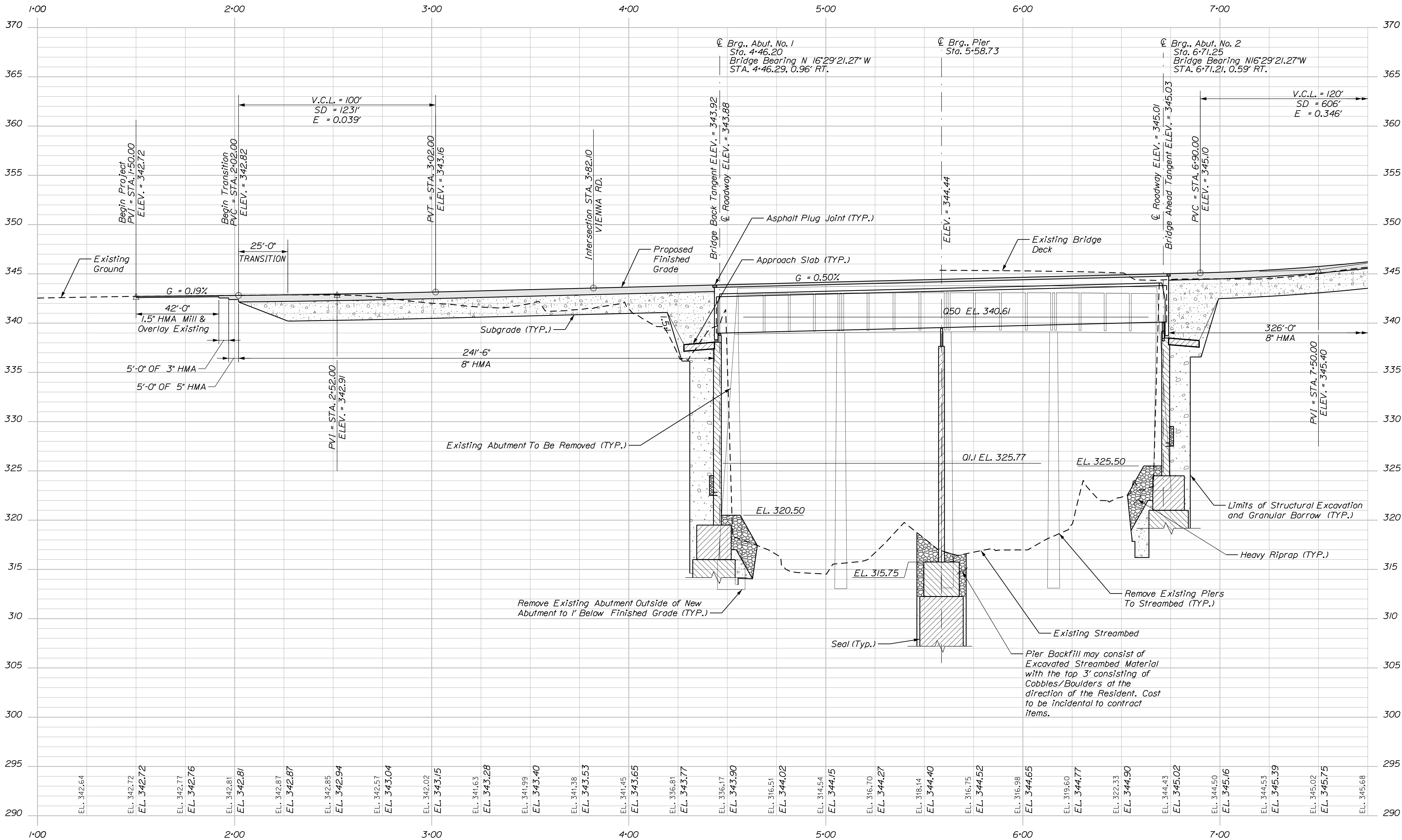


- NOTES
- Guardrail post may need to be cut off and embedded in concrete to clear outlet pipe, payment shall be incidental to Guardrail Items.
 - Relocation of 8' fence panel begins at Sta 8+14.36, 35.99 Right and ties to existing fence at Sta. 8+20.99, 39.11' Right.
 - See Drainage Plan and Cross Sections for drainage structure types, pipe sizes and inverts.

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| STATE OF MAINE DEPARTMENT OF TRANSPORTATION | | 2229600 | | BRIDGE NO. 2273 | | WIN 22296.00 | | BRIDGE PLANS | |
| FARMINGTON FALLS BRIDGE SANDY RIVER CHESTERVILLE-FARMINGTON FRANKLIN COUNTY | | GENERAL PLAN (1 of 2) | | SHEET NUMBER | | 3 | | OF 76 | |
| PROJ. MANAGER | MICHAEL WIGHT | BY | R. PARKER | DATE | 6/2021 | SIGNATURE | | P.E. NUMBER | |
| DESIGN-DETAILED | MYLENE NG | CHECKED-REVIEWED | C. SICHAK | DATE | 6/2021 | DESIGN-DETAILED | | REVISIONS 1 | |
| DESIGN-DETAILED | | DESIGN-DETAILED | | REVISIONS 2 | | DESIGN-DETAILED | | REVISIONS 3 | |
| DESIGN-DETAILED | | DESIGN-DETAILED | | REVISIONS 4 | | DESIGN-DETAILED | | FIELD CHANGES | |

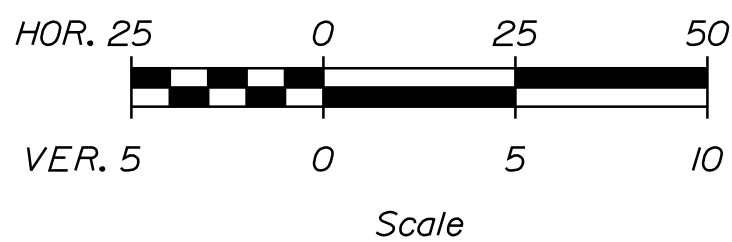


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| FARMINGTON FALLS BRIDGE SANDY RIVER CHESTERVILLE-FARMINGTON FRANKLIN COUNTY | | GENERAL PLAN (2 of 2) | | SHEET NUMBER | | 4 | | OF 76 | |
| PROJ. MANAGER | MICHAEL WRIGHT | BY | R. PARKER C. SICHAK | DATE | 6/2021 | SIGNATURE | P.E. NUMBER | DATE | |
| DESIGNED-DETAILED | MYLENE | CHECKED-REVIEWED | C. SICHAK | DESIGNS-DETAILED | | REVISIONS 1 | | | |
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| | | | | REVISIONS 4 | | FIELD CHANGES | | | |



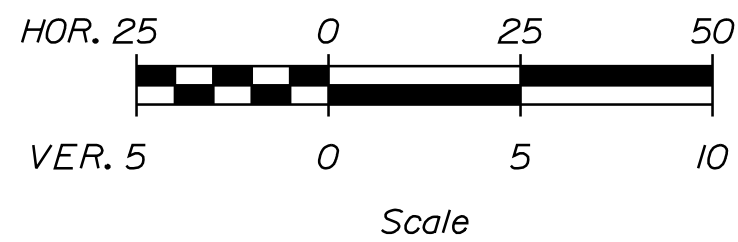
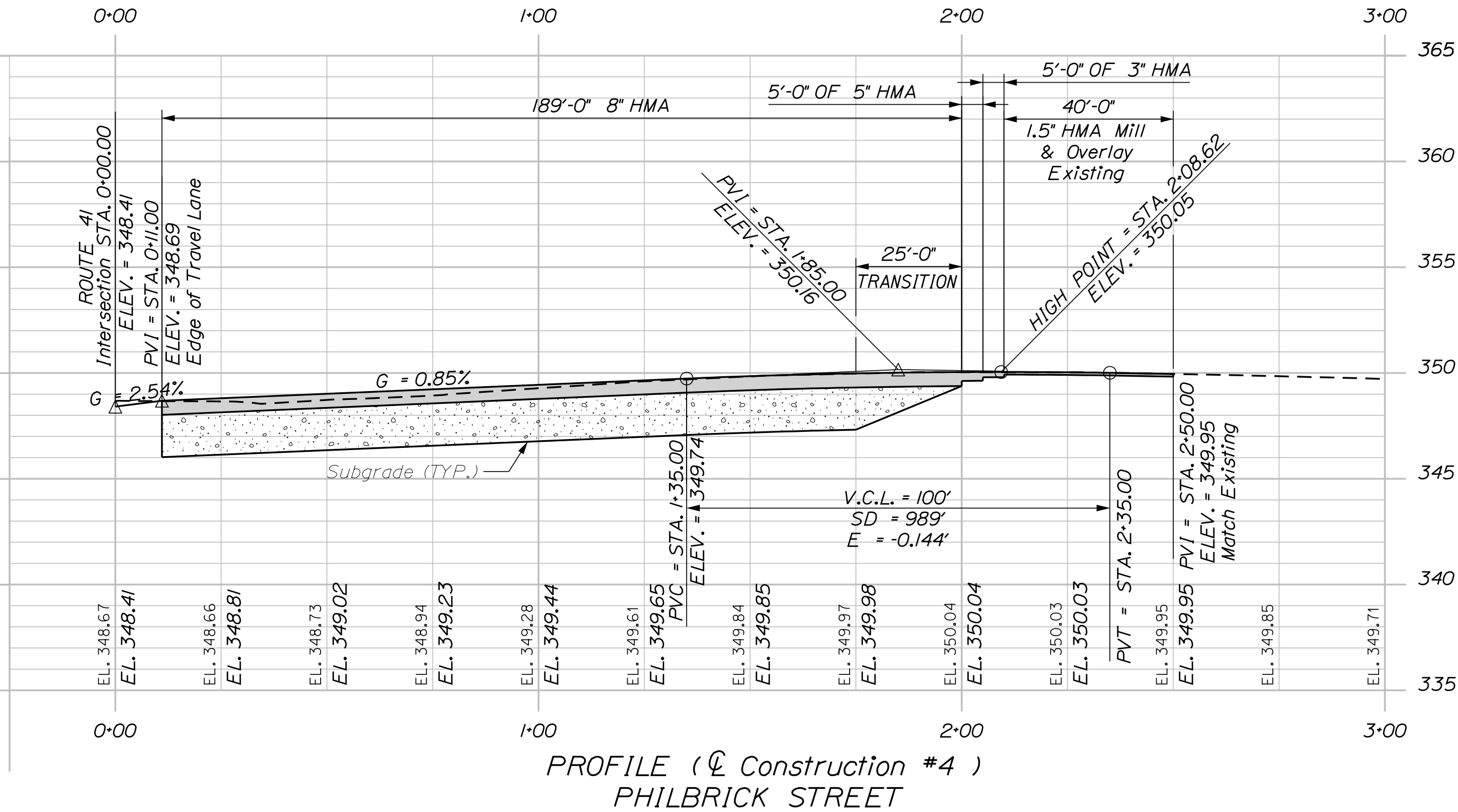
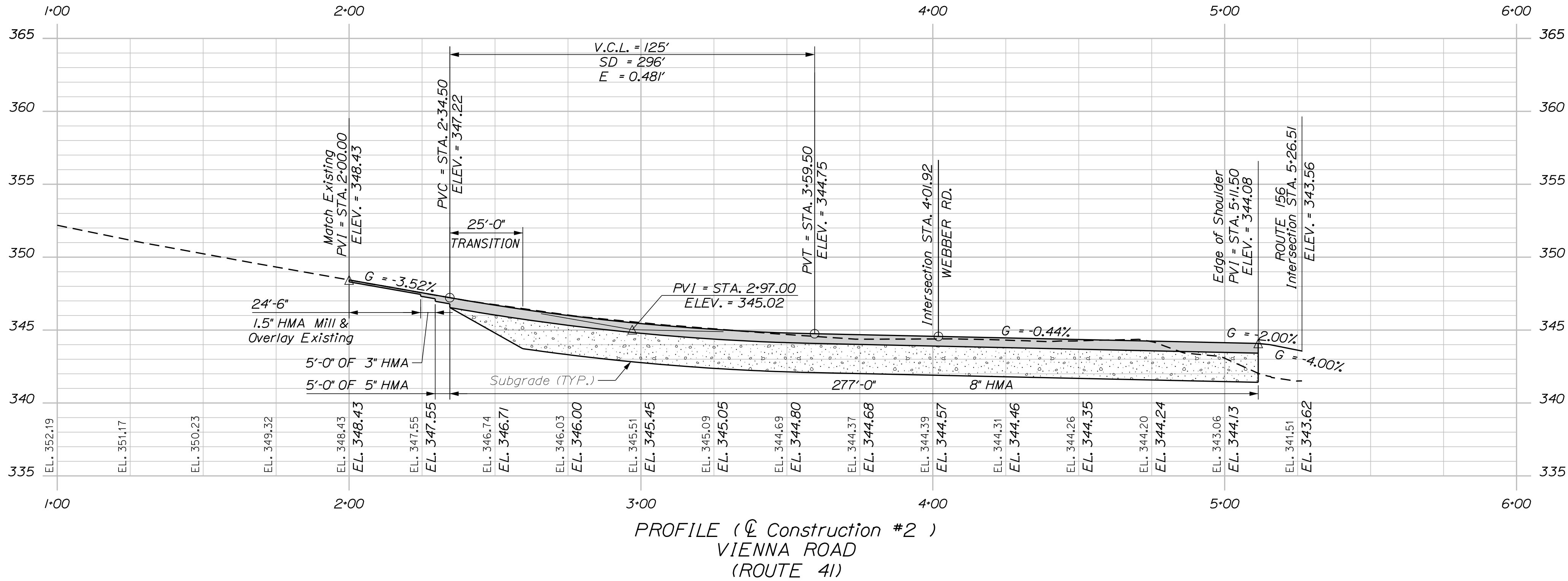
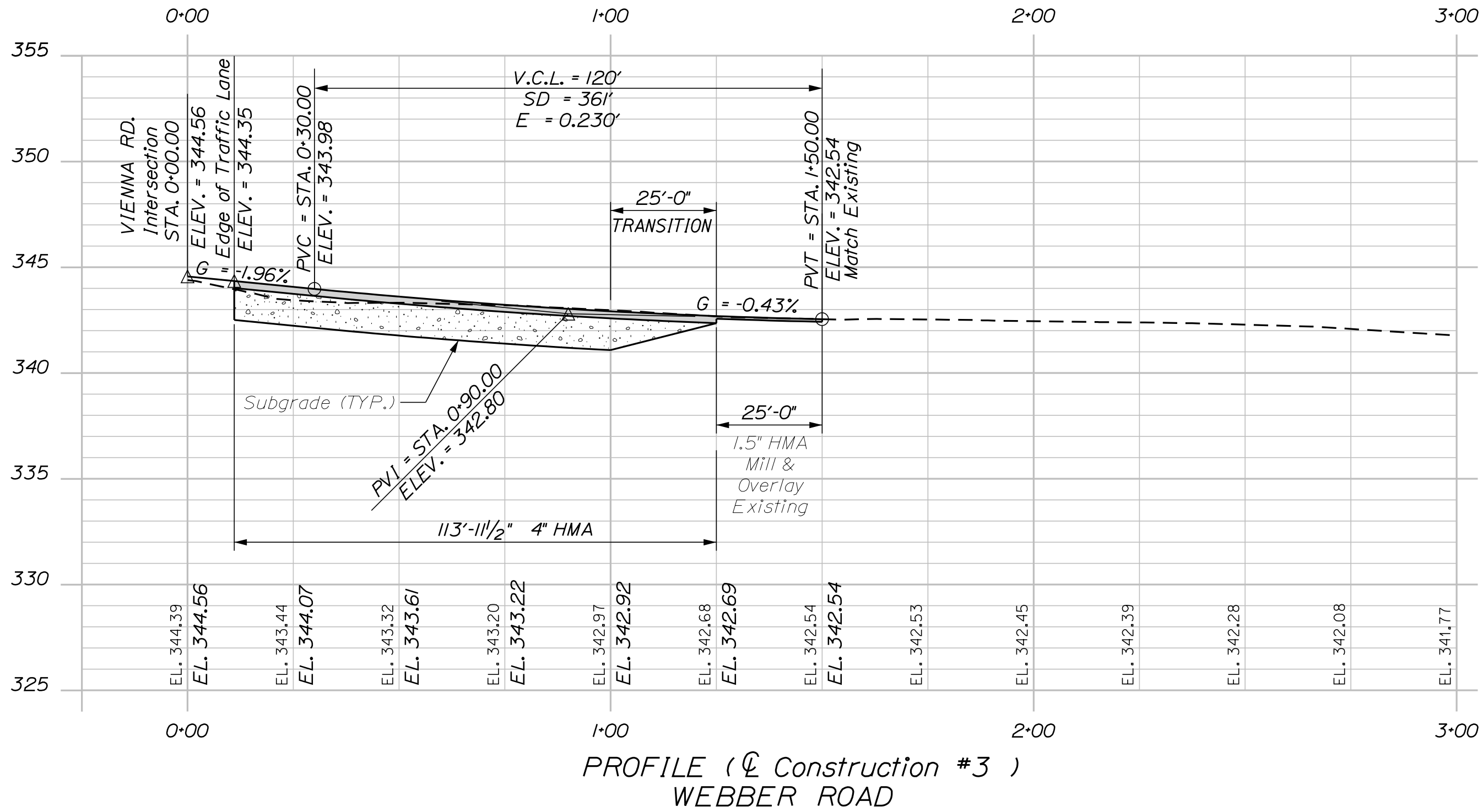
LUCY KNOWLES ROAD (ROUTE 156)

PROFILE (℄ Construction #1)
VIENNA ROAD (ROUTE 41)
STATE HIGHWAY 189



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| STATE OF MAINE DEPARTMENT OF TRANSPORTATION | 2229600 | | BRIDGE NO. 2273 | | WIN | | 22296.00 | | BRIDGE PLANS | | |
| | FARMINGTON FALLS BRIDGE SANDY RIVER CHESTERVILLE-FARMINGTON FRANKLIN COUNTY | | PROFILE (1 of 3) | | SHEET NUMBER | | 5 | | OF 76 | | |
| | DATE | | BY | | MICHAEL WIGHT | | CHECKED | | REVIEWED | | |
| 6/2021 | | R. PARKER | | C. SICHAK | | C. SICHAK | | C. SICHAK | | C. SICHAK | |
| SIGNATURE | | P.E. NUMBER | | DATE | | REVISIONS 1 | | REVISIONS 2 | | REVISIONS 3 | |
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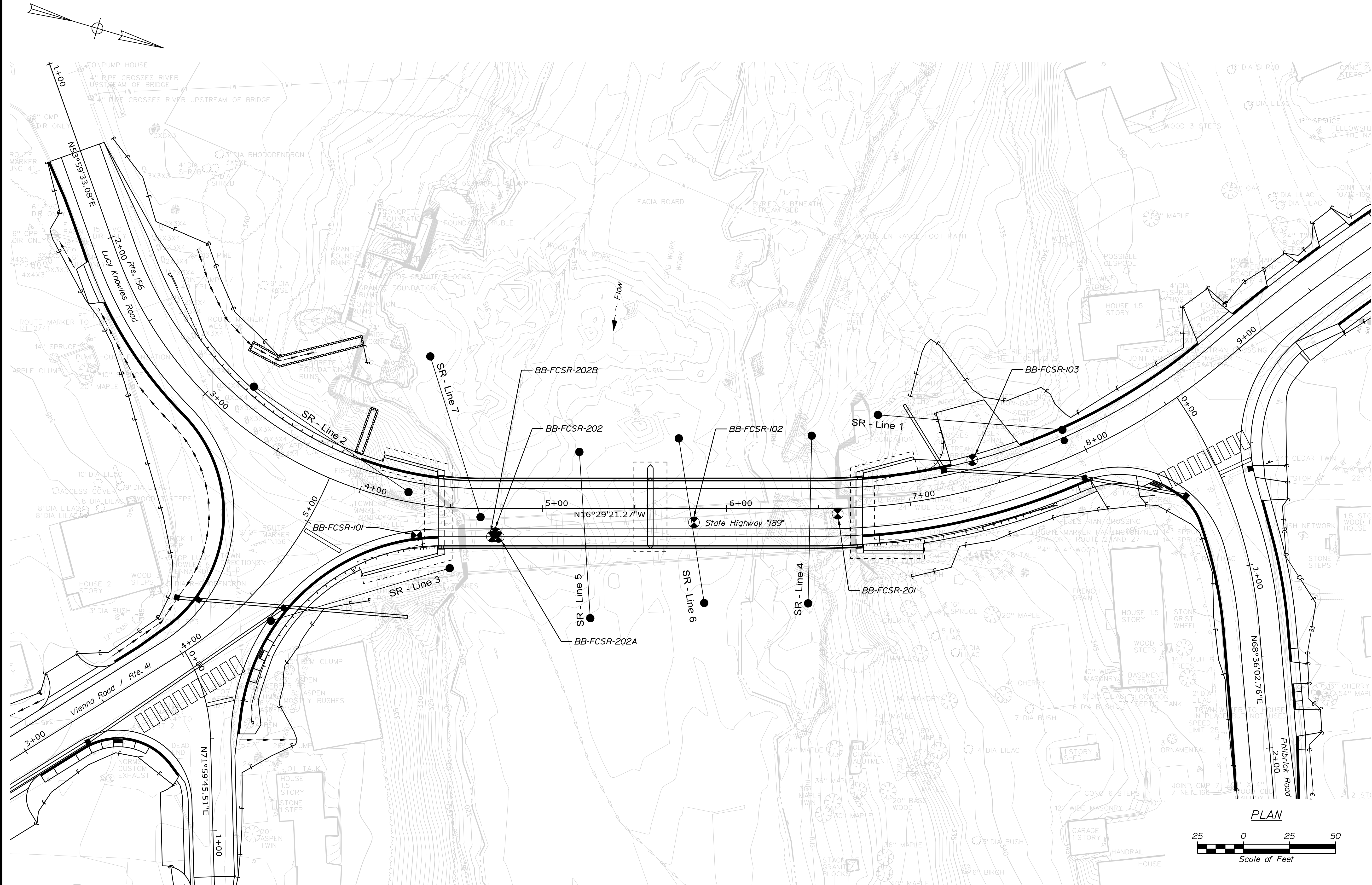
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| FARMINGTON FALLS BRIDGE | | SANDY RIVER | | CHESTERVILLE-FARMINGTON FRANKLIN COUNTY | | PROFILE | | (3 of 3) | |
| PROJECT MANAGER | | BY | | DATE | | SIGNATURE | | P.E. NUMBER | |
| DESIGN-DETAILED | | R. PARKER | | 6/2021 | | | | | |
| CHECKED-REVIEWED | | C. SICHAK | | 6/2021 | | | | | |
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| DESIGNS DETAILING | | | | | | | | | |
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| FIELD CHANGES | | | | | | | | | |
| SHEET NUMBER | | 7 | | OF 76 | | | | | |

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Division: BRIDGE

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LEGEND

- CASED WASH BORING
- 100' Seismic Refraction Line Location

NOTE

Full seismic refraction survey report is available in the Geotechnical Design Report.



STATE OF MAINE
DEPARTMENT OF TRANSPORTATION

2229600

WIN
22296.00

BRIDGE NO. 2273

BRIDGE PLANS

FARMINGTON FALLS BRIDGE
SANDY RIVER
CHESTERVILLE-FARMINGTON FRANKLIN COUNTY

PROJ. MANAGER
DESIGN-DETAILED
CHECKED-REVIEWED
DESIGNS-DETAILED
REVISIONS 1
REVISIONS 2
REVISIONS 3
REVISIONS 4
FIELD CHANGES

DATE
6/2021
6/2021

BY
R. PARKER
C. SICHAK

SIGNATURE

P.E. NUMBER

DATE

SHEET NUMBER

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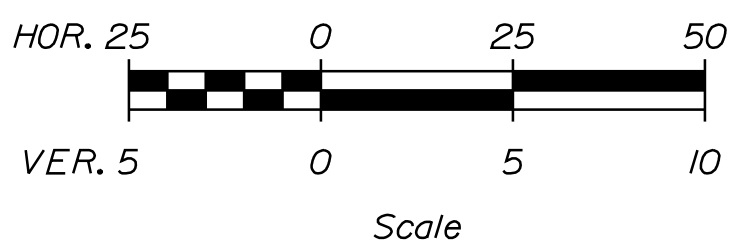
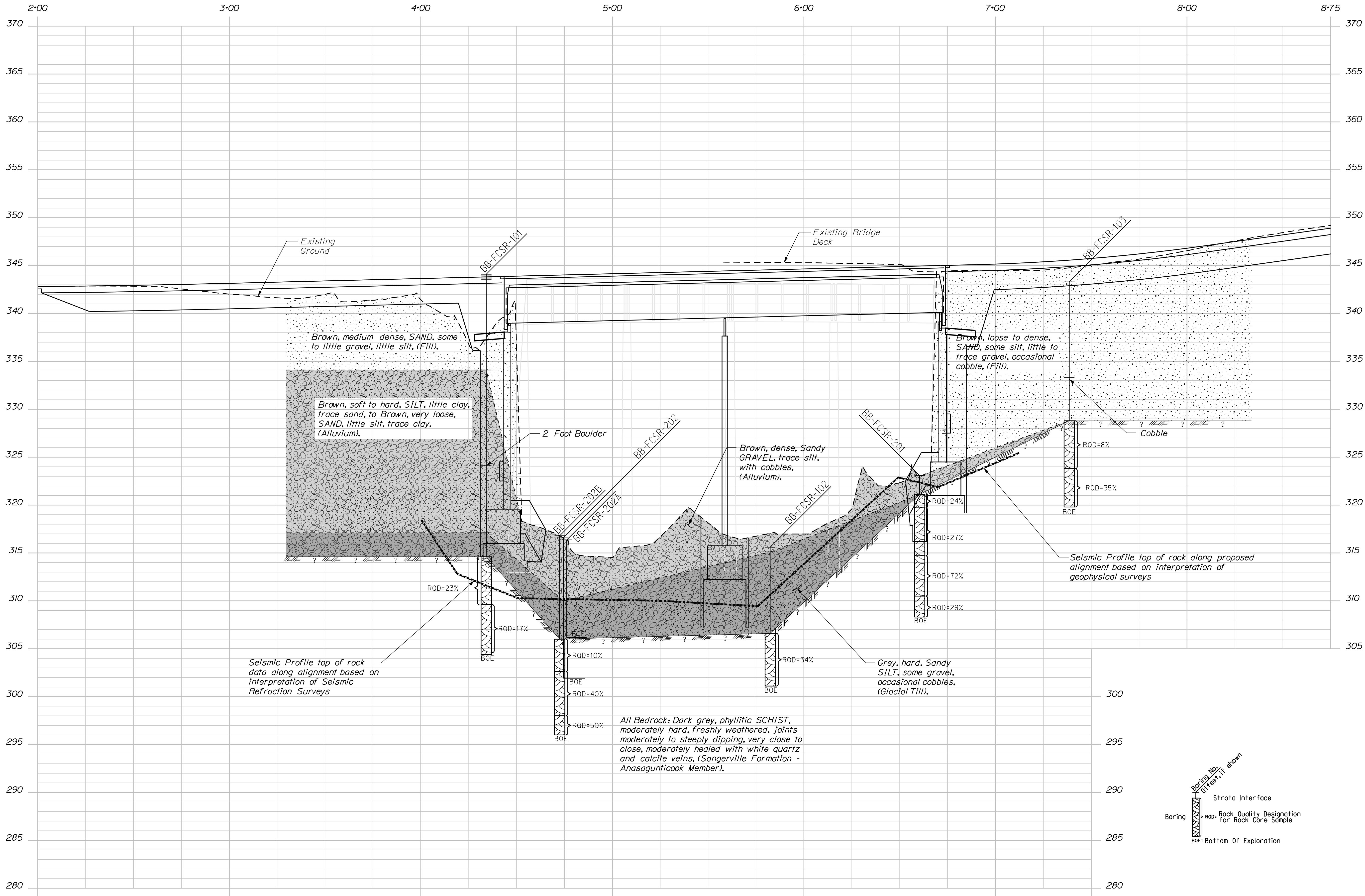
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Username: LindoT

Division: BRIDGE

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Note: This generalized interpretive soil profile is intended to convey trends in subsurface conditions. The boundaries between strata are approximate and idealized, and have been developed by interpretations of widely spaced explorations and samples. Actual soil transitions may vary and are probably more erratic. For more specific information refer to the exploration logs.

PROFILE (@ Construction #1)
VIENNA ROAD (ROUTE 41)
STATE HIGHWAY 189



STATE OF MAINE
DEPARTMENT OF TRANSPORTATION
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WIN
22296.00
BRIDGE NO. 2273
BRIDGE PLANS

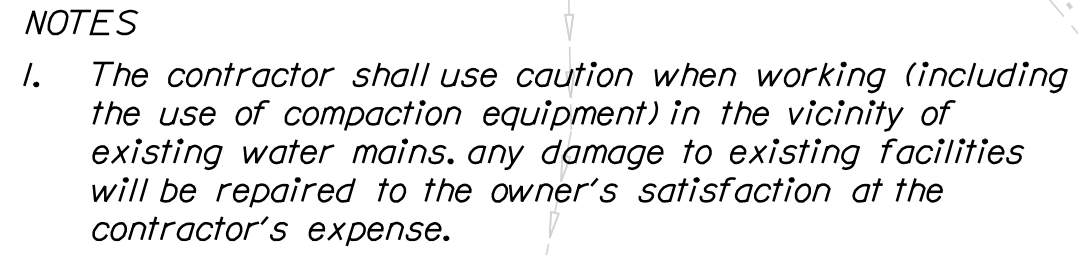
| PROJ. MANAGER | MICHAEL WRIGHT | BY | R. PARKER | DATE | 6/2021 |
|-------------------|----------------|------------------|-----------|-------------|--------|
| CHECKED/REVIEWED | C. SICHAK | DESIGNED/DETAILS | C. SICHAK | SIGNATURE | |
| DESIGNS DETAILING | | REVISIONS 1 | | P.E. NUMBER | |
| REVISIONS 2 | | REVISIONS 3 | | DATE | |
| REVISIONS 4 | | FIELD CHANGES | | | |

FARMINGTON FALLS BRIDGE
SANDY RIVER
CHESTERVILLE-FARMINGTON FRANKLIN COUNTY

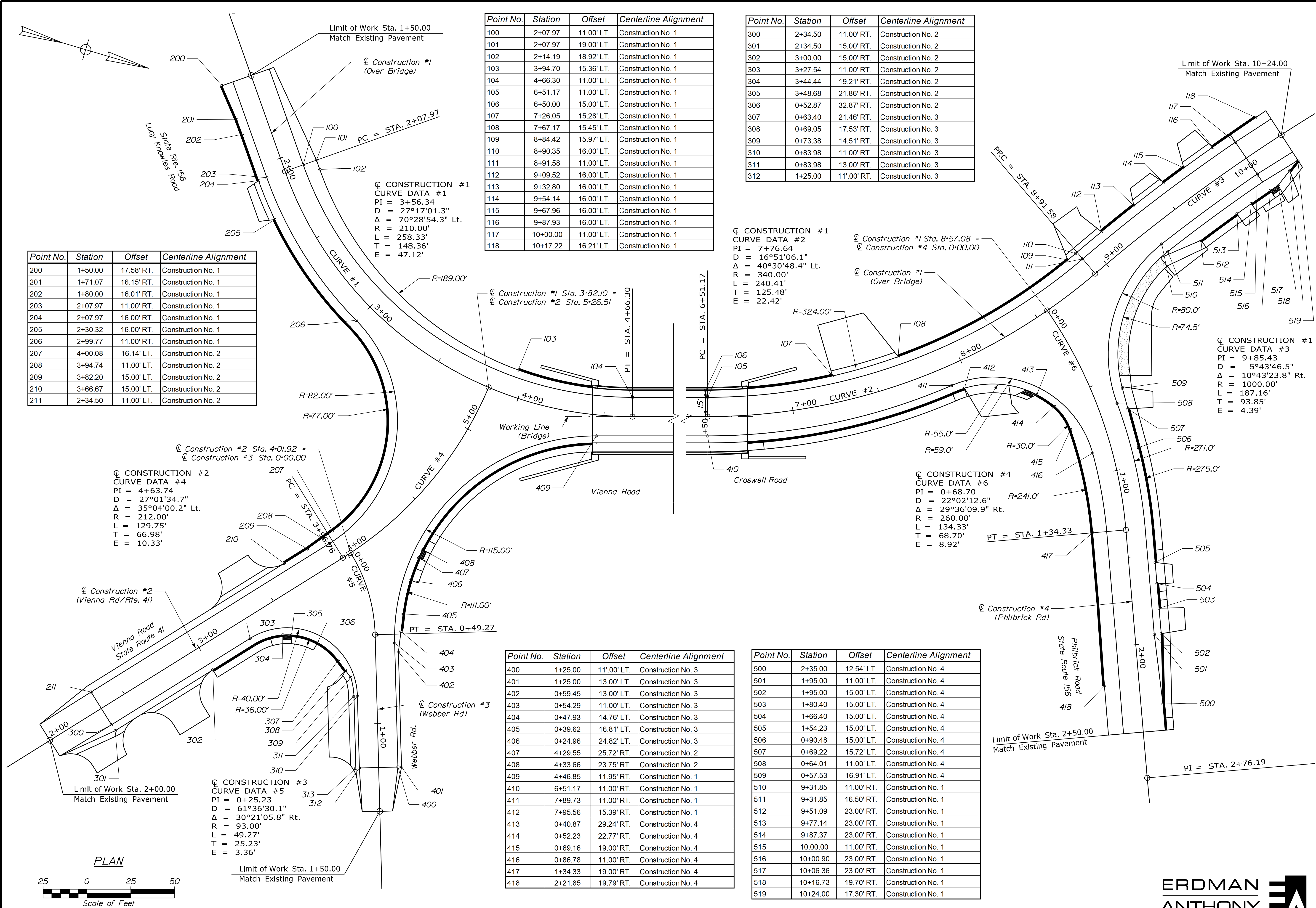
INTERPRETIVE SUBSURFACE
PROFILE

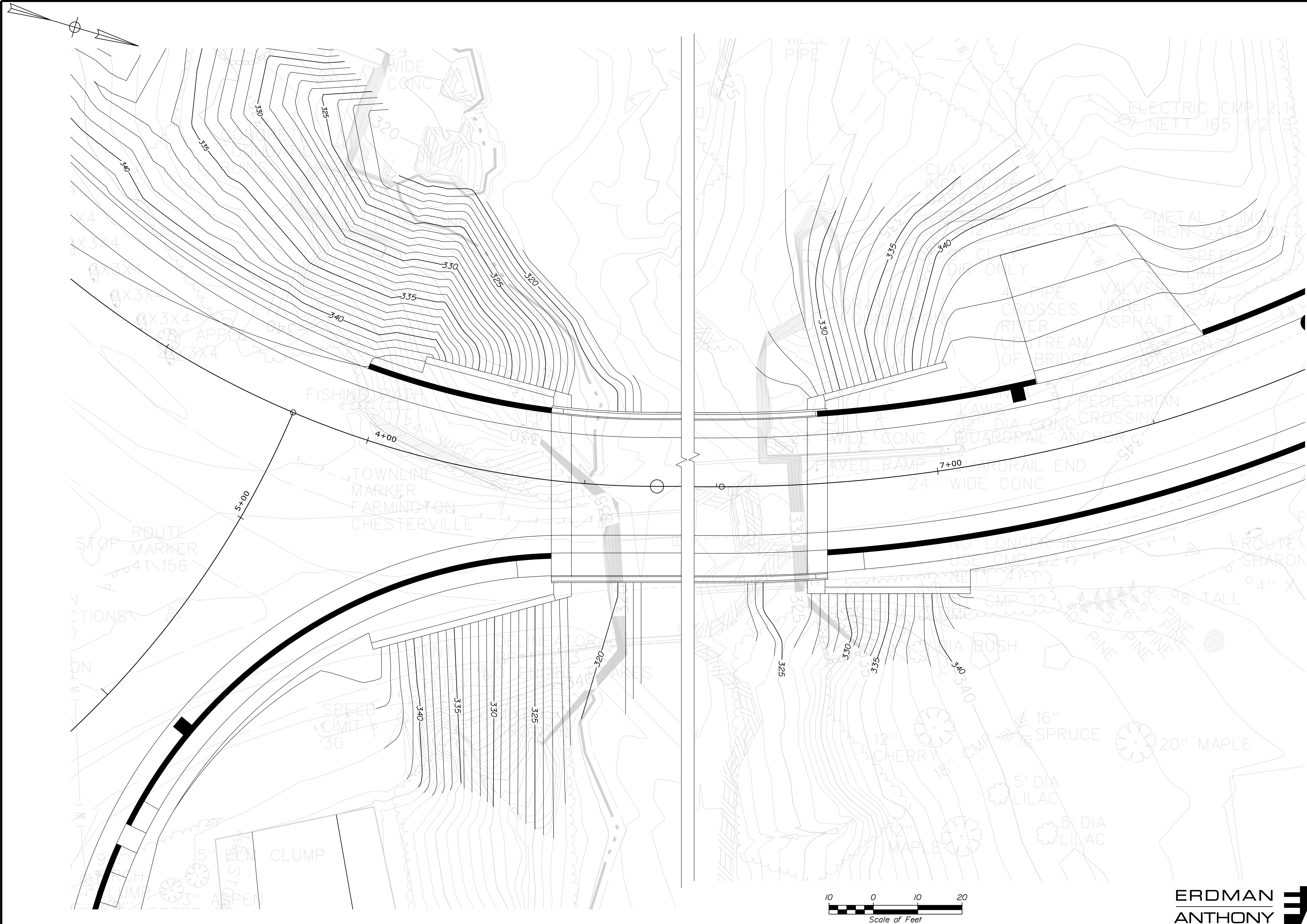
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| <div style="text-align: center;"> <p>FARMINGTON FALLS BRIDGE</p> <p>SANDY RIVER</p> <p>CHESTERVILLE-FARMINGTON FRANKLIN COUNTY</p> <p>DRAINAGE PLAN</p> </div> | PROJ. MANAGER MICHAEL WIGHT | | BY | DATE | STATE OF MAINE DEPARTMENT OF TRANSPORTATION 2229600 |
| | DESIGN-DETAILED | | R. PARKER | 6/2021 | |
| | CHECKED-REVIEWED | | C. SICHAK | 6/2021 | |
| | SIGNATURE | | | | |
| | DESIGN-DETAILED2 | | | | |
| | DESIGNS-DETAILED3 | | | | |
| | REVISONS 1 | | | P.E. NUMBER | WIN 22296.00 |
| | REVISONS 2 | | | | |
| | REVISONS 3 | | | | |
| | REVISONS 4 | | | | |
| | FIELD CHANGES | | | DATE | |
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| OF 76 | | | | | |





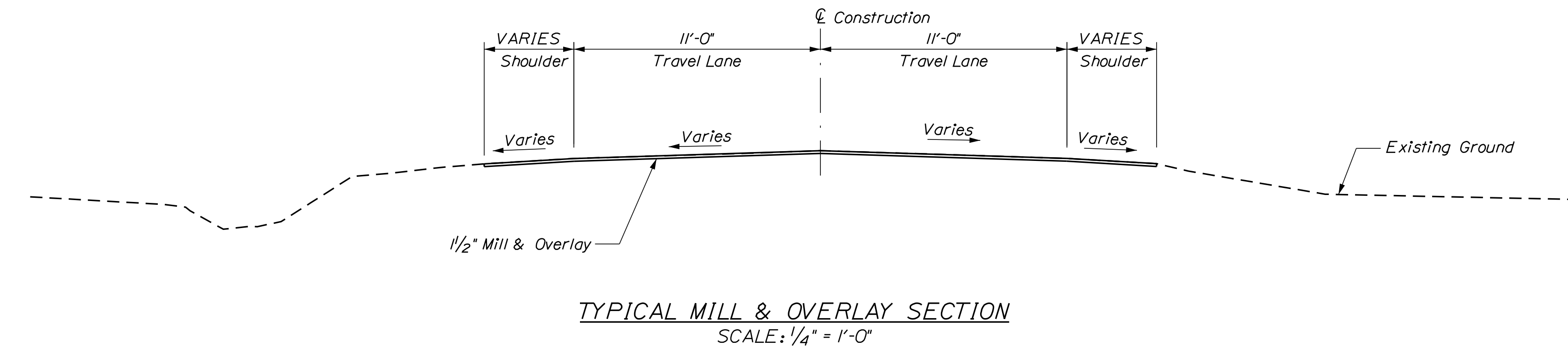
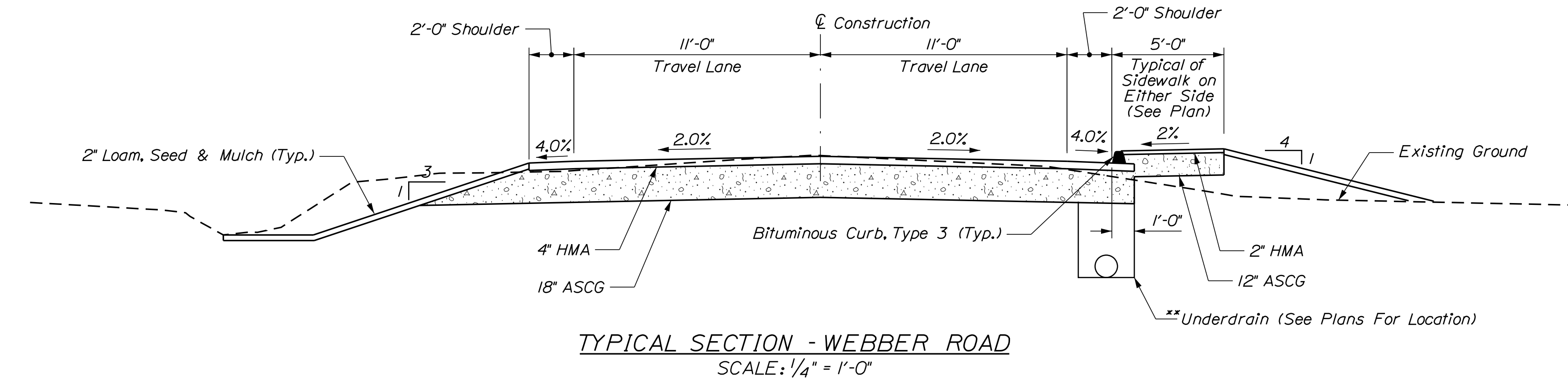
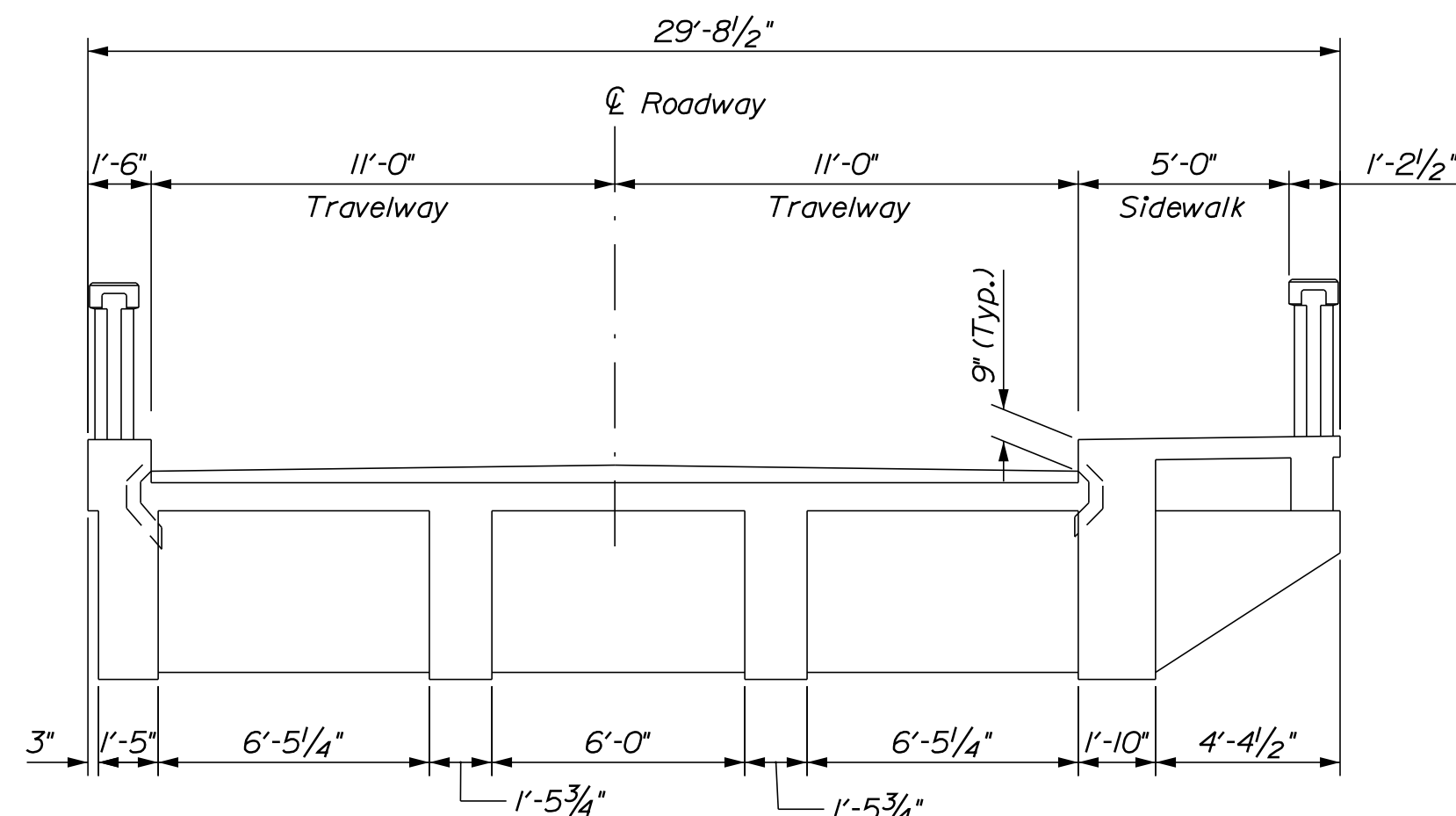
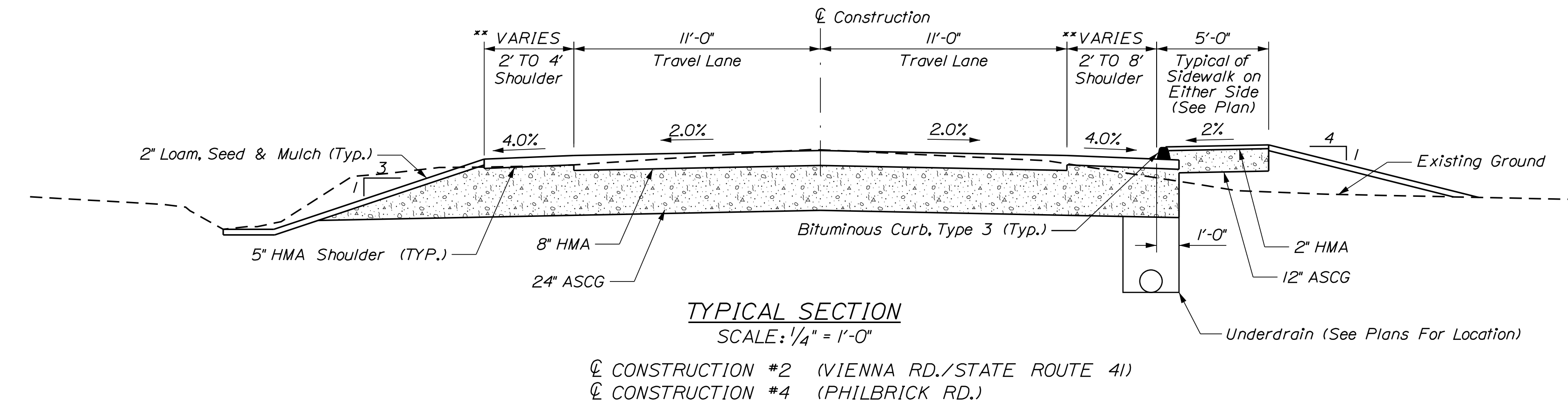
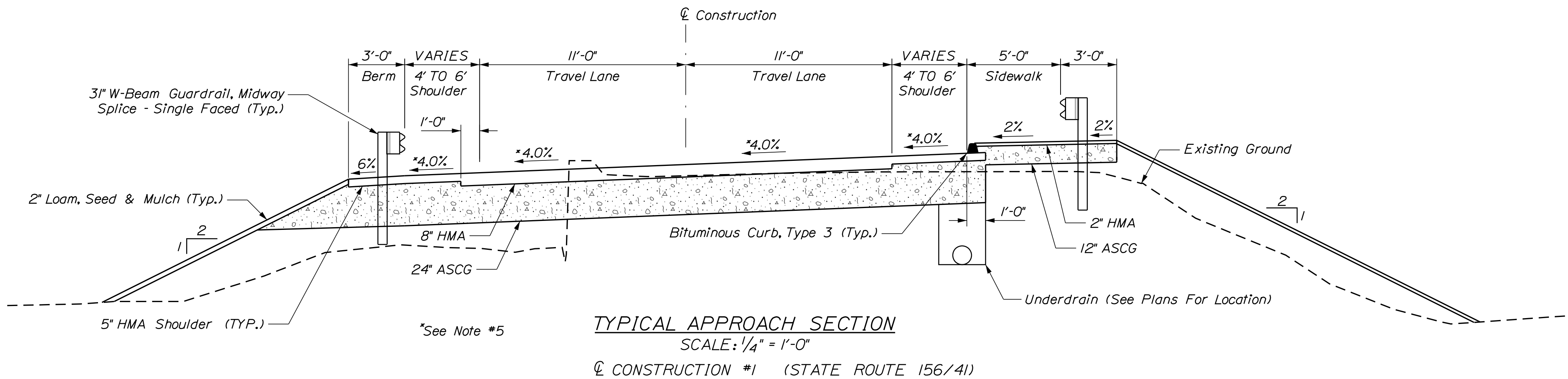
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| | | | | | | DATE | | | |
| 2229600 | | BRIDGE NO. 2273 | | WIN | | 22296.00 | | BRIDGE PLANS | |
| FARMINGTON FALLS BRIDGE SANDY RIVER CHESTERVILLE-FARMINGTON FRANKLIN COUNTY | | GRADING PLAN | | SHEET NUMBER | | 14 | | OF 76 | |

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Division: BRIDGE

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NOTES:

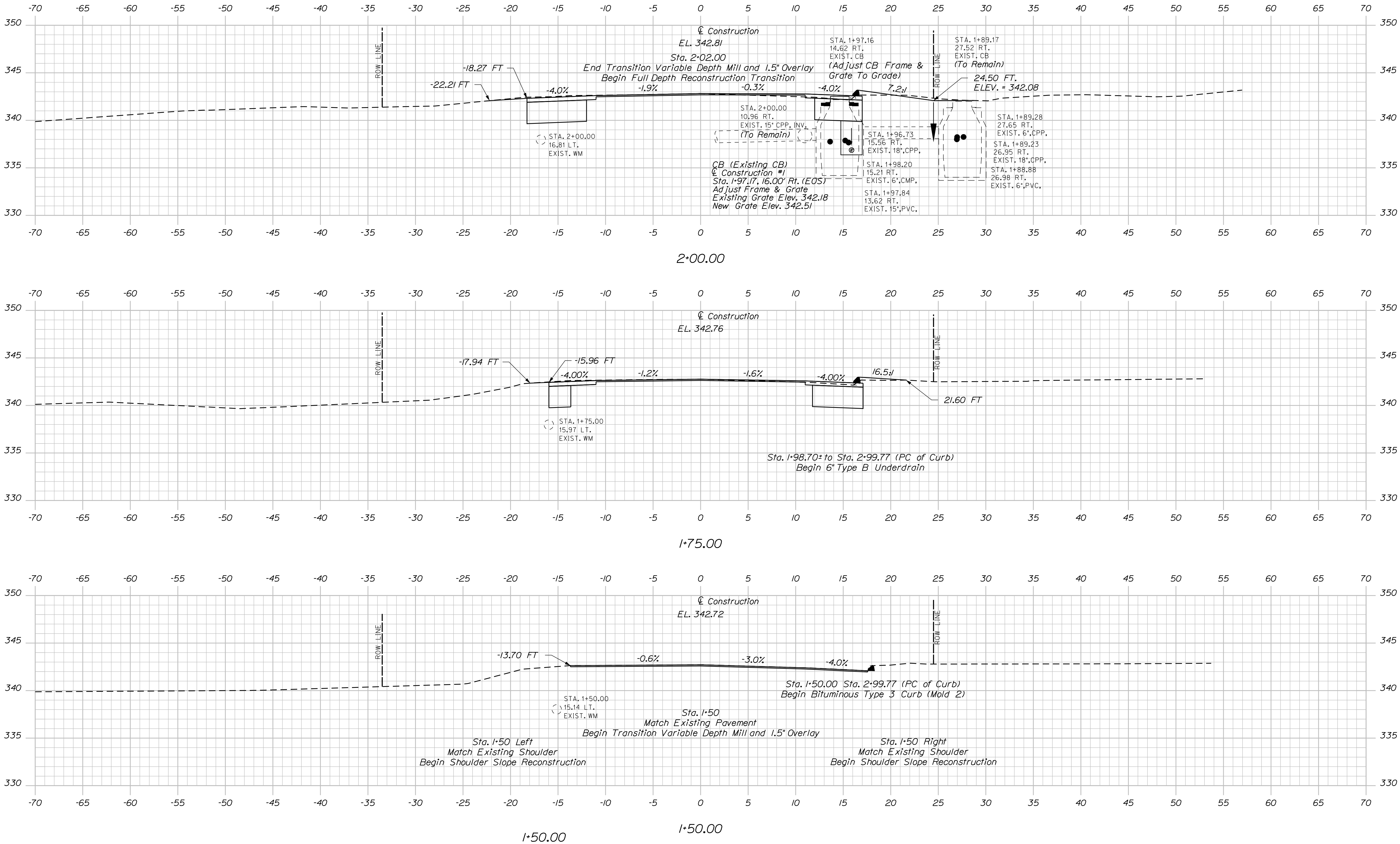
1. The pavement, base and subbase depths as shown on the plans are intended to be nominal.
2. When superelevation exceeds the slope of the low side shoulder, the low side shoulder shall have the same slope as the travelway.
3. Crowns for both normal and superelevation sections for all courses of subbase and pavement shall be straight.
4. The gravel quantity calculation is based on a 2" loam or dirty borrow depth. The actual depth may vary. See General Notes.
5. The algebraic difference between shoulder and travel lane cross slopes "rollover" shall not exceed 8%.
6. Alignment #1 has a 4% Superelevated road and bridge that transitions to a 2% normal crown on both ends.
7. The Bridge Working Line is established by extending Centerline of Construction #1 back tangent and ahead Tangent at a bearing of N 16° 29' 21.27" W.

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Division: BRIDGE

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Construction #1 (Over Bridge)



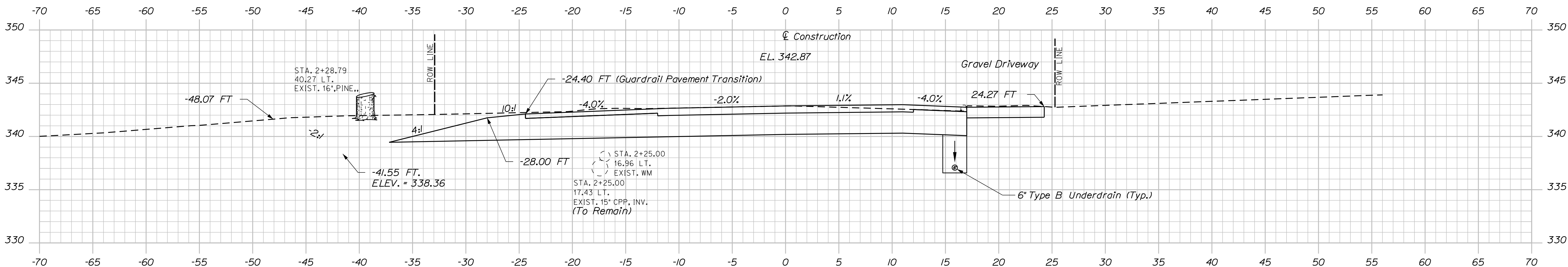
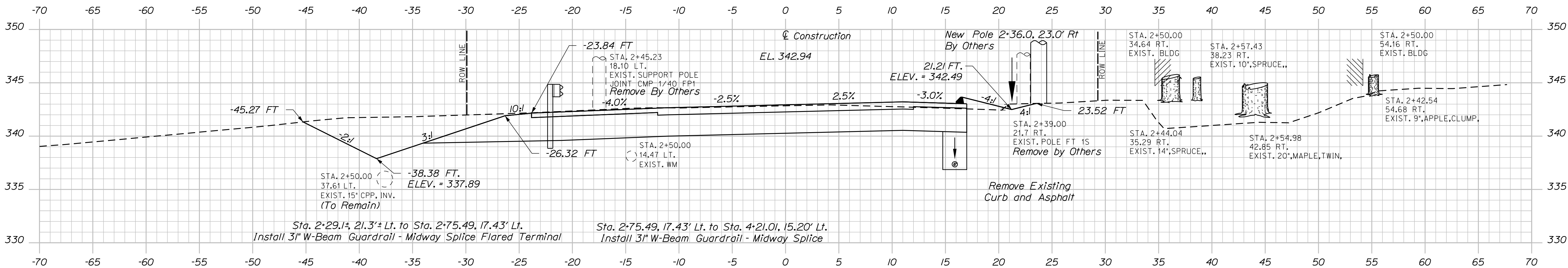
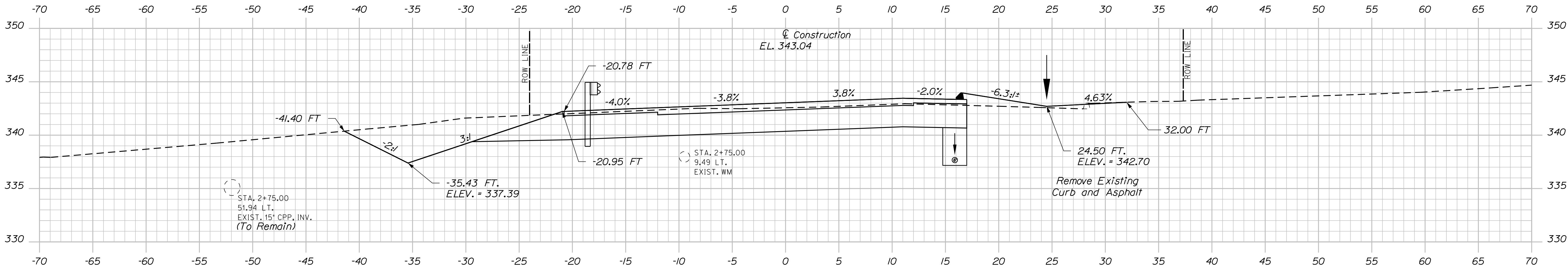
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| STATE OF MAINE DEPARTMENT OF TRANSPORTATION | | 2229600 | | WIN 22296.00 | |
| FARMINGTON FALLS BRIDGE SANDY RIVER CHESTERVILLE-FARMINGTON FRANKLIN COUNTY | | CROSS SECTIONS | | 2+00.00 | |
| SHEET NUMBER | | 16 | | OF 76 | |
| DATE | | 6/2021 | | SIGNATURE | |
| BY | | R. PARKER | | P.E. NUMBER | |
| MICHAEL WRIGHT | | C. SICHAK | | DATE | |
| DESIGNED-Detailed | | MYLENE | | REVISIONS 1 | |
| CHECKED-Reviewed | | C. SICHAK | | REVISIONS 2 | |
| DESIGNED-Detailed | | C. SICHAK | | REVISIONS 3 | |
| CHECKED-Reviewed | | C. SICHAK | | REVISIONS 4 | |
| DESIGNED-Detailed | | C. SICHAK | | FIELD CHANGES | |

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Division: BRIDGE

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Construction #1 (Over Bridge)

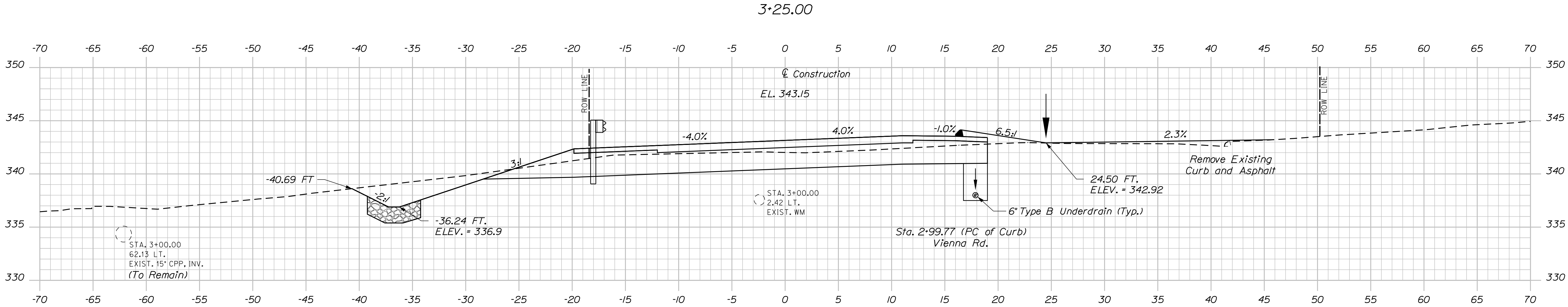
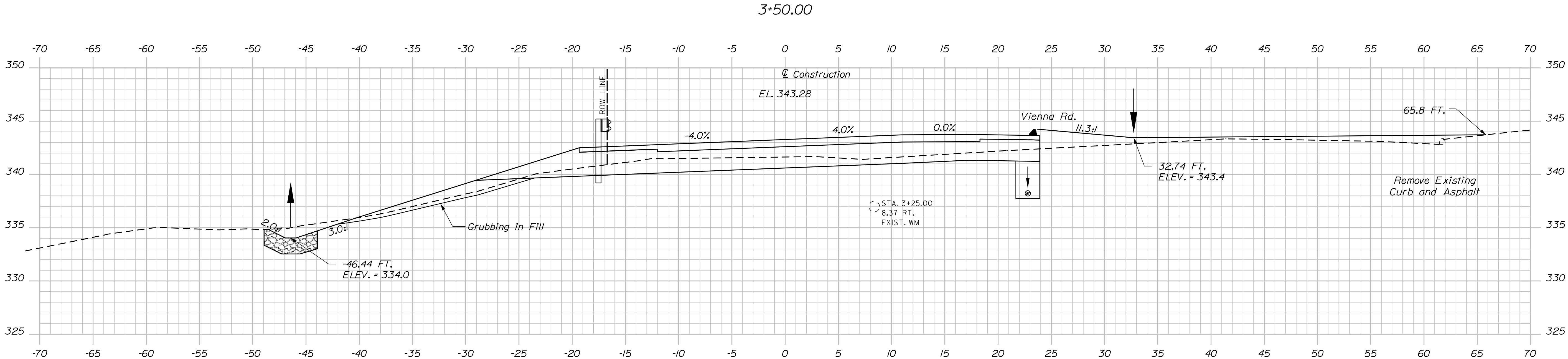
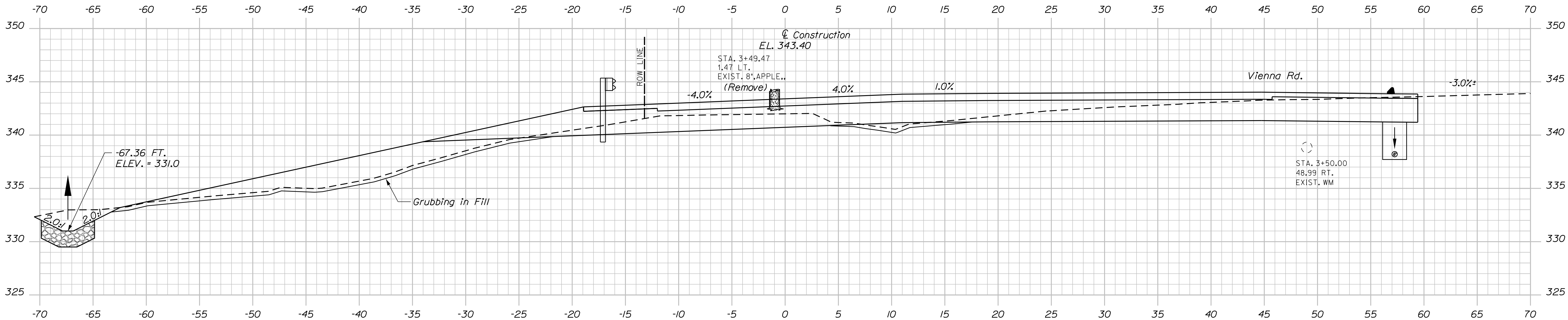
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| STATE OF MAINE | | DEPARTMENT OF TRANSPORTATION | | 2229600 | | WIN | | 22296.00 | | BRIDGE NO. 2273 | | BRIDGE PLANS | |
| FARMINGTON FALLS BRIDGE | | SANDY RIVER | | CHESTERVILLE-FARMINGTON FRANKLIN COUNTY | | 2+25.00 | | CROSS SECTIONS | | 2+75.00 | | SHEET NUMBER | |
| PROJ. MANAGER | | BY | | DATE | | SIGNATURE | | P.E. NUMBER | | DATE | | FIELD CHANGES | |
| DESIGN-DETAILED | | CHECKED-REVIEWED | | DESIGN-DETAILED | | DESIGN-DETAILED | | REVISIONS 1 | | REVISIONS 2 | | REVISIONS 3 | |
| MICHAEL WIGHT | | R. PARKER | | 6/2021 | | C. SICHAK | | C. SICHAK | | 6/2021 | | C. SICHAK | |
| MICHAEL WIGHT | | R. PARKER | | 6/2021 | | C. SICHAK | | C. SICHAK | | 6/2021 | | C. SICHAK | |
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| MICHAEL WIGHT | | R. PARKER | | 6/2021 | | C. SICHAK | | C. SICHAK | | 6/2021 | | C. SICHAK | |

Date: 6/29/2021

Username: LindoT

Division: BRIDGE

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Construction #1 (Over Bridge)



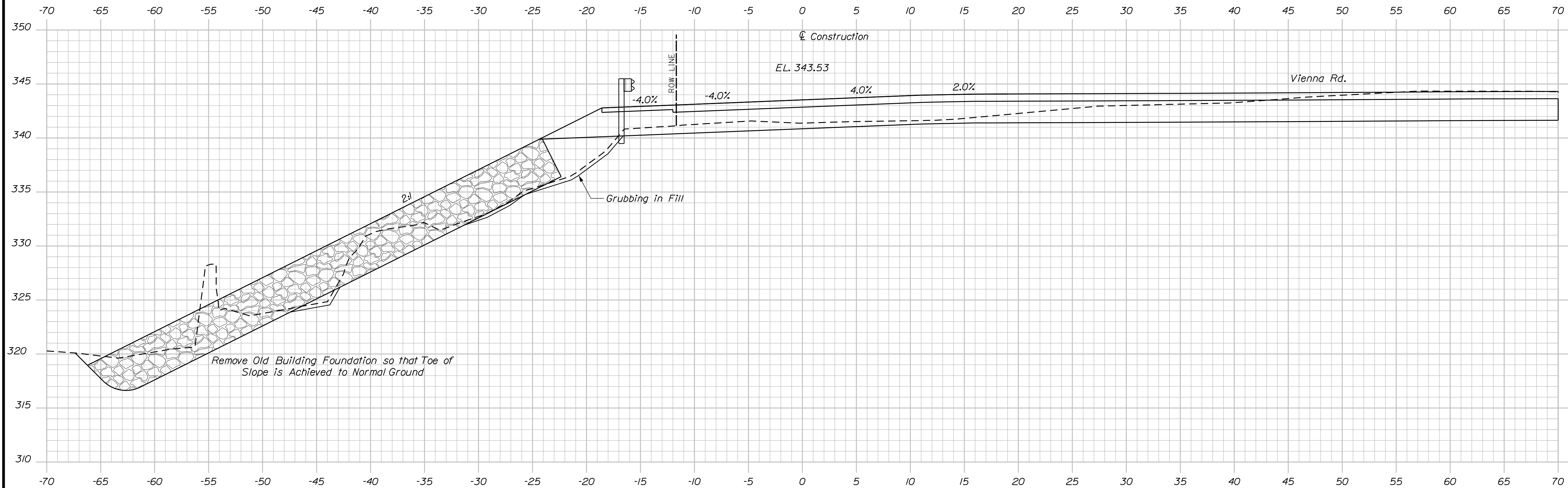
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| DEPARTMENT OF TRANSPORTATION |
| 2229600 |
| WIN 22296.00 |
| BRIDGE NO. 2273 |
| BRIDGE PLANS |


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| PROJ. MANAGER | CHECKED-DETAILED | DATE | BY | DATE |
| MICHAEL WIGHT | MYLENE R. PARKER | 6/2021 | R. PARKER | 6/2021 |
| CHECKED-REVIEWED | C. SICHAK | 6/2021 | C. SICHAK | 6/2021 |
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| DESIGN-REVIEWED | | | | |
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| REVISIONS 4 | | | | |
| FIELD CHANGES | | | | |

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|---|
| FARMINGTON FALLS BRIDGE |
| SANDY RIVER |
| CHESTERVILLE-FARMINGTON FRANKLIN COUNTY |
| 3+00.00 CROSS SECTIONS 3+50.00 |

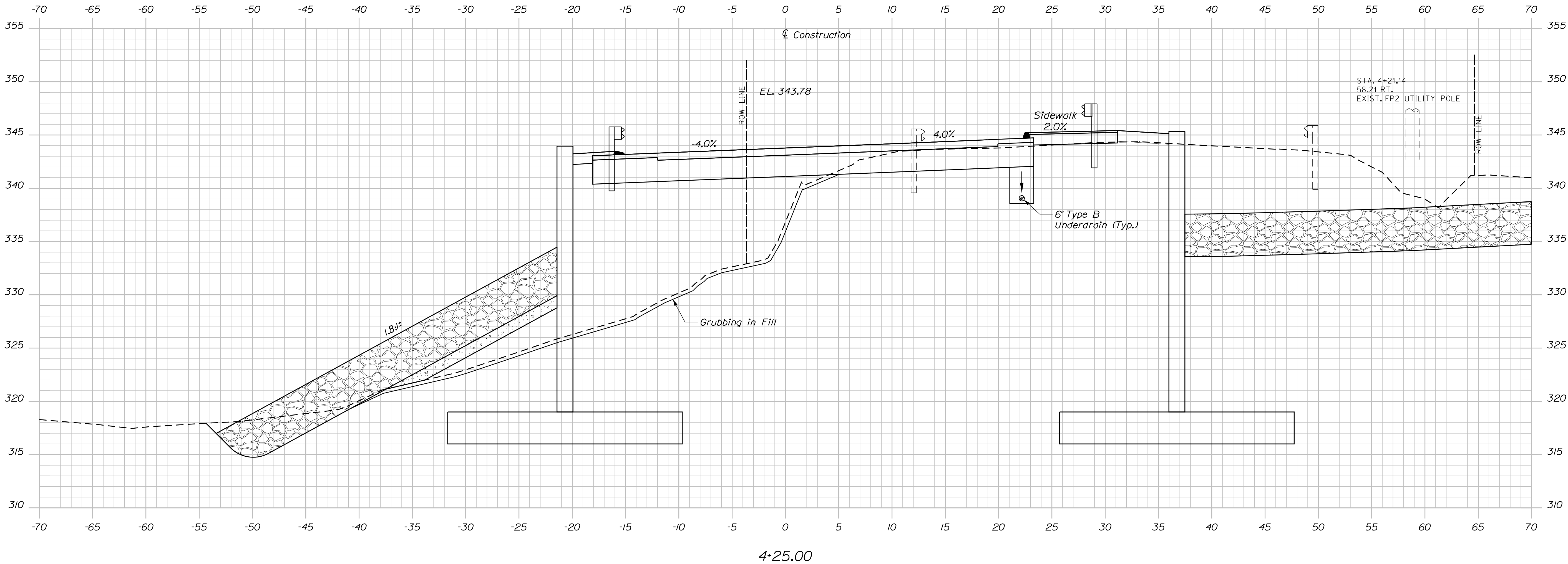
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| SHEET NUMBER |
| 18 |
| OF 76 |

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375.00 

[illegible]



CL Construction #1 (Over Bridge)



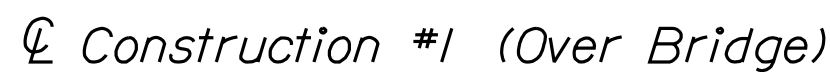
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| STATE OF MAINE | |
| DEPARTMENT OF TRANSPORTATION | |
| 2229600 | |
| BRIDGE NO. 2273 | WIN 22296.00 |
| BRIDGE PLANS | |

| | | | |
|------------------|--------|-----------|--------|
| PROJ. MANAGER | DATE | BY | DATE |
| DESIGN-DETAILED | 6/2021 | R. PARKER | 6/2021 |
| CHECKED-REVIEWED | | C. SCHAK | |
| DESIGN-DETAILED | | | |
| DESIGN-DETAILED | | | |
| REVISIONS 1 | | | |
| REVISIONS 2 | | | |
| REVISIONS 3 | | | |
| REVISIONS 4 | | | |
| FIELD CHANGES | | | |

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|---|------------------------|
| FARMINGTON FALLS BRIDGE | |
| SANDY RIVER | |
| CHESTERVILLE-FARMINGTON FRANKLIN COUNTY | |
| 4+25.00 | CROSS SECTIONS 4+25.00 |

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|--------------|--|
| SHEET NUMBER | |
| 20 | |
| OF 76 | |

Date: 6/29/2021



21
OF 76

FARMINGTON FALLS BRIDGE
SANDY RIVER
CHESTERVILLE-FARMINGTON FRANKLIN COUNTY

6+75.00 CROSS SECTIONS 6+75.00

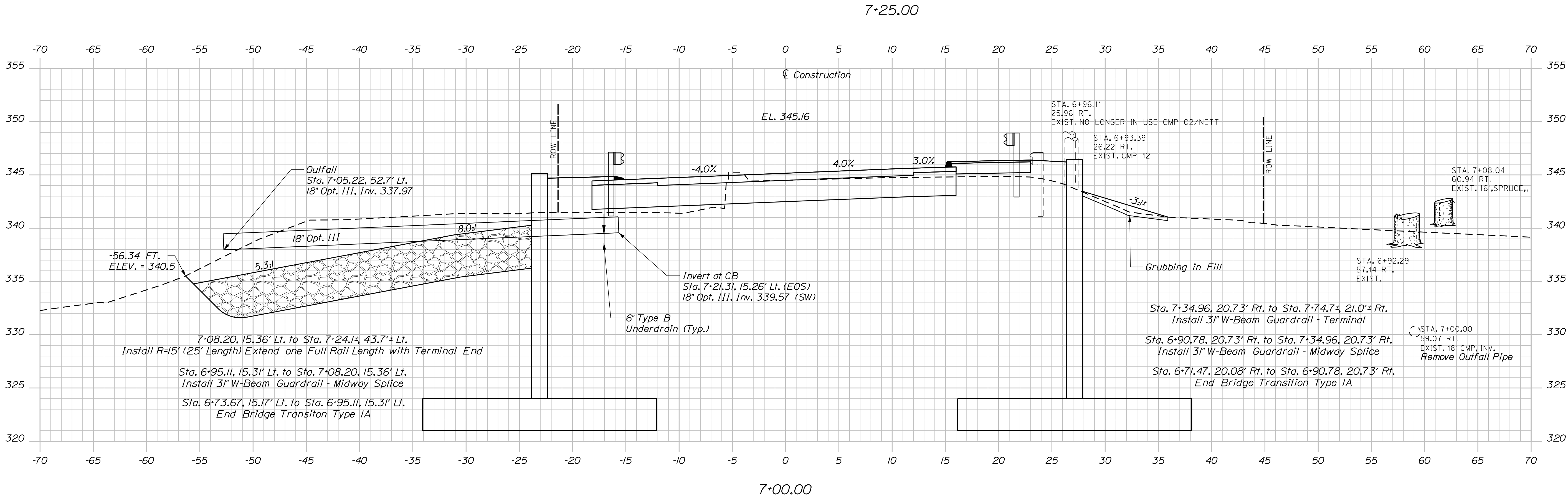
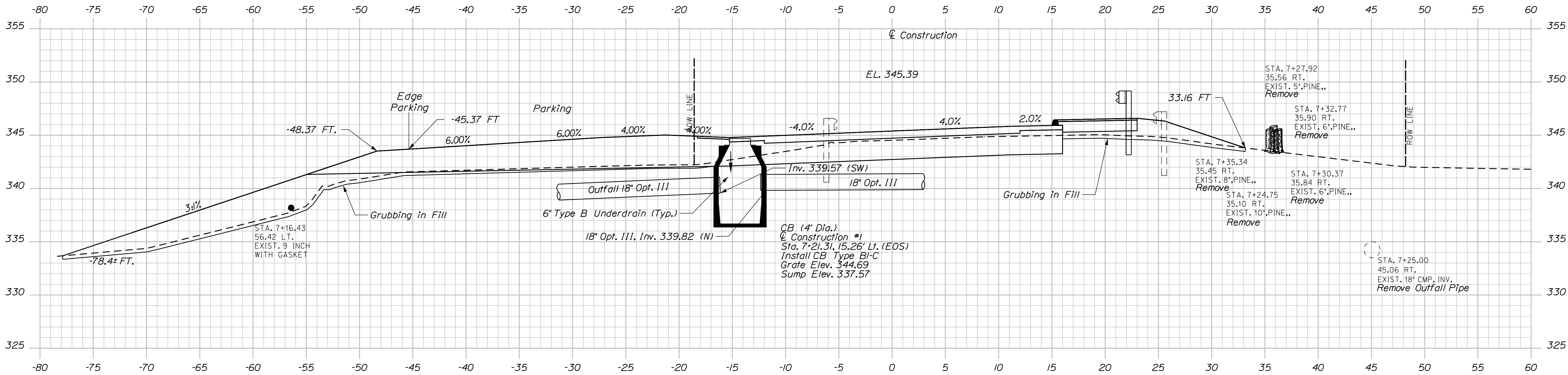
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| PROJ. MANAGER | MICHAEL NIGHT | BY | DATE | STATE OF MAINE DEPARTMENT OF TRANSPORTATION 2229600 WIN 22296.00 BRIDGE NO. 2273 BRIDGE PLANS |
| DESIGN-DETAILED | M.V. LEF VNB | R. PARKER | 6/2021 | |
| CHECKED-REVIEWED | C. SIOCHAK | C. SIOCHAK | 6/2021 | |
| DESIGNS-OET JAL D01 | | | | |
| DESIGNS-OET JAL D03 | | | | |
| REVISIONS 1 | | | | |
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| FIELD CHANGES | | | | |

Date: 6/29/2021

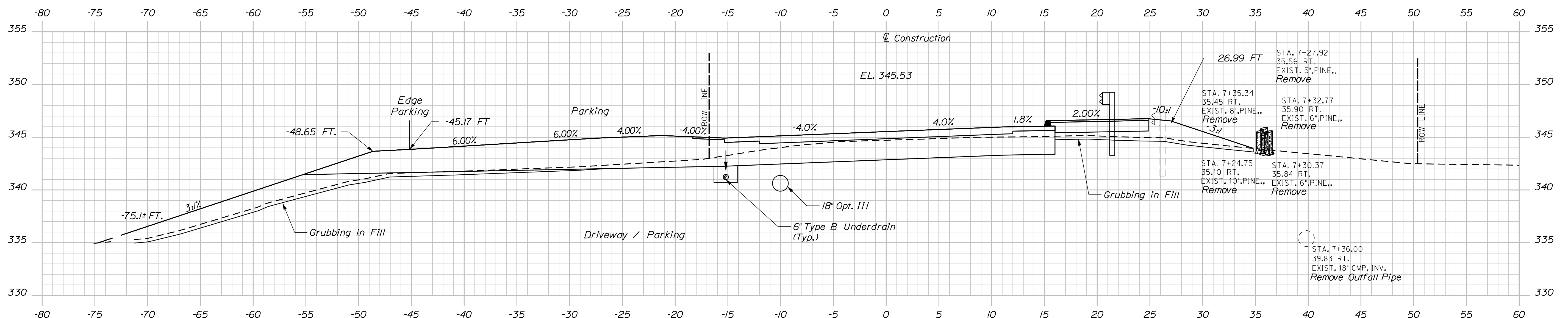
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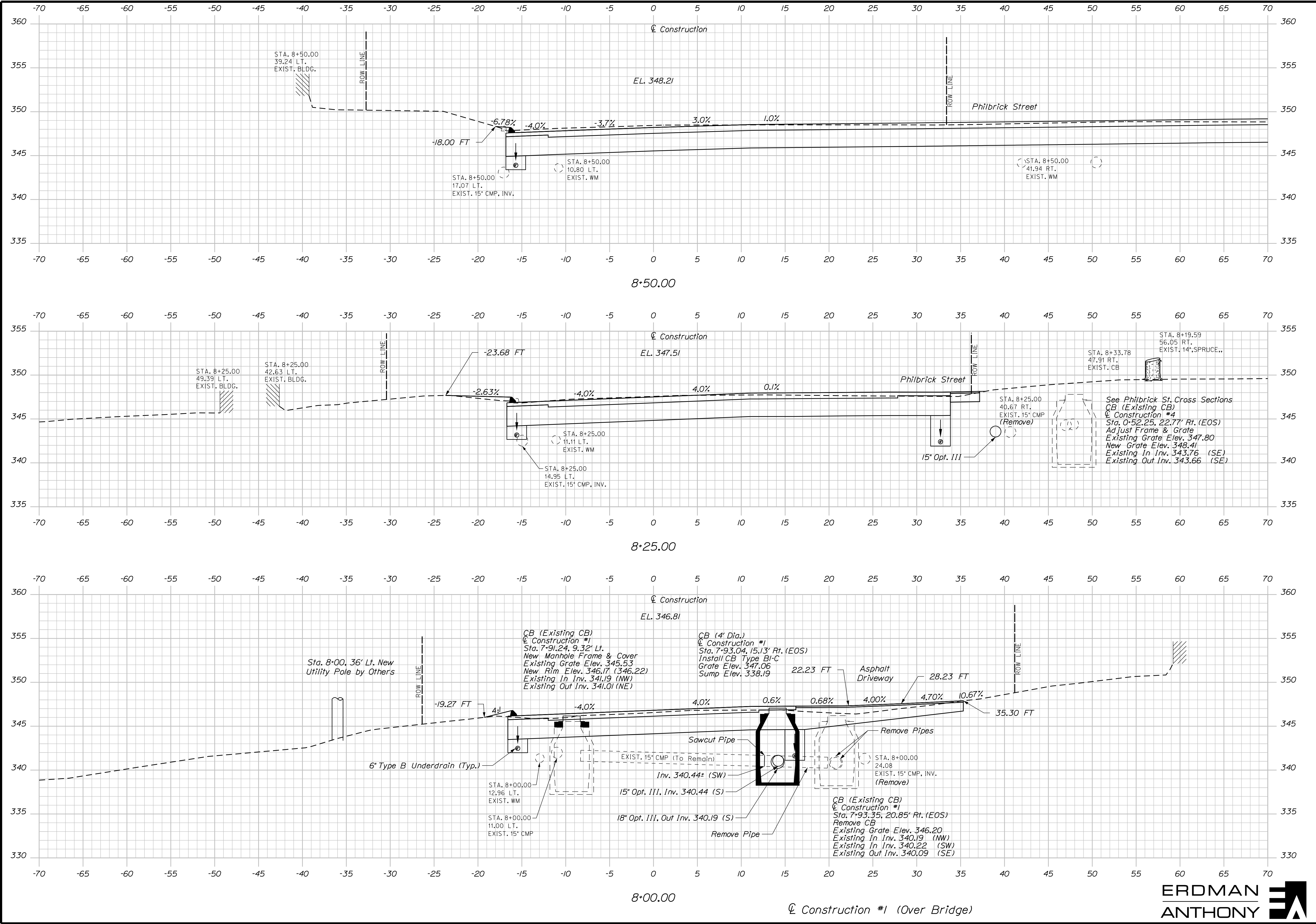
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Construction #1 (Over Bridge)





STATE OF MAINE
DEPARTMENT OF TRANSPORTATION

2229600

WIN
22296.00

BRIDGE NO. 2273

BRIDGE PLANS

PROJ. MANAGER
MICHEL WIGHT

CHECKED
MYLENE

DESIGNED
C. SICHAK

DATE
6/2021

BY
R. PARKER

SIGNATURE
C. SICHAK

P.E. NUMBER

DATE

FARMINGTON FALLS BRIDGE
SANDY RIVER
CHESTERVILLE-FARMINGTON FRANKLIN COUNTY

8+00.00 CROSS SECTIONS

8+50.00

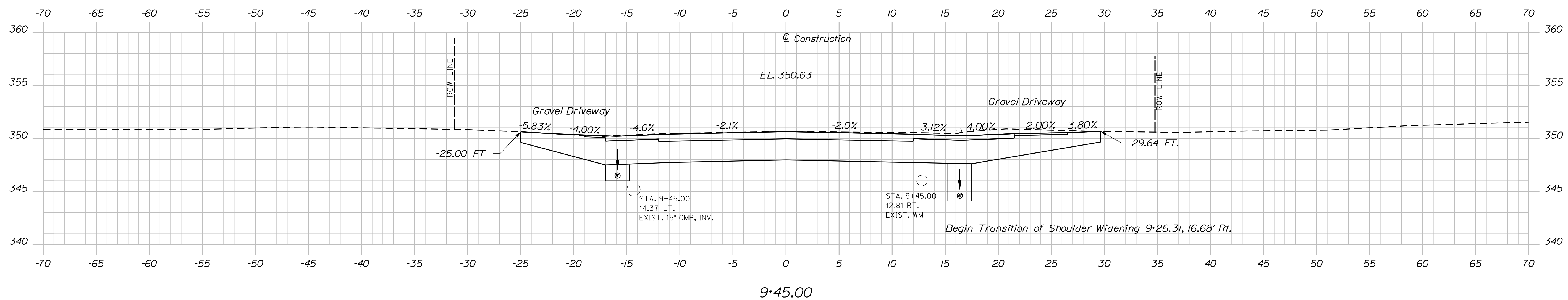
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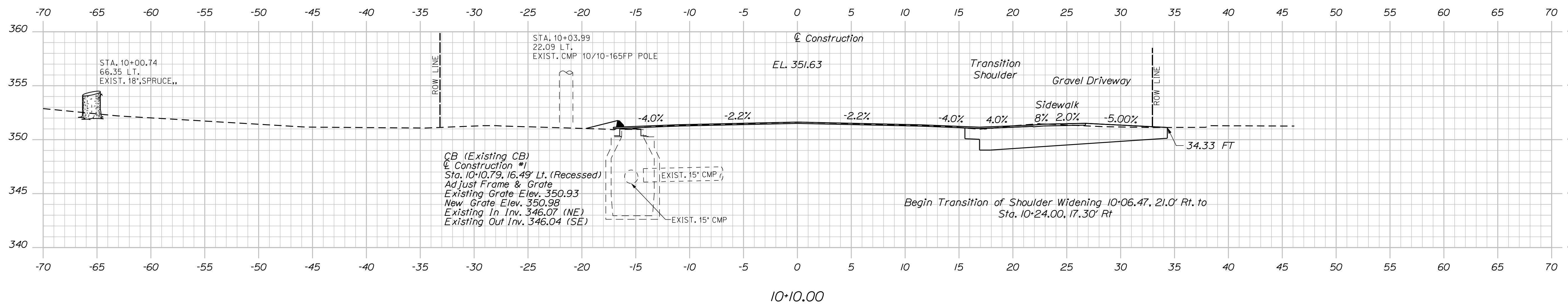
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OF 76

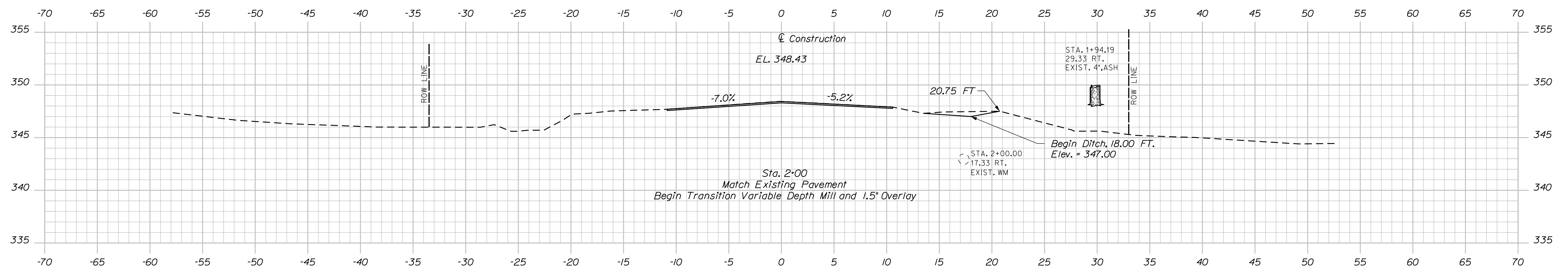
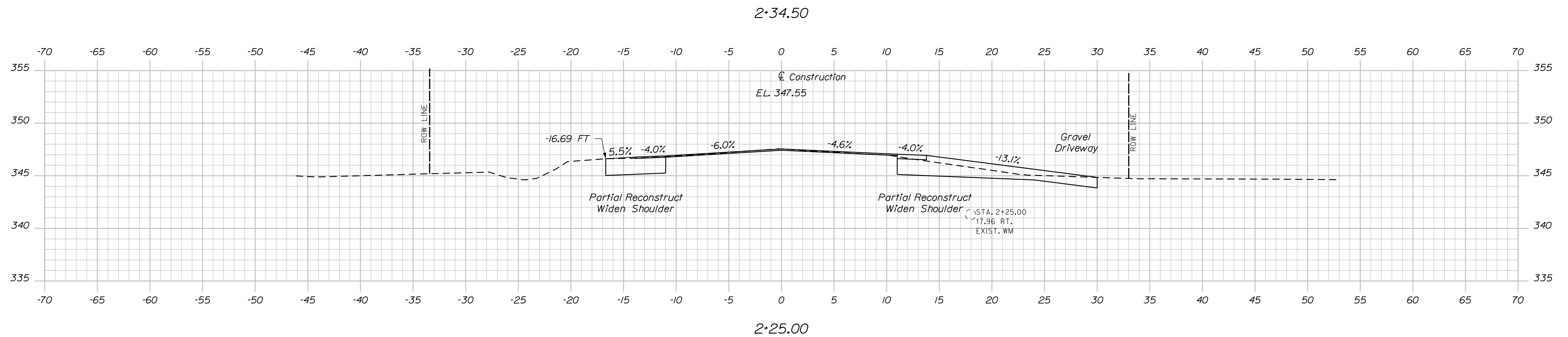
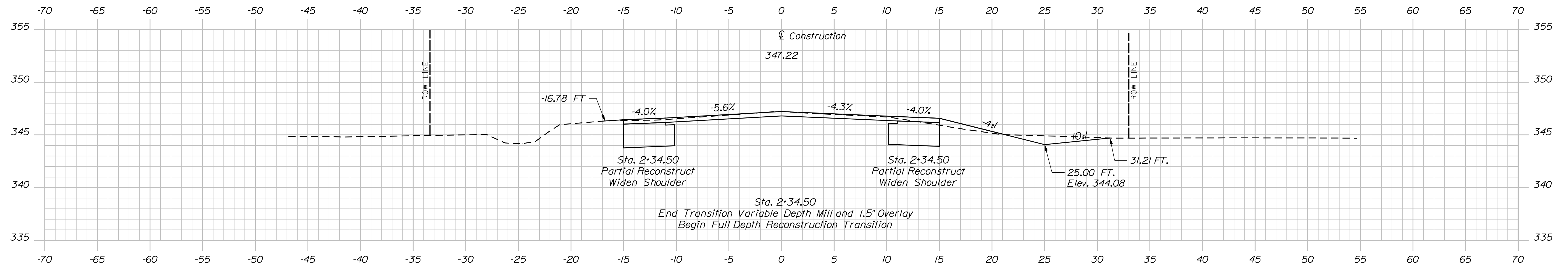
ERDMAN
ANTHONY

Filename: ... \00\Bridge\MSTA\026_Xsect11.dgn





ℒ Construction #1 (Over Bridge)



VIENNA ROAD @ Construction #2

ERDMAN
ANTHONY

DEPARTMENT OF TRANSPORTATION

2229600

2229600

| BRIDGE NO. 2273 | 22296.00 | BRIDGE PLANS |
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| P.E. NUMBER | REVISIONS 1 | | | | |
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| | REVISIONS 3 | | | | |
| | REVISIONS 4 | | | | |
| DATE | FIELD CHANGES | | | | |
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| SIGNATURE | DESIGN-DETAILED3 | | | | |
| | DESIGN-DETAILED2 | | | | |
| | CHECKED-REVIEWED | | | | |
| | DESIGN-DETAILED1 | | | | |
| P.E. NUMBER | REVISIONS 1 | | | | |
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| DATE | FIELD CHANGES | | | | |
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| SIGNATURE | DESIGN-DETAILED3 | | | | |
| | DESIGN-DETAILED2 | | | | |
| | CHECKED-REVIEWED | | | | |
| | DESIGN-DETAILED1 | | | | |
| P.E. NUMBER | REVISIONS 1 | | | | |
| | REVISIONS 2 | | | | |
| | REVISIONS 3 | | | | |
| | REVISIONS 4 | | | | |
| DATE | FIELD CHANGES | | | | |
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| SIGNATURE | DESIGN-DETAILED3 | | | | |
| | DESIGN-DETAILED2 | | | | |
| | CHECKED-REVIEWED | | | | |
| | DESIGN-DETAILED1 | | | | |
| P.E. NUMBER | REVISIONS 1 | | | | |
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| DATE | FIELD CHANGES | | | | |
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| SIGNATURE | DESIGN-DETAILED3 | | | | |
| | DESIGN-DETAILED2 | | | | |
| | CHECKED-REVIEWED | | | | |
| | DESIGN-DETAILED1 | | | | |
| P.E. NUMBER | REVISIONS 1 | | | | |
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| | REVISIONS 3 | | | | |
| | REVISIONS 4 | | | | |
| DATE | FIELD CHANGES | | | | |
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| SIGNATURE | DESIGN-DETAILED3 | | | | |
| | DESIGN-DETAILED2 | | | | |
| | CHECKED-REVIEWED | | | | |
| | DESIGN-DETAILED1 | | | | |
| P.E. NUMBER | REVISIONS 1 | | | | |
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| | REVISIONS 3 | | | | |
| | REVISIONS 4 | | | | |
| DATE | FIELD CHANGES | | | | |
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| SIGNATURE | DESIGN-DETAILED3 | | | | |
| | DESIGN-DETAILED2 | | | | |
| | CHECKED-REVIEWED | | | | |
| | DESIGN-DETAILED1 | | | | |
| P.E. NUMBER | REVISIONS 1 | | | | |
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| DATE | FIELD CHANGES | | | | |
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| SIGNATURE | DESIGN-DETAILED3 | | | | |
| | DESIGN-DETAILED2 | | | | |
| | CHECKED-REVIEWED | | | | |
| | DESIGN-DETAILED1 | | | | |
| P.E. NUMBER | REVISIONS 1 | | | | |
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| | REVISIONS 3 | | | | |
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| DATE | FIELD CHANGES | | | | |
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| SIGNATURE | DESIGN-DETAILED3 | | | | |
| | DESIGN-DETAILED2 | | | | |
| | CHECKED-REVIEWED | | | | |
| | DESIGN-DETAILED1 | | | | |
| P.E. NUMBER | REVISIONS 1 | | | | |
| | REVISIONS 2 | | | | |
| | REVISIONS 3 | | | | |
| | REVISIONS 4 | | | | |
| DATE | FIELD CHANGES | | | | |
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FARMINGTON FALLS BRIDGE
SANDY RIVER
CHESTERVILLE-FARMINGTON FRANKLIN COUNTY

2+00.00 VIENNA ROAD CROSS SECTIONS 2+34.50

SHEET NUMBER

29

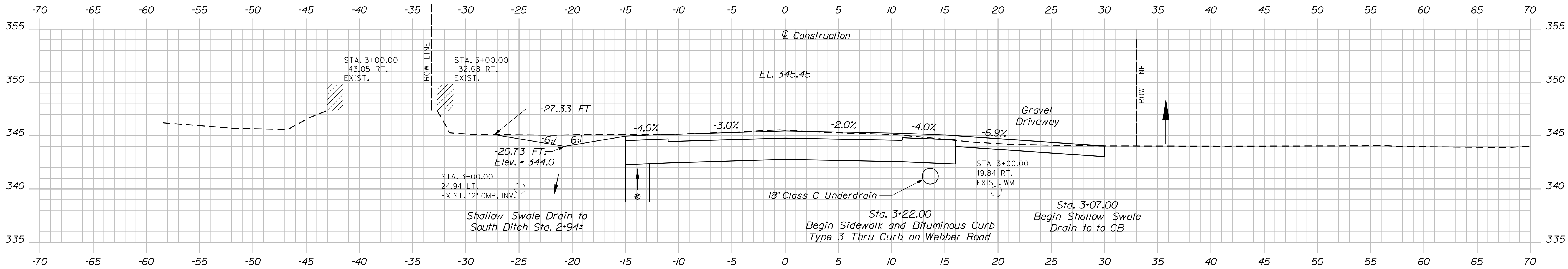
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Date:6/29/2021

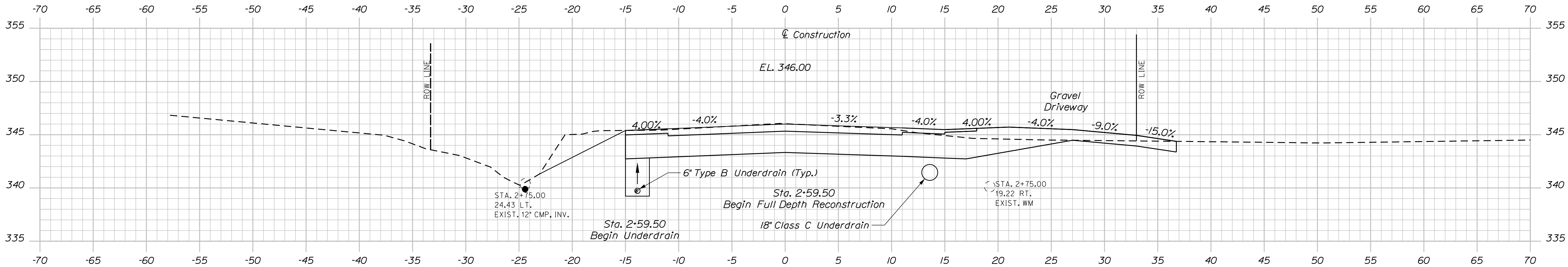
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Division: BRIDGE

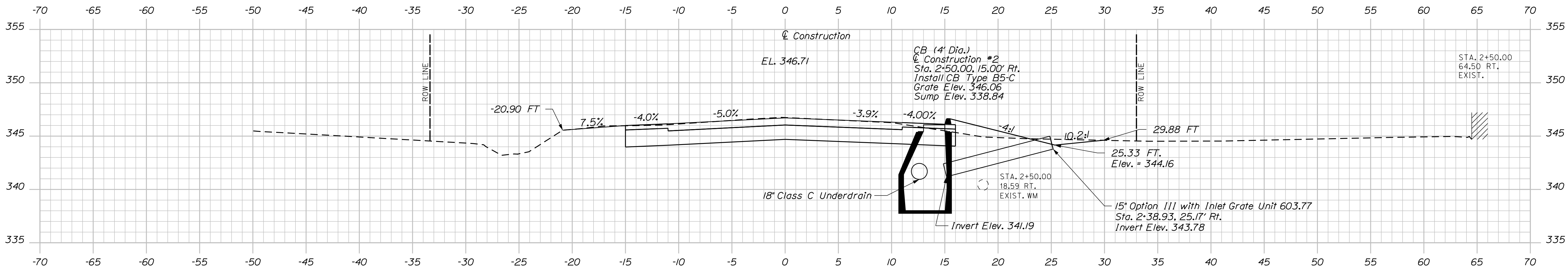
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3+00.00



2+75.00



2+50.00

VIENNA ROAD @ Construction #2



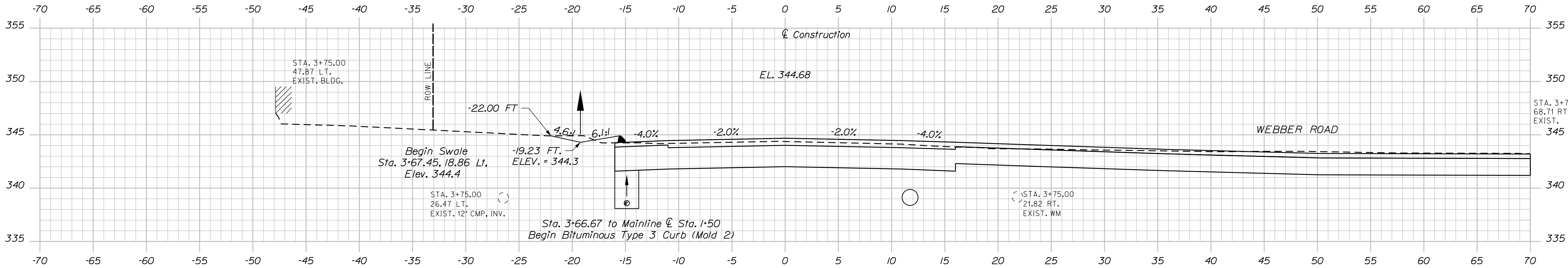
| PROJ. MANAGER | BY | DATE | SIGNATURE | P.E. NUMBER | DATE |
|--|---|------------------|-----------|-------------|------|
| DESIGN-DETAILED CHECKED-REVIEWED DESIGN-DETAILED DESIGN-DETAILED REVISIONS 1 REVISIONS 2 REVISIONS 3 REVISIONS 4 FIELD CHANGES | MICHAEL WIGHT R. PARKER C. SICHAK | 6/2021 6/2021 | | | |

Date:6/29/2021

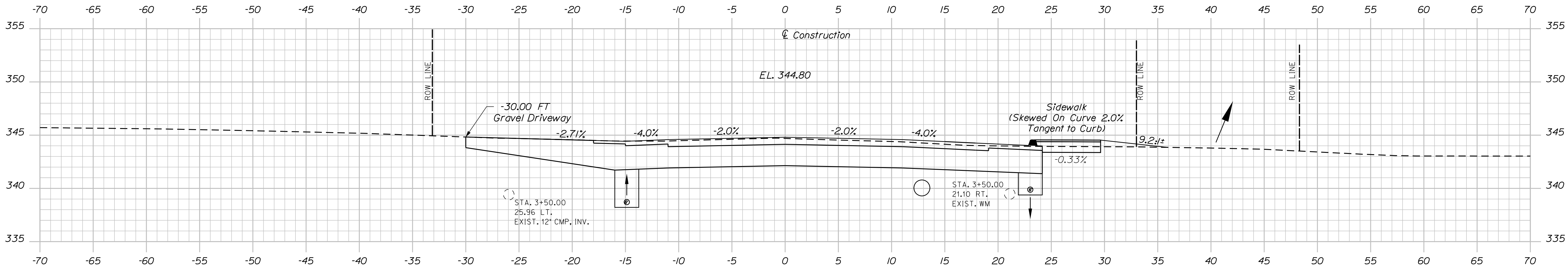
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Division: BRIDGE

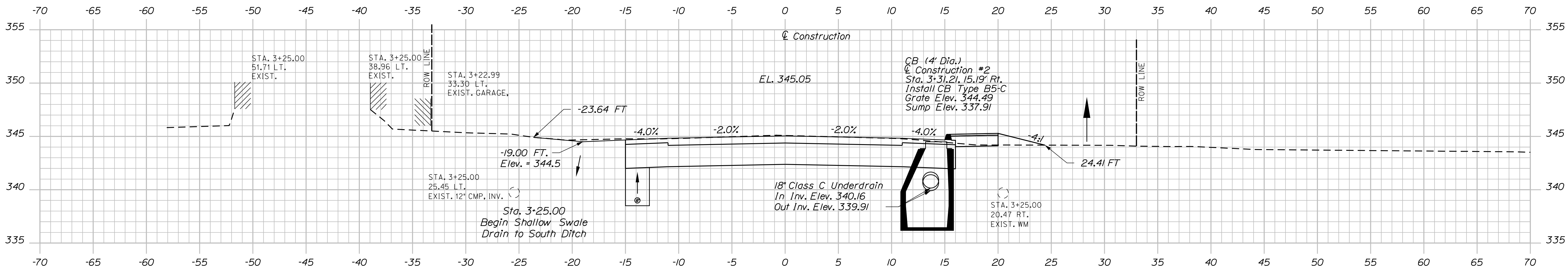
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3+75.00



3+50.00



3+25.00

VIENNA ROAD @ Construction #2



| | |
|------------------------------|----------|
| STATE OF MAINE | |
| DEPARTMENT OF TRANSPORTATION | |
| 2229600 | |
| WIN | 22296.00 |
| BRIDGE NO. 2273 | |
| BRIDGE PLANS | |

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|---------------|---------|-----------|-------------|
| PROJ. MANAGER | CHECKED | DESIGNED | DATE |
| MICHAEL WIGHT | MYLENE | 6/2021 | 6/2021 |
| BY | DATE | SIGNATURE | P.E. NUMBER |
| R. PARKER | 6/2021 | | |
| C. SICHAK | 6/2021 | | |
| DESIGNED | | DATE | |
| REVISIONS 1 | | DATE | |
| REVISIONS 2 | | DATE | |
| REVISIONS 3 | | DATE | |
| REVISIONS 4 | | DATE | |
| FIELD CHANGES | | DATE | |

| | |
|---|---------|
| FARMINGTON FALLS BRIDGE | 3+75.00 |
| SANDY RIVER | |
| CHESTERVILLE-FARMINGTON FRANKLIN COUNTY | |
| VIENNA ROAD | |
| CROSS SECTIONS | |

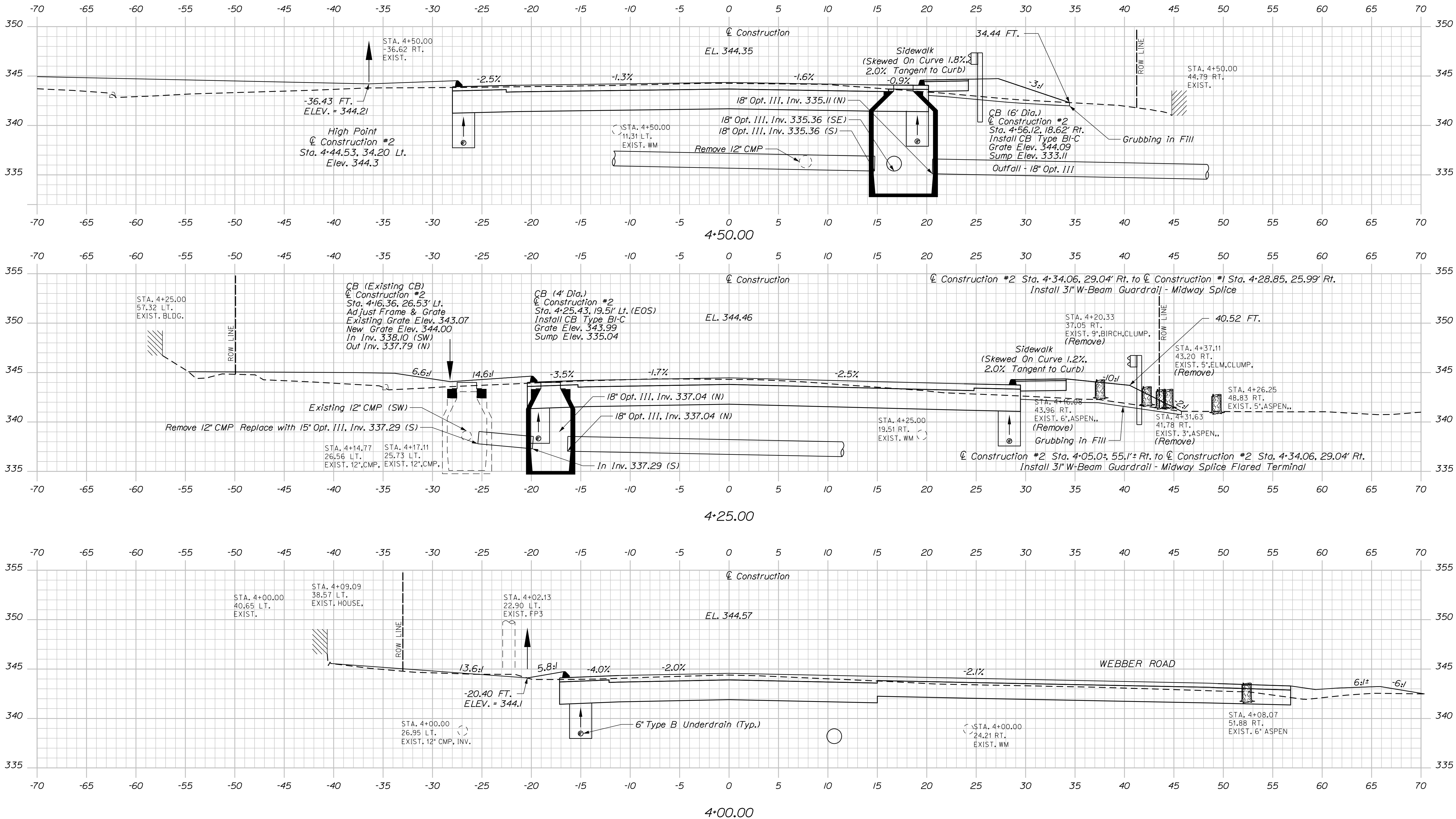
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| SHEET NUMBER |
| 31 |
| OF 76 |

Date: 6/29/2021

Username: LindoT

Division: BRIDGE

Filename: ... \Bridge\WSTA\032_Xsect-2-17.dgn



VIENNA ROAD @ Construction #2



STATE OF MAINE
DEPARTMENT OF TRANSPORTATION

2229600

WIN
22296.00

BRIDGE NO. 2273

BRIDGE PLANS

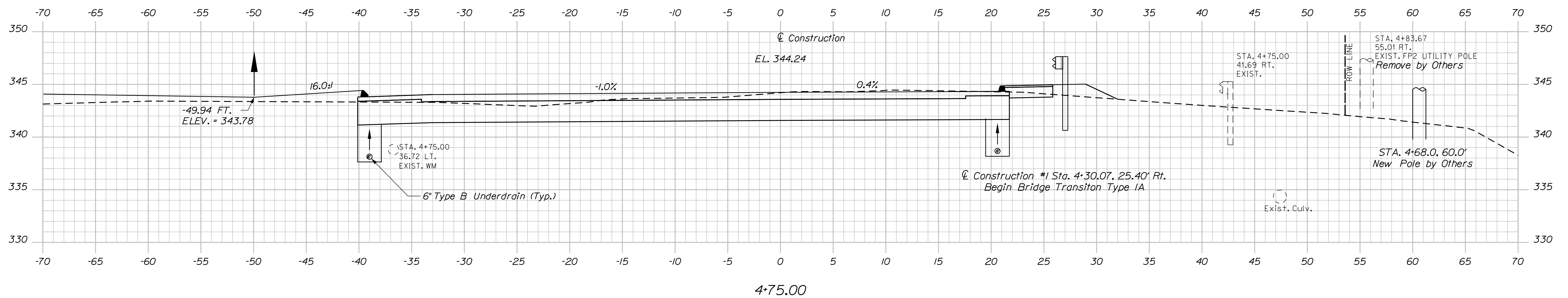
FARMINGTON FALLS BRIDGE
SANDY RIVER
CHESTERVILLE-FARMINGTON FRANKLIN COUNTY

VIENNA ROAD
CROSS SECTIONS

4+00.00
4+50.00

| PROJ. MANAGER | BY | DATE | SIGNATURE | P.E. NUMBER | DATE |
|------------------|-----------|--------|-----------|-------------|------|
| MICHAEL WIGHT | R. PARKER | 6/2021 | | | |
| CHECKED-REVIEWED | C. SICHAK | 6/2021 | | | |
| DESIGN-DETAILED | | | | | |
| DESIGN-DETAILED | | | | | |
| REVISIONS 1 | | | | | |
| REVISIONS 2 | | | | | |
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| FIELD CHANGES | | | | | |

SHEET NUMBER
32
OF 76

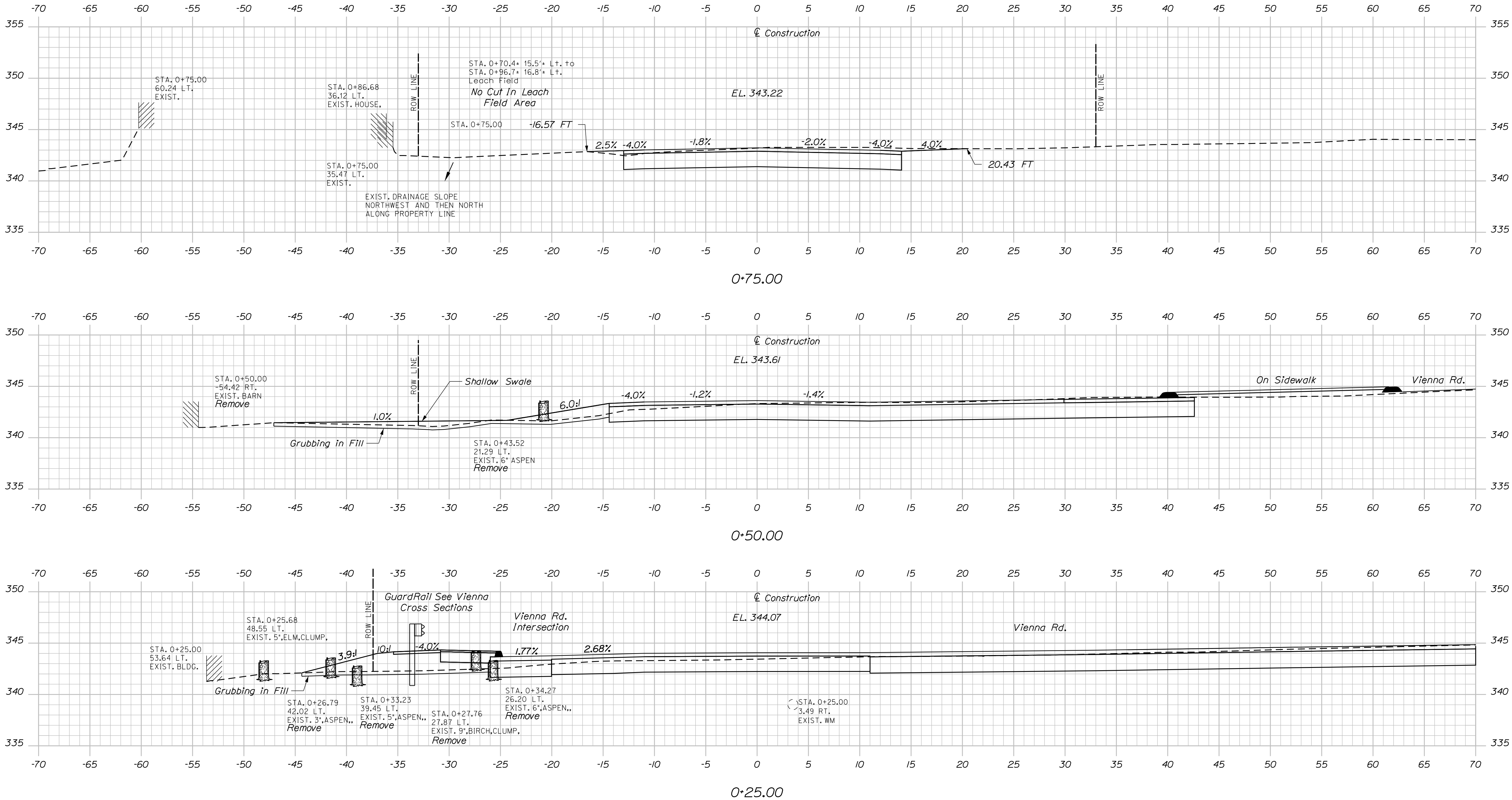


Date: 6/29/2021

Username: LindoT

Division: BRIDGE

Filename: ...\\Bridge\\MSTA\\034-Xsect-3-19.dgn



WEBBER ROAD @ Construction #3



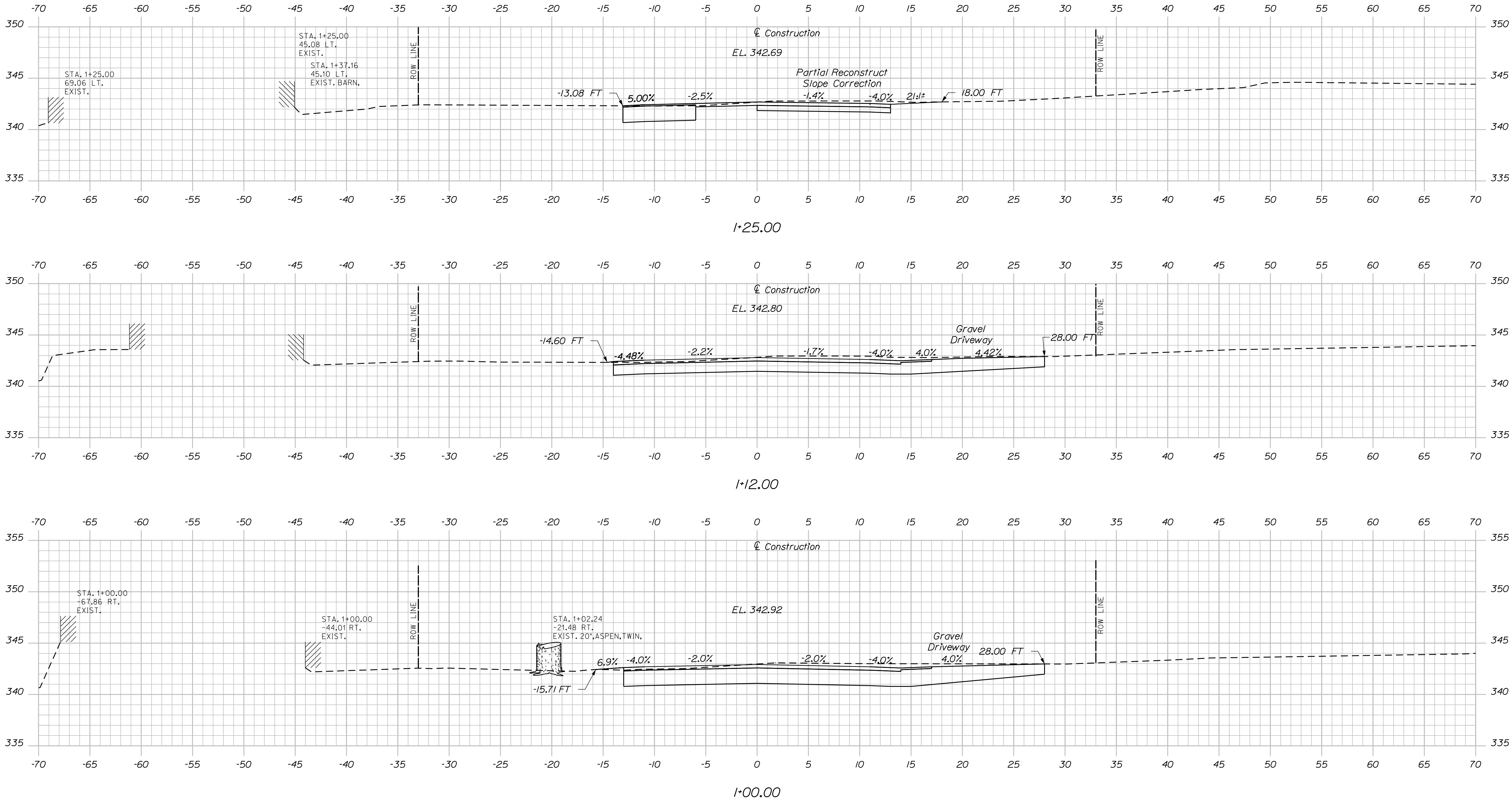
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| STATE OF MAINE | | DEPARTMENT OF TRANSPORTATION | |
| 2229600 | | 22296.00 | |
| BRIDGE NO. 2273 | | WIN | |
| 22296.00 | | BRIDGE PLANS | |
| FARMINGTON FALLS BRIDGE | | SANDY RIVER | |
| CHESTERVILLE-FARMINGTON FRANKLIN COUNTY | | WEBBER ROAD | |
| 0+25.00 | | 0+75.00 | |
| CROSS SECTIONS | | CROSS SECTIONS | |
| SHEET NUMBER | | 34 | |
| OF 76 | | OF 76 | |

Date:6/29/2021

Username: LindoT

Division: BRIDGE

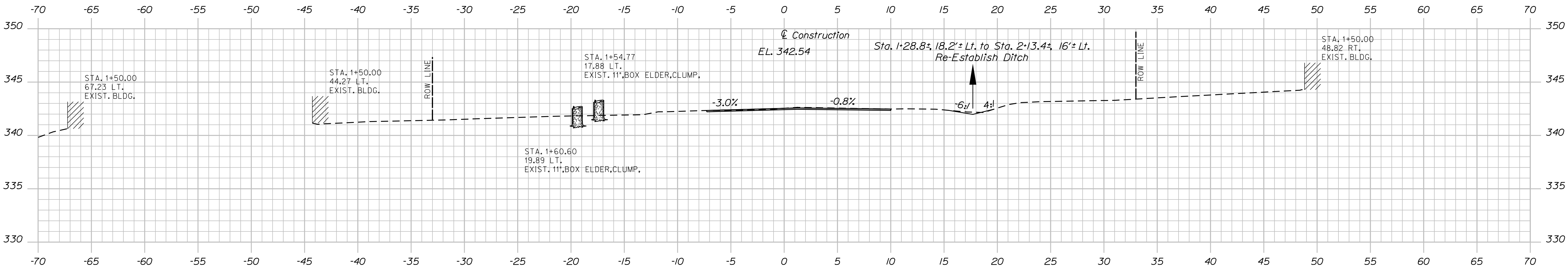
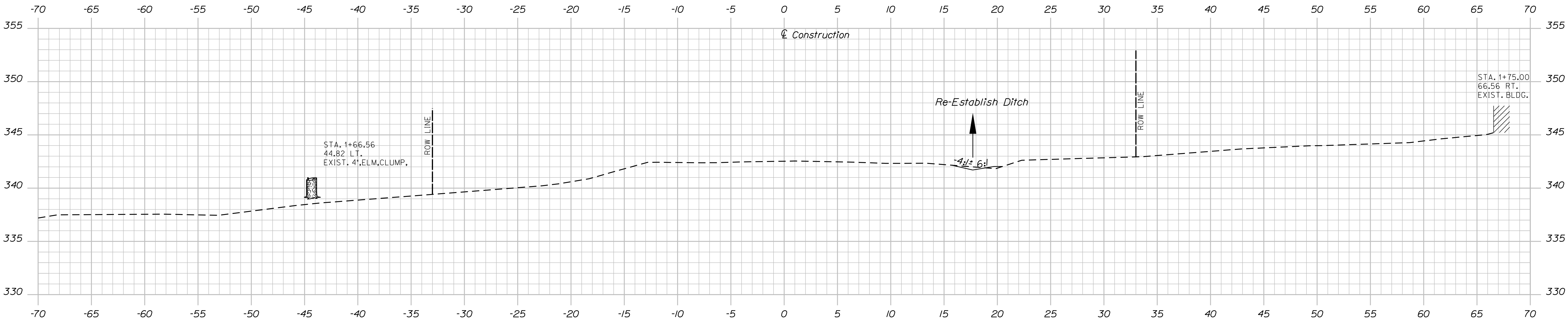
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WEBBER ROAD @ Construction #3



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| STATE OF MAINE | | DEPARTMENT OF TRANSPORTATION | |
| 2229600 | | WIN | |
| 22296.00 | | BRIDGE NO. 2273 | |
| 22296.00 | | BRIDGE PLANS | |
| FARMINGTON FALLS BRIDGE | | SANDY RIVER | |
| CHESTERVILLE-FARMINGTON FRANKLIN COUNTY | | WEBBER ROAD | |
| I+00.00 | | I+25.00 | |
| CROSS SECTIONS | | SHEET NUMBER | |
| 35 | | OF 76 | |
| PROJ. MANAGER | | BY | |
| DESIGN-DETAILED | | DATE | |
| CHECKED-REVIEWED | | SIGNATURE | |
| DESIGN-DETAILED | | P.E. NUMBER | |
| DESIGN-DETAILED | | DATE | |
| REVISIONS 1 | | FIELD CHANGES | |
| REVISIONS 2 | | | |
| REVISIONS 3 | | | |
| REVISIONS 4 | | | |



WEBBER ROAD @ Construction #3



| | |
|------------------------------|--------------|
| STATE OF MAINE | |
| DEPARTMENT OF TRANSPORTATION | |
| 2229600 | |
| BRIDGE NO. 2273 | WIN 22296.00 |
| BRIDGE PLANS | |

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|-----------------|---------------|------------------|-----------|-------------|--------|
| PROJ. MANAGER | MICHAEL WIGHT | BY | R. PARKER | DATE | 6/2021 |
| DESIGN-DETAILED | MYLENE | CHECKED-REVIEWED | C. SICHAK | SIGNATURE | |
| DESIGN-DETAILED | | DESIGN-DETAILED | | P.E. NUMBER | |
| REVISIONS 1 | | REVISIONS 2 | | DATE | |
| REVISIONS 3 | | REVISIONS 4 | | | |
| FIELD CHANGES | | | | | |

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| FARMINGTON FALLS BRIDGE | |
| SANDY RIVER | |
| CHESTERVILLE-FARMINGTON FRANKLIN COUNTY | |
| 1+50.00 | 1+75.00 |
| WEBBER ROAD CROSS SECTIONS | |

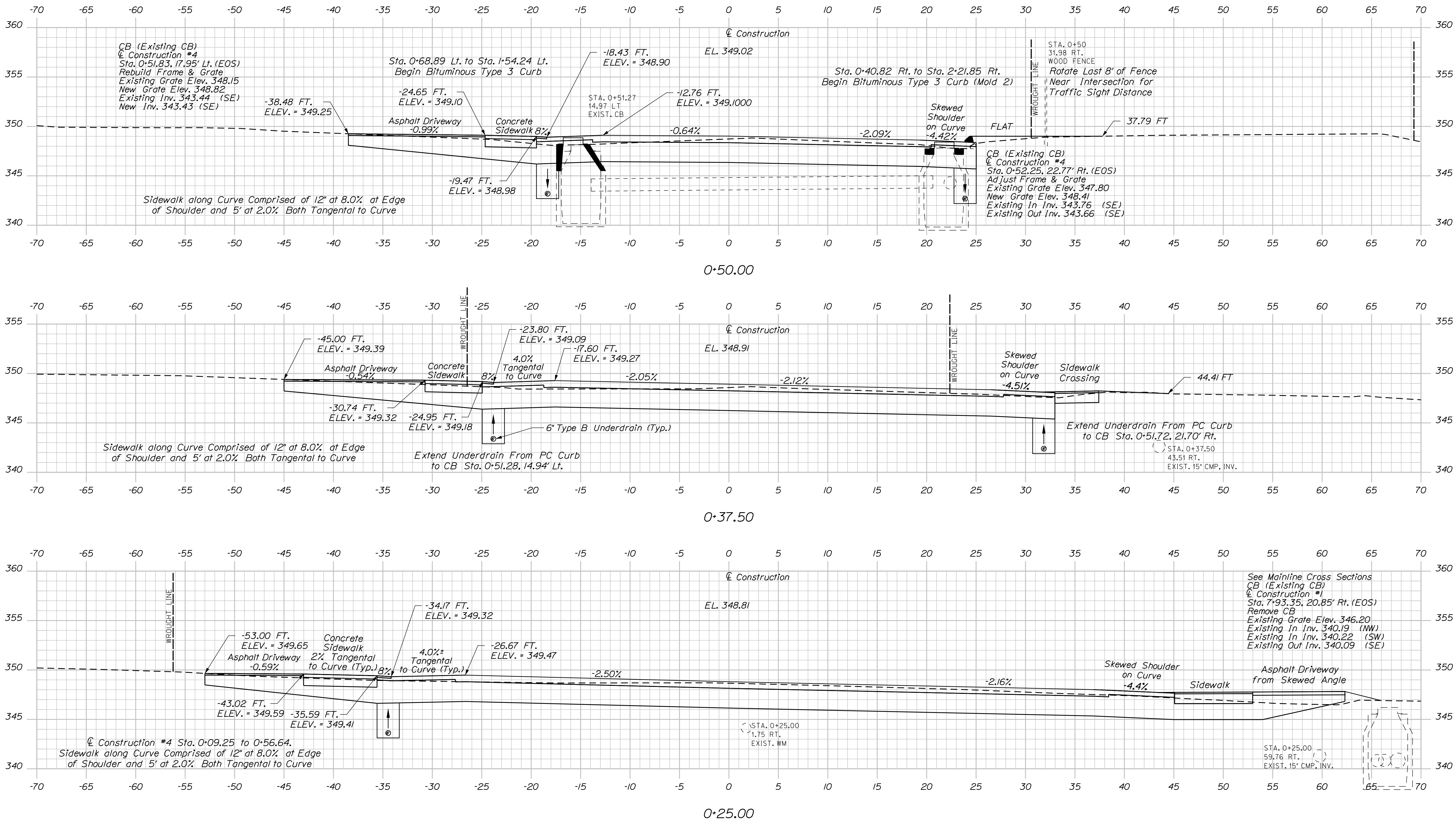
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| 36 | |
| OF 76 | |

Date: 6/29/2021

Username: LindoT

Division: BRIDGE

Filename: ...\\Bridge\\MST\\037_Xsect-4-22.dgn



Construction #4 (Philbrick Rd)



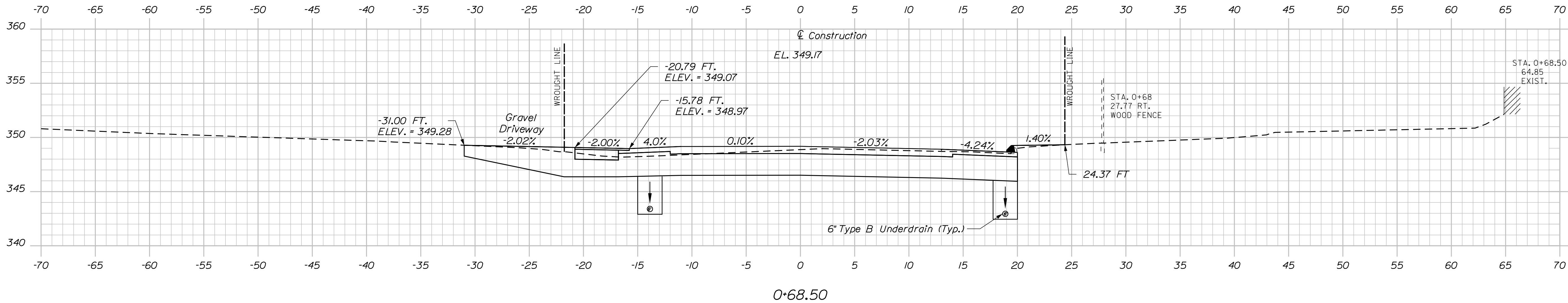
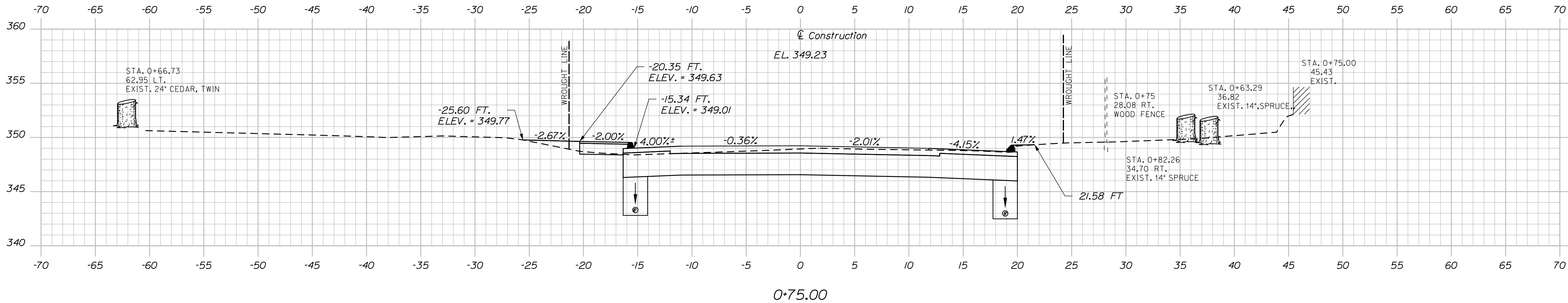
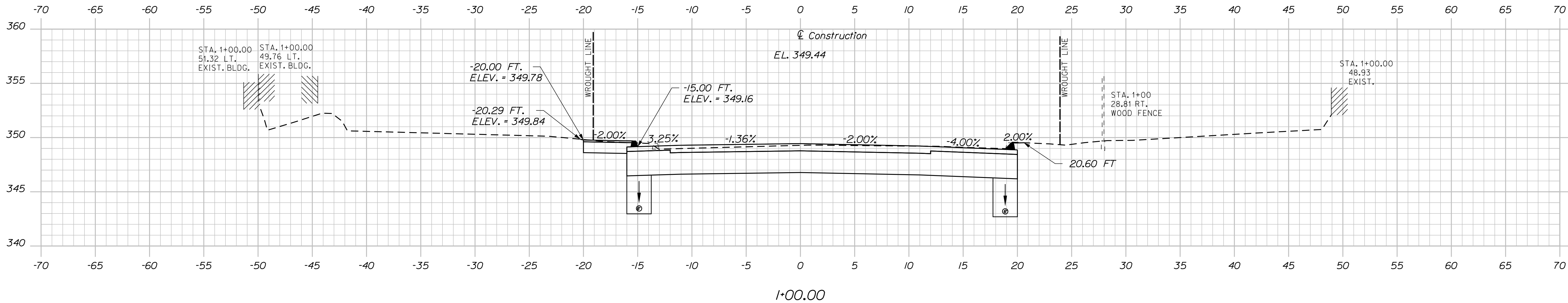
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| STATE OF MAINE | | DEPARTMENT OF TRANSPORTATION | | 2229600 | | WIN | | 22296.00 | | BRIDGE NO. 2273 | | BRIDGE PLANS | |
| FARMINGTON FALLS BRIDGE | | SANDY RIVER | | CHESTERVILLE-FARMINGTON FRANKLIN COUNTY | | PHILBRICK ROAD | | CROSS SECTIONS | | 0+25.00 | | 0+50.00 | |
| PROJECT MANAGER | | CHECKED | | DESIGNED | | REVISIONS | | DATE | | SIGNATURE | | P.E. NUMBER | |
| MICHAEL WIGHT | | MYLENE | | C. SICHAK | | C. SICHAK | | 6/2021 | | 6/2021 | | 6/2021 | |
| BY | | DATE | | SIGNATURE | | P.E. NUMBER | | DATE | | SIGNATURE | | P.E. NUMBER | |
| R. PARKER | | 6/2021 | | C. SICHAK | | 6/2021 | | 6/2021 | | 6/2021 | | 6/2021 | |
| REVISIONS 1 | | REVISIONS 2 | | REVISIONS 3 | | REVISIONS 4 | | FIELD CHANGES | | REVISIONS 1 | | REVISIONS 2 | |
| REVISIONS 3 | | REVISIONS 4 | | FIELD CHANGES | | REVISIONS 1 | | REVISIONS 2 | | REVISIONS 3 | | REVISIONS 4 | |
| SHEET NUMBER | | 37 | | OF 76 | | | | | | | | | |

Date:6/29/2021

Username: LindoT

Division: BRIDGE

Filename: ... \Bridge\MSTA\038_Xsect-4-23.dgn



Construction #4 (Philbrick Rd)



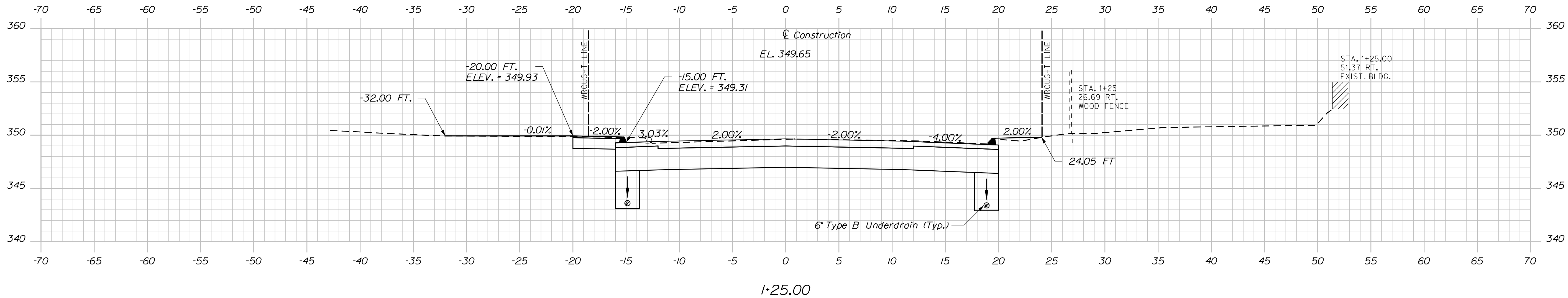
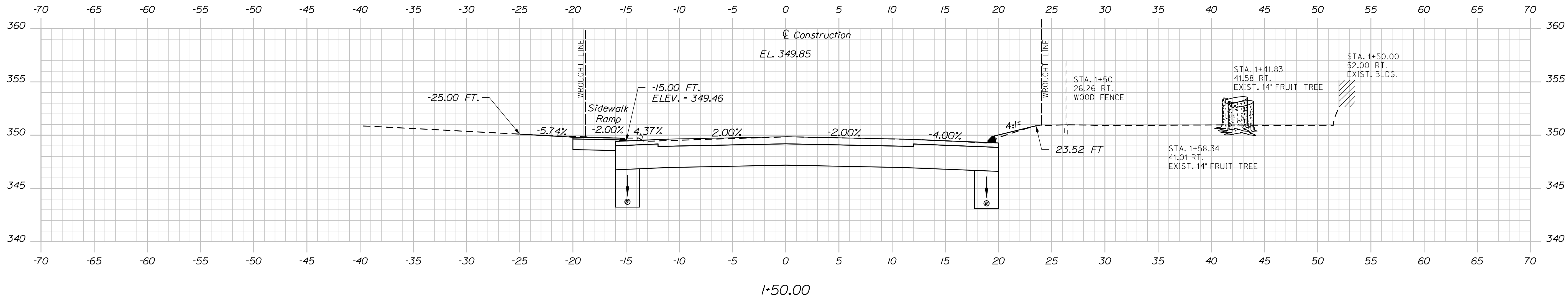
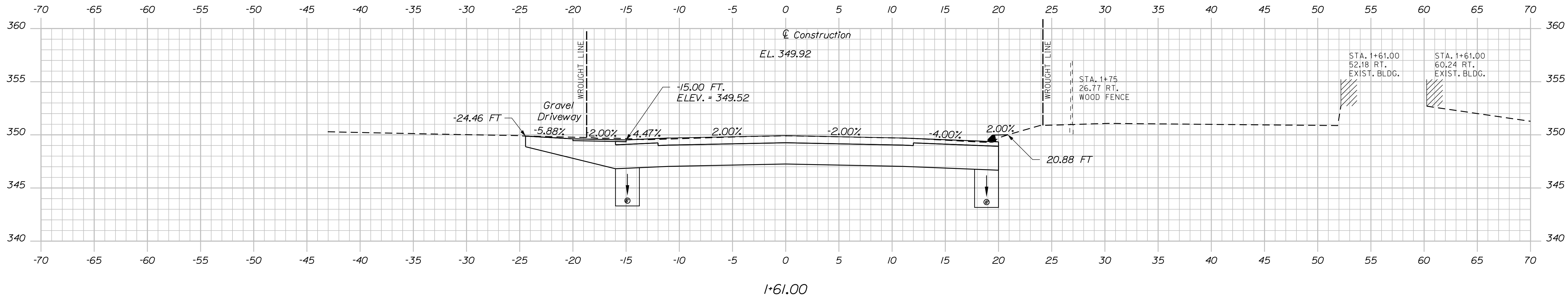
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| STATE OF MAINE DEPARTMENT OF TRANSPORTATION | | 2229600 | | BRIDGE NO. 2273 | | WIN 22296.00 | | BRIDGE PLANS | |
| FARMINGTON FALLS BRIDGE SANDY RIVER CHESTERVILLE-FARMINGTON FRANKLIN COUNTY | | PHILBRICK ROAD CROSS SECTIONS | | SHEET NUMBER 38 OF 76 | | DATE 6/2021 | | SIGNATURE P.E. NUMBER DATE | |
| PROJ. MANAGER MICHAEL WIGHT | | BY R. PARKER C. SICHAK | | CHECKED-REVIEWED M. WIGHT C. SICHAK | | DESIGN-DETAILED DESIGN-REVIEWED DESIGN-DETAILED DESIGN-REVIEWED | | REVISIONS 1 2 3 4 | |
| DATE 6/2021 | | DATE 6/2021 | | DATE 6/2021 | | DATE 6/2021 | | DATE 6/2021 | |

Date:6/29/2021

Username: LindoT

Division: BRIDGE

Filename: ... \Bridge\MS1A\039_Xsect-4-24.dgn



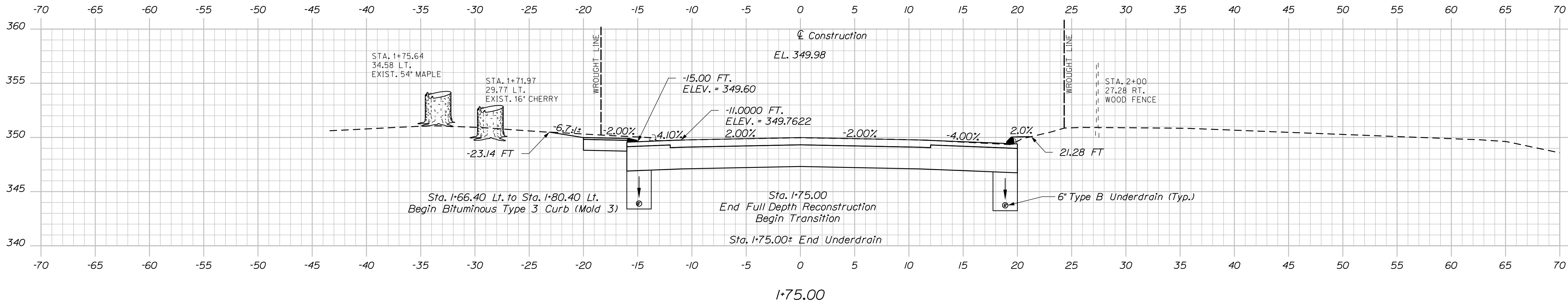
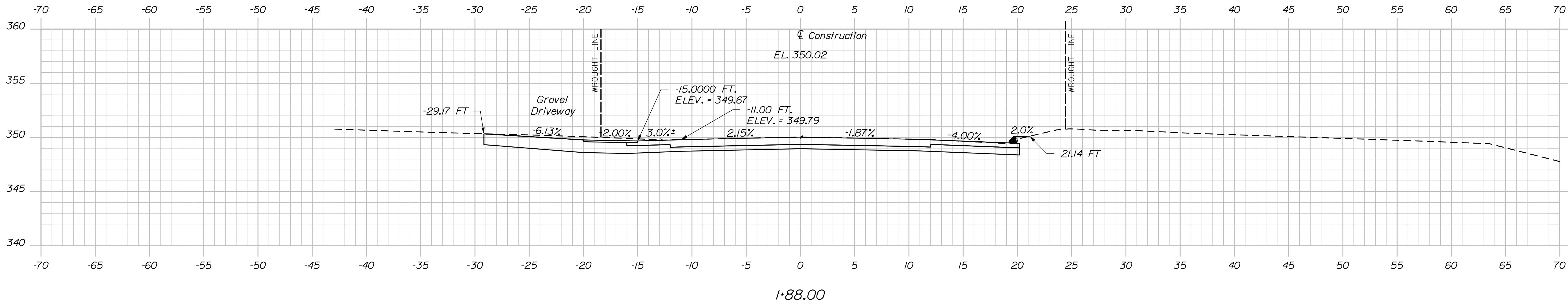
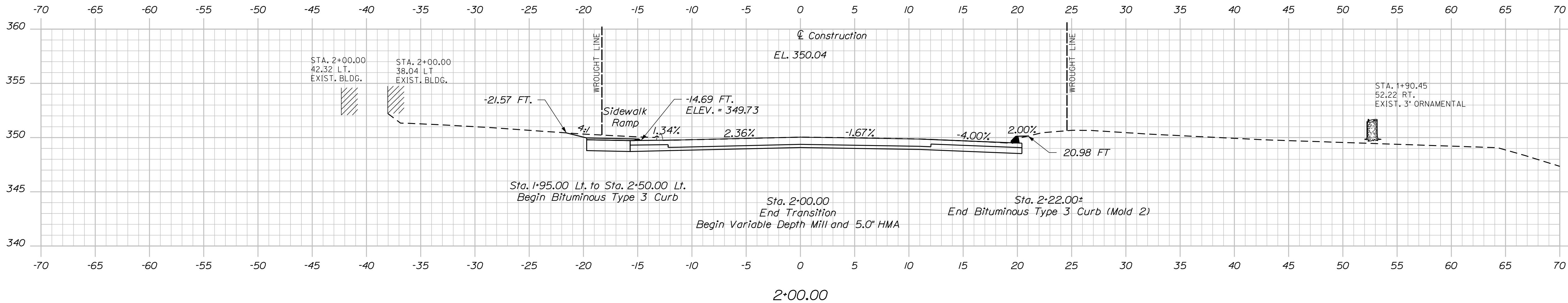
Construction #4 (Philbrick Rd)

Date:6/29/2021

Username: LindoT

Division: BRIDGE

Filename: ... \Bridge\MSTA\040_Xsect-4-25.dgn



Construction #4 (Philbrick Rd)



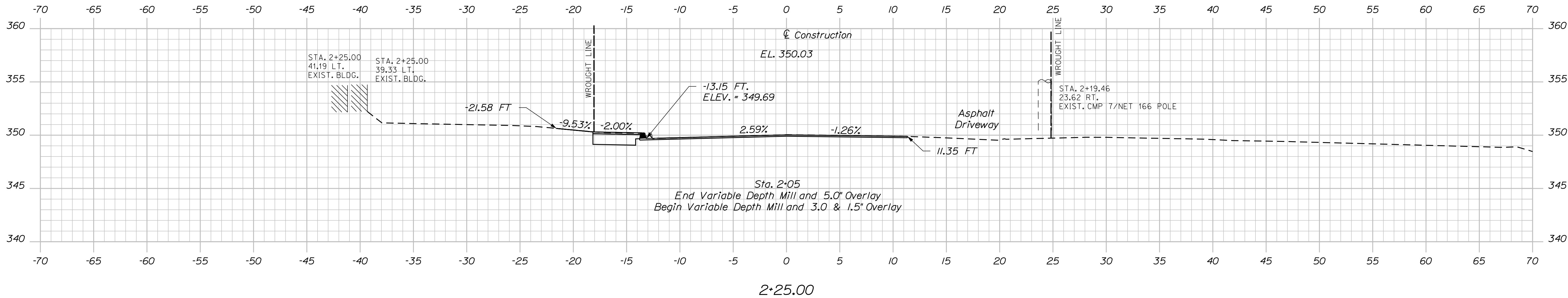
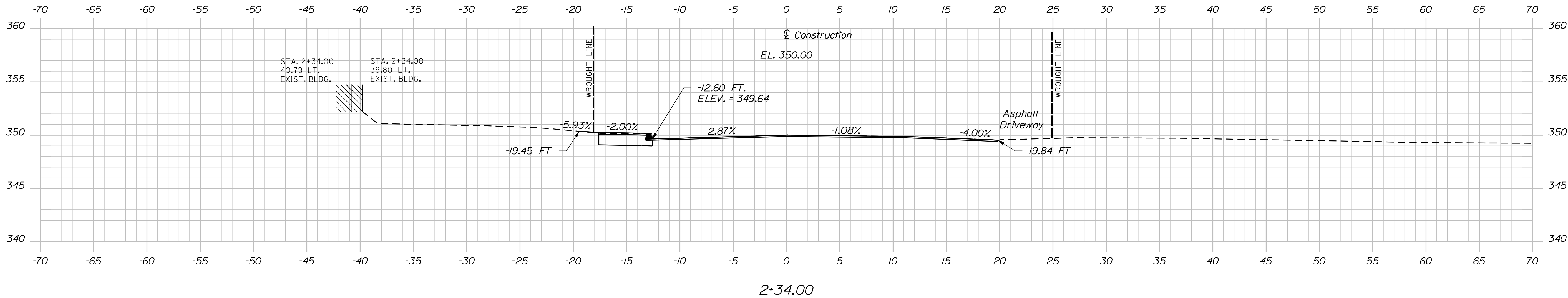
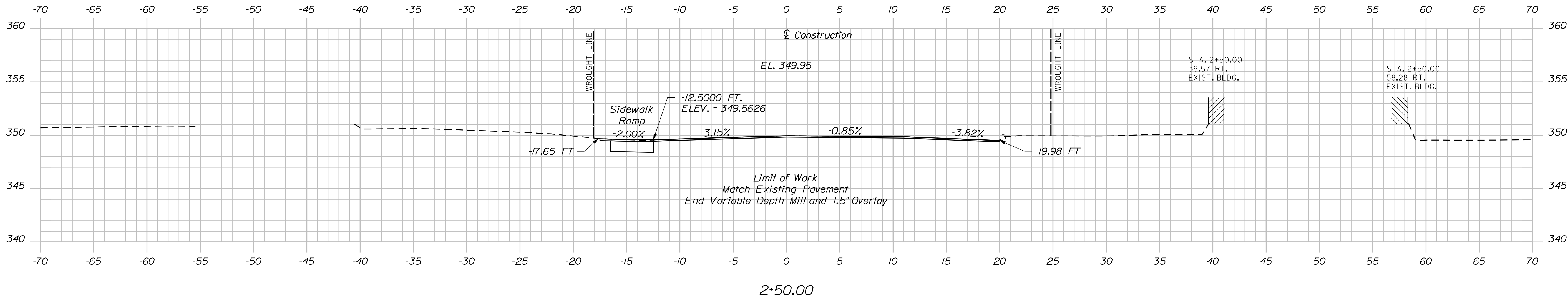
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| STATE OF MAINE DEPARTMENT OF TRANSPORTATION | | 2229600 | | BRIDGE NO. 2273 | | WIN 22296.00 | | BRIDGE PLANS | |
| FARMINGTON FALLS BRIDGE SANDY RIVER CHESTERVILLE-FARMINGTON FRANKLIN COUNTY | | PHILBRICK ROAD CROSS SECTIONS | | SHEET NUMBER 40 | | OF 76 | | | |
| PROJ. MANAGER | | MICHAEL WIGHT | | BY | | DATE | | SIGNATURE | |
| DESIGN-DETAILED | | MYLENE | | R. PARKER | | 6/2021 | | | |
| CHECKED-REVIEWED | | C. SICHAK | | C. SICHAK | | 6/2021 | | | |
| DESIGNS-DETAILED | | | | | | | | P.E. NUMBER | |
| DESIGNS-DETAILED | | | | | | | | DATE | |
| REVISIONS 1 | | | | | | | | | |
| REVISIONS 2 | | | | | | | | | |
| REVISIONS 3 | | | | | | | | | |
| REVISIONS 4 | | | | | | | | | |
| FIELD CHANGES | | | | | | | | | |

Date:6/29/2021

Username: LindoT

Division: BRIDGE

Filename: ... \Bridge\MSTA\041_xsect-4-26.dgn



Construction #4 (Philbrick Rd)



| |
|------------------------------|
| STATE OF MAINE |
| DEPARTMENT OF TRANSPORTATION |
| 2229600 |
| WIN 22296.00 |
| BRIDGE NO. 2273 |
| BRIDGE PLANS |

| | | | |
|------------------|------------------|--------|--------|
| PROJ. MANAGER | CHECKED-DETAILED | BY | DATE |
| MICHAEL WIGHT | MYLENE R. PARKER | 6/2021 | 6/2021 |
| CHECKED-REVIEWED | C. SICHAK | | |
| DESIGNS-DETAILED | | | |
| DESIGNS-REVIEWED | | | |
| REVISIONS 1 | | | |
| REVISIONS 2 | | | |
| REVISIONS 3 | | | |
| REVISIONS 4 | | | |
| FIELD CHANGES | | | |
| SIGNATURE | P.E. NUMBER | DATE | |

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|---|
| FARMINGTON FALLS BRIDGE |
| SANDY RIVER |
| CHESTERVILLE-FARMINGTON FRANKLIN COUNTY |
| PHILBRICK ROAD |
| CROSS SECTIONS |
| 2+25.00 |
| 2+50.00 |

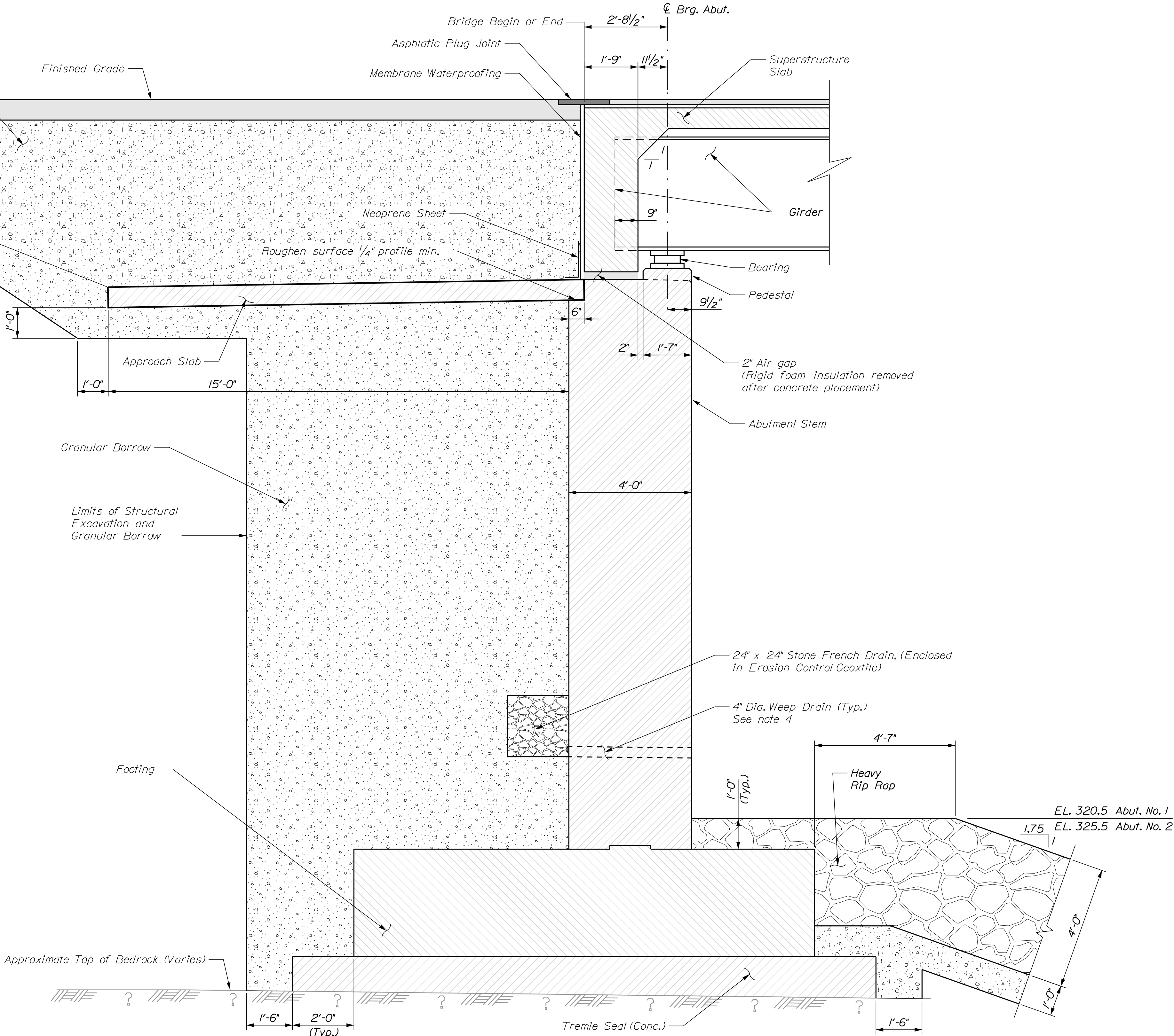
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| SHEET NUMBER |
| 41 |
| OF 76 |

ABUTMENT NOTES

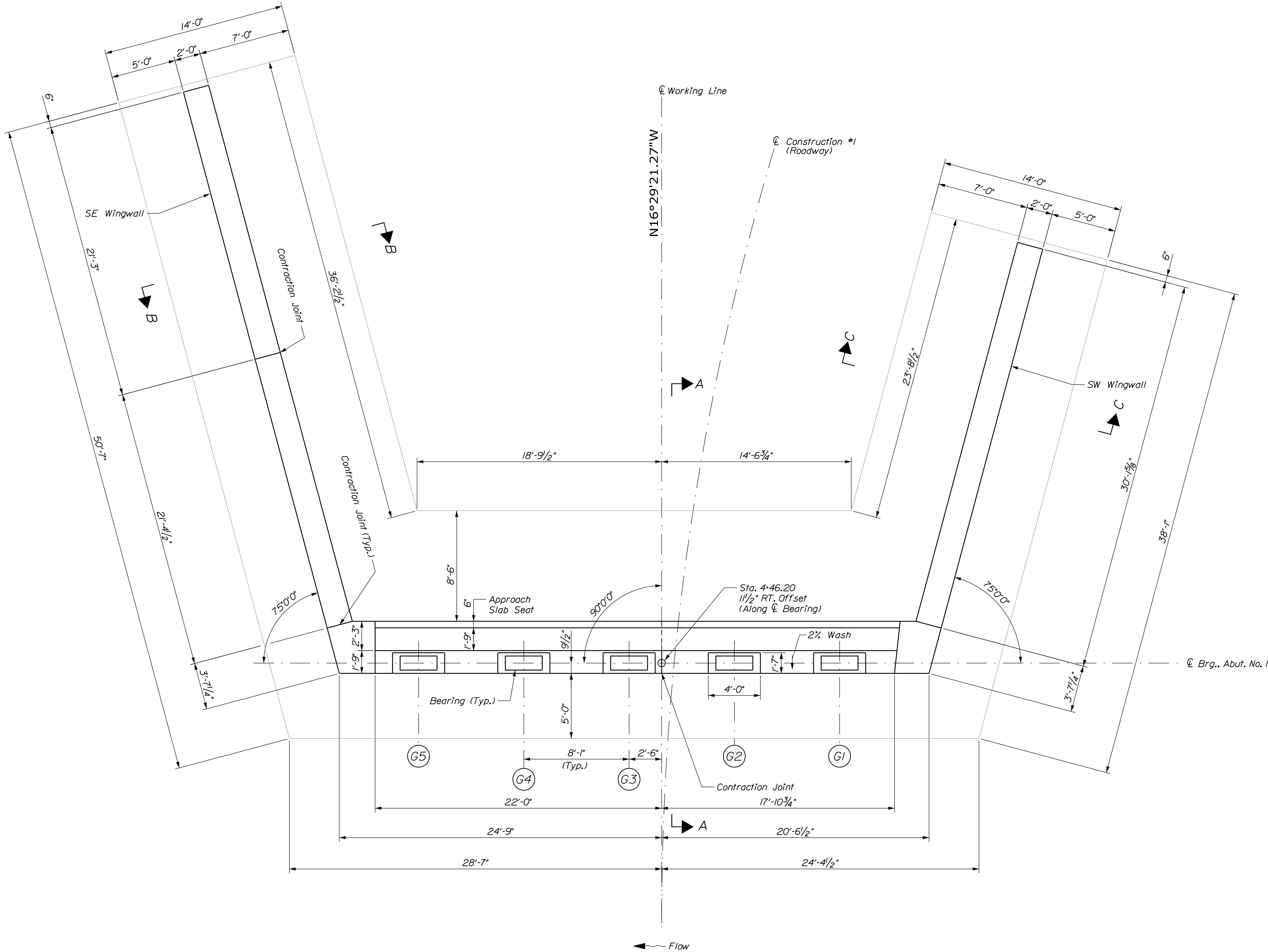
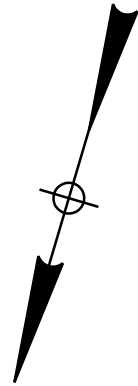
1. The maximum factored applied footing pressure is 9.2 ksf at the Strength Limit State.
2. Structural Earth Excavation required more than 12 inches below the bottom of the structure will be paid for in accordance with Standard Specifications Section 206, Structural Excavation.
3. Abutments, wingwalls, and their footings shall be backfilled with Granular Borrow. Pay limits will be the structural excavation limits in cut areas and a vertical plane located 10 feet behind the walls in fill areas.
4. Reinforcing steel shall have a minimum concrete cover of 2 inches in the walls and 3 inches in the footings unless otherwise noted.
5. Place drains with a 4-inch diameter in the breastwall and wingwalls at 10 feet maximum spacing. The exact location will be determined by the Resident.
6. Cover joints where waterstops are not required in accordance with Standard Details Section 502.
7. When bedrock protrudes above the bottom of footing, the footing may be raised and the vertical reinforcing may be cut in the field with the approval of the Resident. The minimum footing elevations are shown on the Plans and shall not be lowered without prior approval of the Engineer of Record. Payment for adjusting the footing elevations and reinforcing steel will be considered incidental to the related Contract Items. No separate payment will be made.
8. At the option of the Resident, bedrock which protrudes above a horizontal plane 12 inches below the proposed bottom of footing elevation may be removed. Payment for bedrock removal shall be made under Item No. 206.092 Structural Rock Excavation - Major Structures.
9. Abutment/Seal concrete shall be placed on bedrock cleaned of all weathered rock, loose fractured rock and soil. The bedrock subgrade shall be confirmed to be relatively level. Where the bedrock slope exceeds 4H:1V, the bedrock surface shall be benched to create level steps or made completely level. The Resident shall approve the bedrock subgrade prior to the placement of the abutment concrete.
10. Prior to placing abutment concrete, the bedrock subgrade shall be washed with high-pressure water and air.

SEAL COFFERDAM NOTES

1. When sheet piling is used for seal cofferdams, appropriate rolled corners shall be used, and the inside face of the sheet piling shall be at or outside of the seal concrete dimensions shown.
2. The seal concrete placement dimensions shown represent the minimum seal size necessary to meet design requirements and are not based on the use of any particular sheet pile section.
3. The horizontal pay limit for seal concrete will be to the dimensions shown on the plans. No additional payment will be made for concrete placed outside these limits.
4. The assumed 1'-3" depth of the seal is set for a water elevation of 319.00. If the water elevation at the time of construction is higher, the depth of the seal shall be adjusted.
5. Seal concrete above the normal water line shall have a smooth surface. Cofferdams with an irregular inside surface, such as sand bags, shall have formwork for the seal concrete installed inside the cofferdam. Seal formwork will not be paid separately, but will be considered incidental to the appropriate Cofferdam Pay Item.
6. Seal concrete shall be placed on bedrock cleaned of weathered rock, loose fractured bedrock, boulders and soil. Where the bedrock surface slope exceeds 4H:1V, the bedrock surface shall be benched in level steps or made completely level. Cofferdam excavation inspection and reporting shall be in accordance with Standard Specifications Section 511 - Cofferdams. At a minimum, the inspection shall include bedrock elevation measurements and sediment measurements, taken at a minimum of 4 feet evenly distributed plan locations.
7. Each seal shall be cored full depth in at least 4 locations to ensure that the seal is satisfactorily placed. The final core run shall sample the bedrock interface and a minimum of 1 foot of bedrock. Seal core locations will be approved by the Department. Seal concrete cores will be a minimum 3 inch outer diameter and be stored in boxes labeled. In the event that voids or other defects are indicated, the Contractor shall correct the defects in a manner approved by the Department. For each core that reveals a defect, 2 additional cores shall be taken in approximately the same area in locations approved by the Department. All core holes shall be filled using non-shrink grout selected from the MaineDOT Qualified Products List of Grout Materials. The cost of coring and repairs will be considered incidental to related pay items.
8. Seal concrete shall be Class "A".
9. If the Contractor elects to place the seal and footing together, horizontal pay limits for the seal will be the limits of the footing. If placed separately, the horizontal pay limits shall be to the lines of the seal shown on the plans.



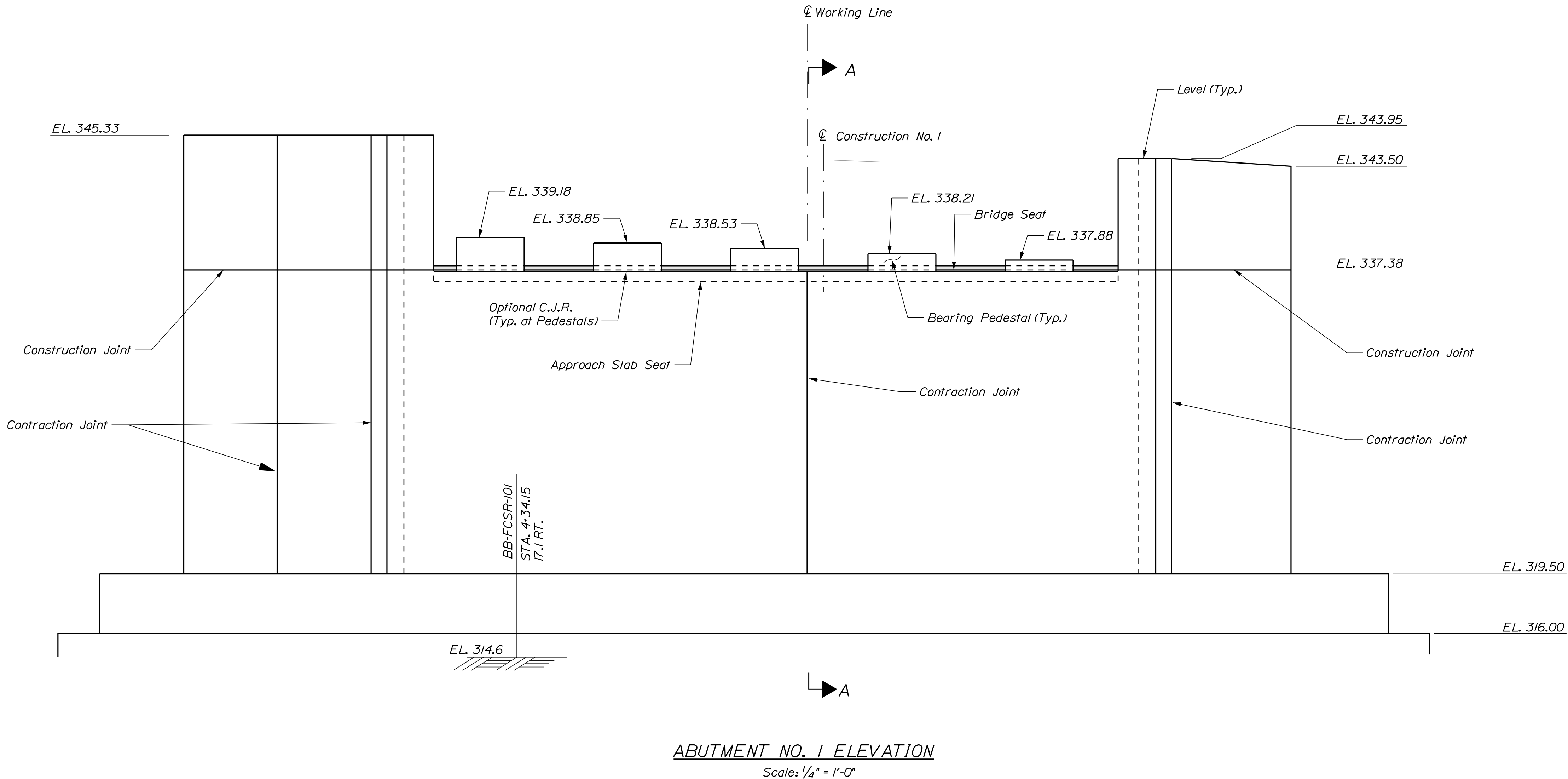
TYPICAL ABUTMENT SECTION



ABUTMENT NO. 1 PLAN
 Scale: 1/4" = 1'-0"



| | | | | | | | |
|---|--|--|---------------|-----------------|--------|--|--|
| STATE OF MAINE DEPARTMENT OF TRANSPORTATION | | <div> <div>2229600</div> <div>WIN</div> <div>22296.00</div> </div> | | BRIDGE NO. 2273 | | BRIDGE PLANS | |
| | | | | | | | |
| FARMINGTON FALLS BRIDGE SANDY RIVER CHESTERVILLE-FARMINGTON FRANKLIN COUNTY | | PROJ. MANAGER | MICHAEL WIGHT | BY | DATE | <div> <div>SIGNATURE</div> <div>P.E. NUMBER</div> <div>DATE</div> </div> | |
| | | DESIGNED | MYLENE | R. PARKER | 6/2021 | | |
| | | CHECKED | REVIEWED | C. SICHAK | 6/2021 | | |
| | | DESIGNED | REVIEWED | C. SICHAK | 6/2021 | | |
| ABUTMENT NO. 1 PLAN | | DESIGNED | REVIEWED | C. SICHAK | 6/2021 | <div> <div>SIGNATURE</div> <div>P.E. NUMBER</div> <div>DATE</div> </div> | |
| | | DESIGNED | REVIEWED | C. SICHAK | 6/2021 | | |
| | | DESIGNED | REVIEWED | C. SICHAK | 6/2021 | | |
| | | DESIGNED | REVIEWED | C. SICHAK | 6/2021 | | |
| SHEET NUMBER | | DESIGNED | REVIEWED | C. SICHAK | 6/2021 | <div> <div>SIGNATURE</div> <div>P.E. NUMBER</div> <div>DATE</div> </div> | |
| | | DESIGNED | REVIEWED | C. SICHAK | 6/2021 | | |
| | | DESIGNED | REVIEWED | C. SICHAK | 6/2021 | | |
| | | DESIGNED | REVIEWED | C. SICHAK | 6/2021 | | |
| 43 | | DESIGNED | REVIEWED | C. SICHAK | 6/2021 | <div> <div>SIGNATURE</div> <div>P.E. NUMBER</div> <div>DATE</div> </div> | |
| | | DESIGNED | REVIEWED | C. SICHAK | 6/2021 | | |
| | | DESIGNED | REVIEWED | C. SICHAK | 6/2021 | | |
| | | DESIGNED | REVIEWED | C. SICHAK | 6/2021 | | |
| OF 76 | | DESIGNED | REVIEWED | C. SICHAK | 6/2021 | <div> <div>SIGNATURE</div> <div>P.E. NUMBER</div> <div>DATE</div> </div> | |
| | | DESIGNED | REVIEWED | C. SICHAK | 6/2021 | | |
| | | DESIGNED | REVIEWED | C. SICHAK | 6/2021 | | |
| | | DESIGNED | REVIEWED | C. SICHAK | 6/2021 | | |



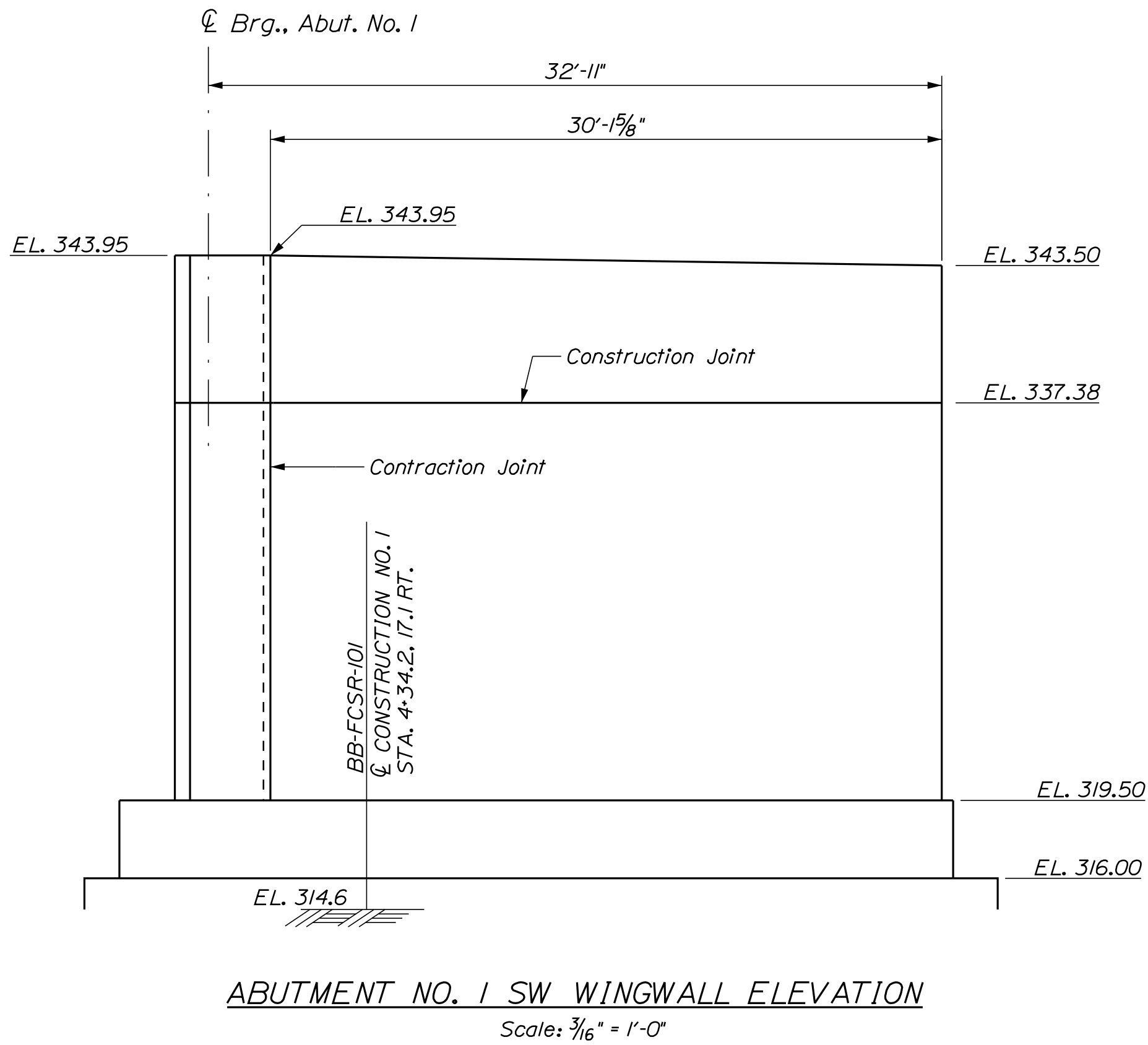
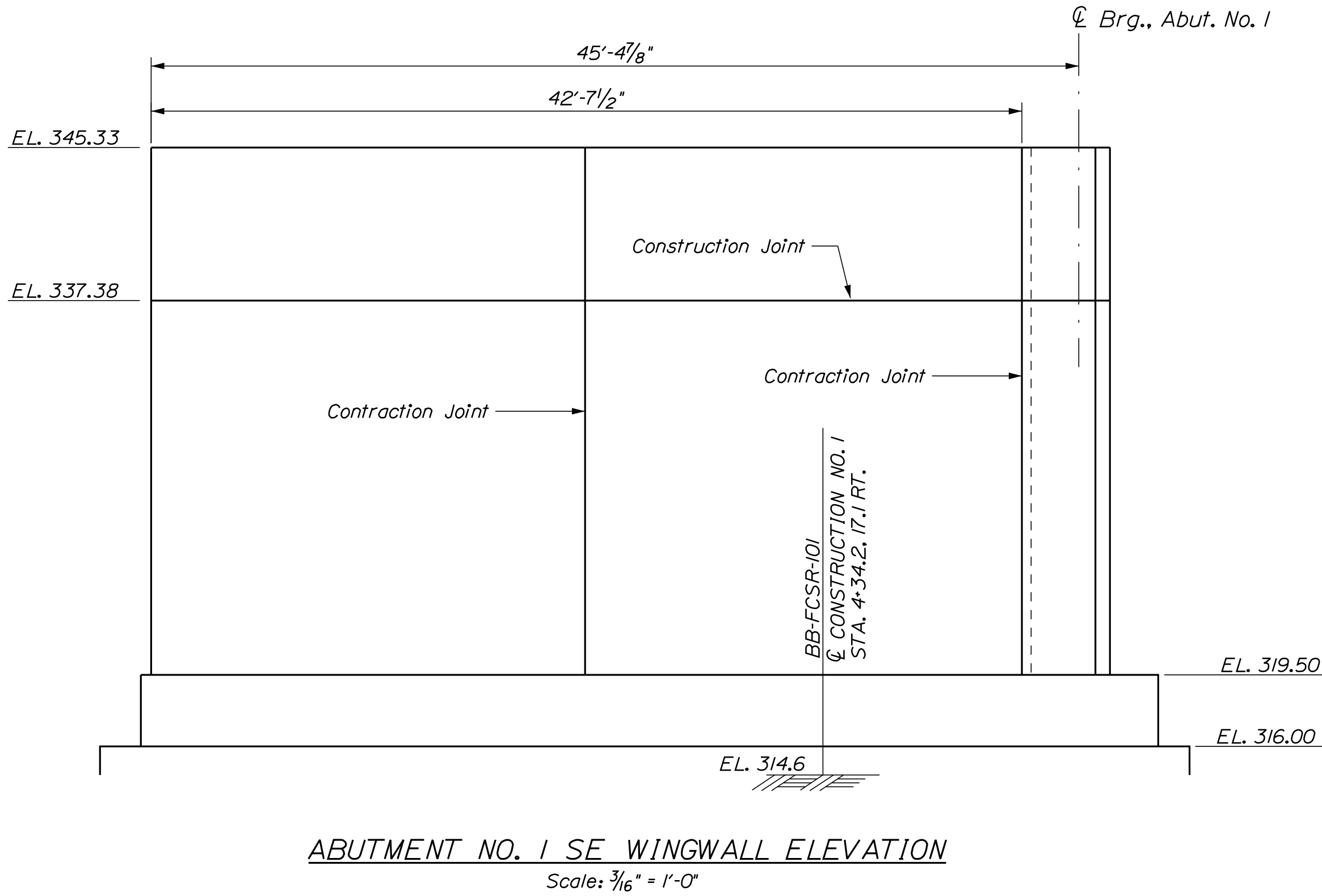
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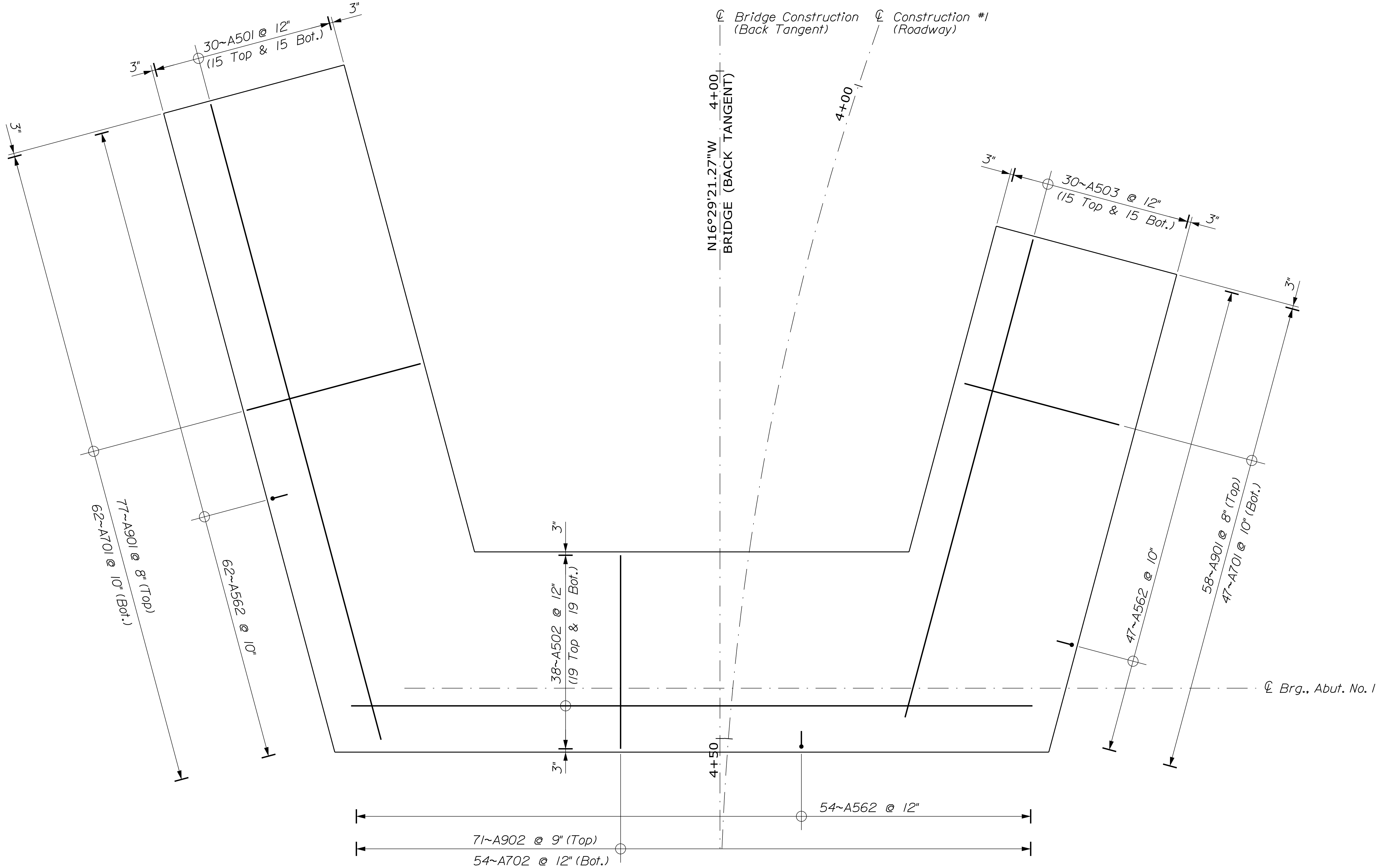
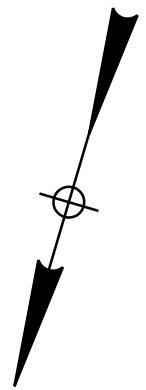
C.J.R. = Construction Joint, Roughen
Surface 1/4" profile min. (Typ.)



| | | | |
|-------------------|---------------|-----------|--------|
| PROJ. MANAGER | MICHAEL WIGHT | BY | DATE |
| DESIGNED-Detailed | MYLENE | R. PARKER | 6/2021 |
| CHECKED-Reviewed | C. SICHAK | C. SICHAK | 6/2021 |
| DESIGNED-Detailed | | | |
| DESIGNED-Detailed | | | |
| REVISIONS 1 | | | |
| REVISIONS 2 | | | |
| REVISIONS 3 | | | |
| REVISIONS 4 | | | |
| FIELD CHANGES | | | |

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| SIGNATURE | P.E. NUMBER | DATE |
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ABUTMENT NO.1 FOOTING REINFORCEMENT PLAN



STATE OF MAINE
DEPARTMENT OF TRANSPORTATION

2229600

BRIDGE NO. 2273
WIN
22296.00

BRIDGE PLANS

FARMINGTON FALLS BRIDGE
SANDY RIVER
CHESTERVILLE-FARMINGTON FRANKLIN COUNTY

ABUTMENT NO.1
FOOTING REINFORCEMENT PLAN

| | | | |
|------------------|---------------|-----------|--------|
| PROJ. MANAGER | MICHAEL WIGHT | BY | DATE |
| DESIGN-DETAILED | MYLENE | R. PARKER | 6/2021 |
| CHECKED-REVIEWED | C. SICHAK | C. SICHAK | 6/2021 |
| DESIGN-DETAILED | | | |
| DESIGN-DETAILED | | | |
| REVISIONS 1 | | | |
| REVISIONS 2 | | | |
| REVISIONS 3 | | | |
| REVISIONS 4 | | | |
| FIELD CHANGES | | | |

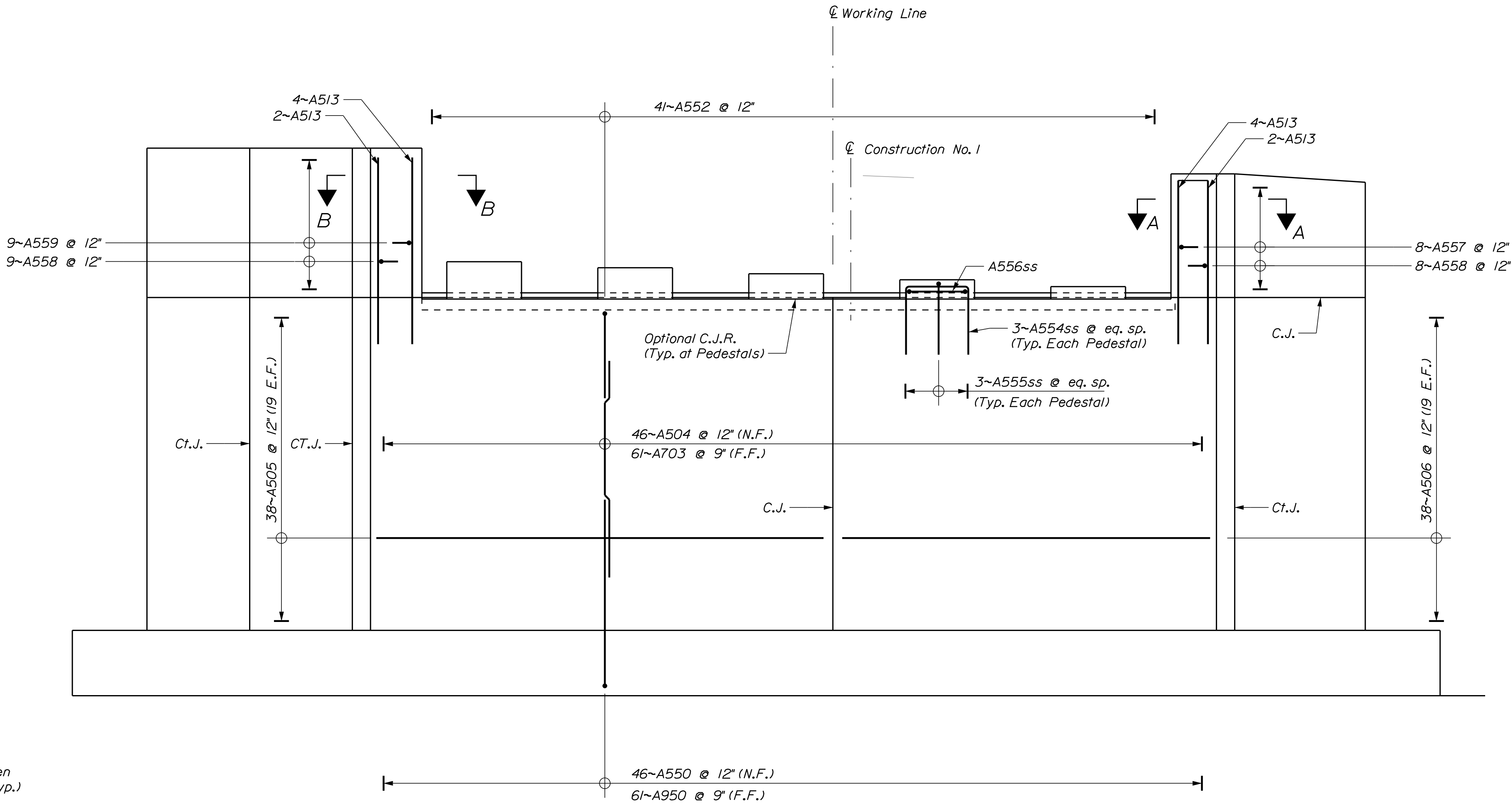
SIGNATURE
P.E. NUMBER
DATE

SHEET NUMBER

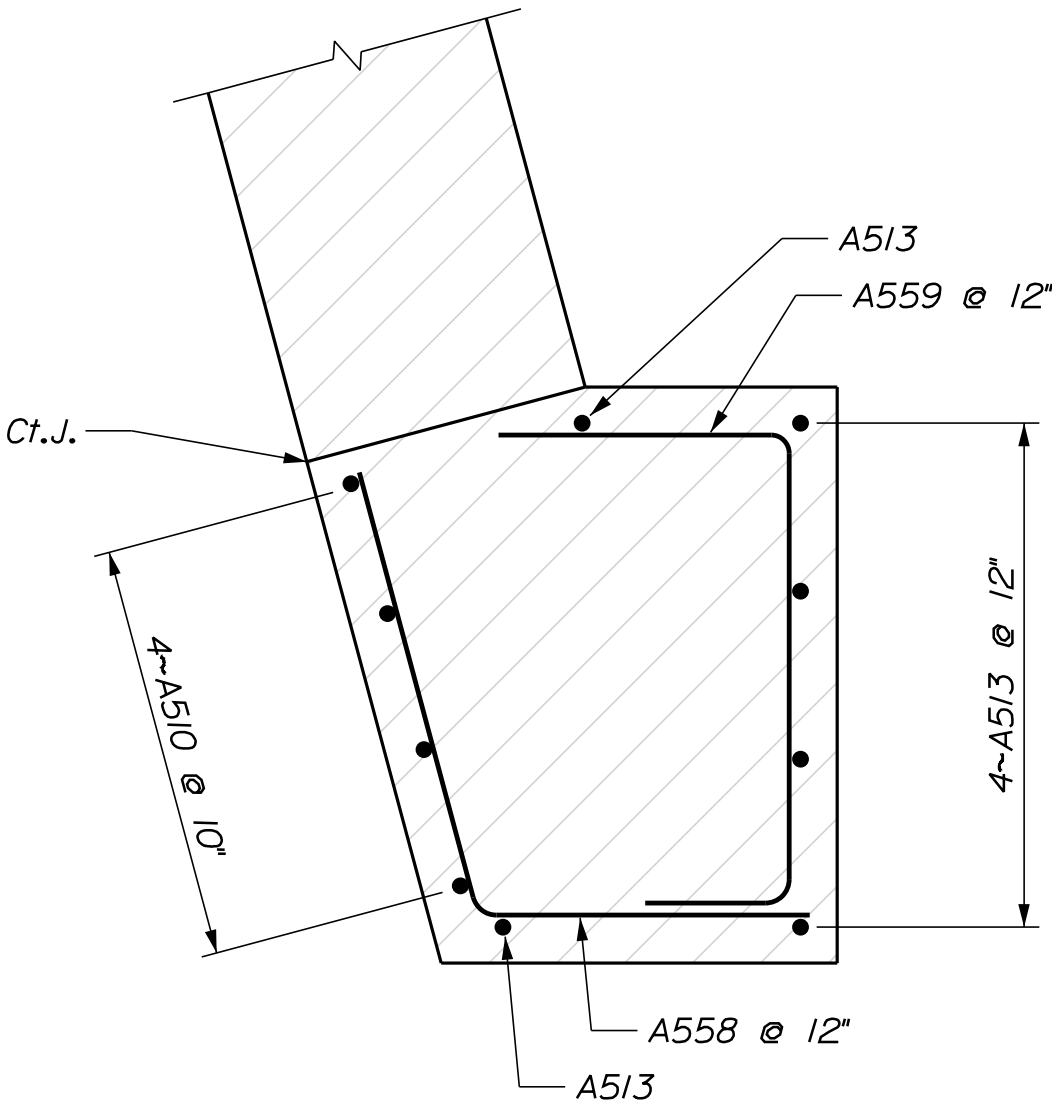
46

OF 76

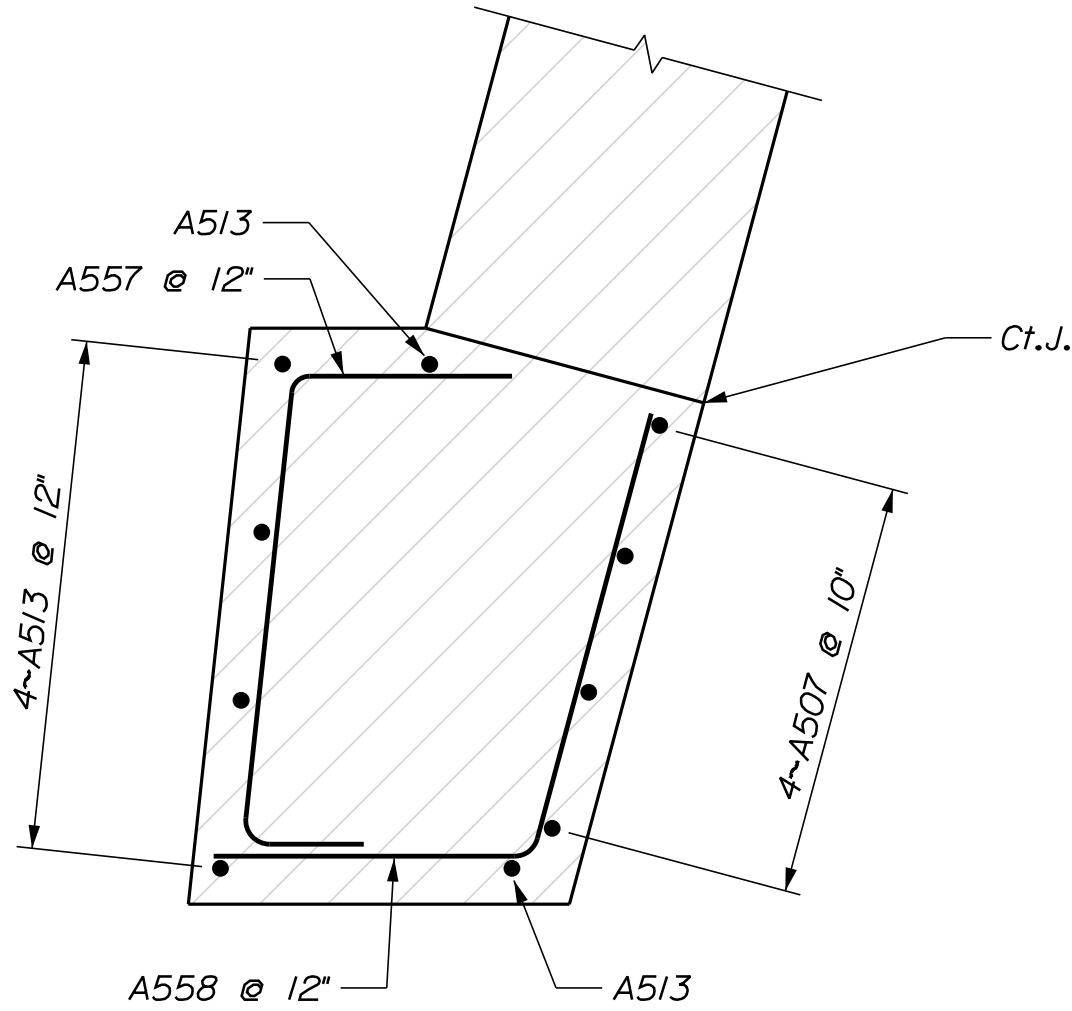
LEGEND:
C.J. = Construction Joint
Ct.J. = Contraction Joint
C.J.R. = Construction Joint, Roughen
Surface 1/4" profile min. (Typ.)



ABUTMENT NO. 1 REINFORCEMENT ELEVATION
Scale: 1/4" = 1'-0"

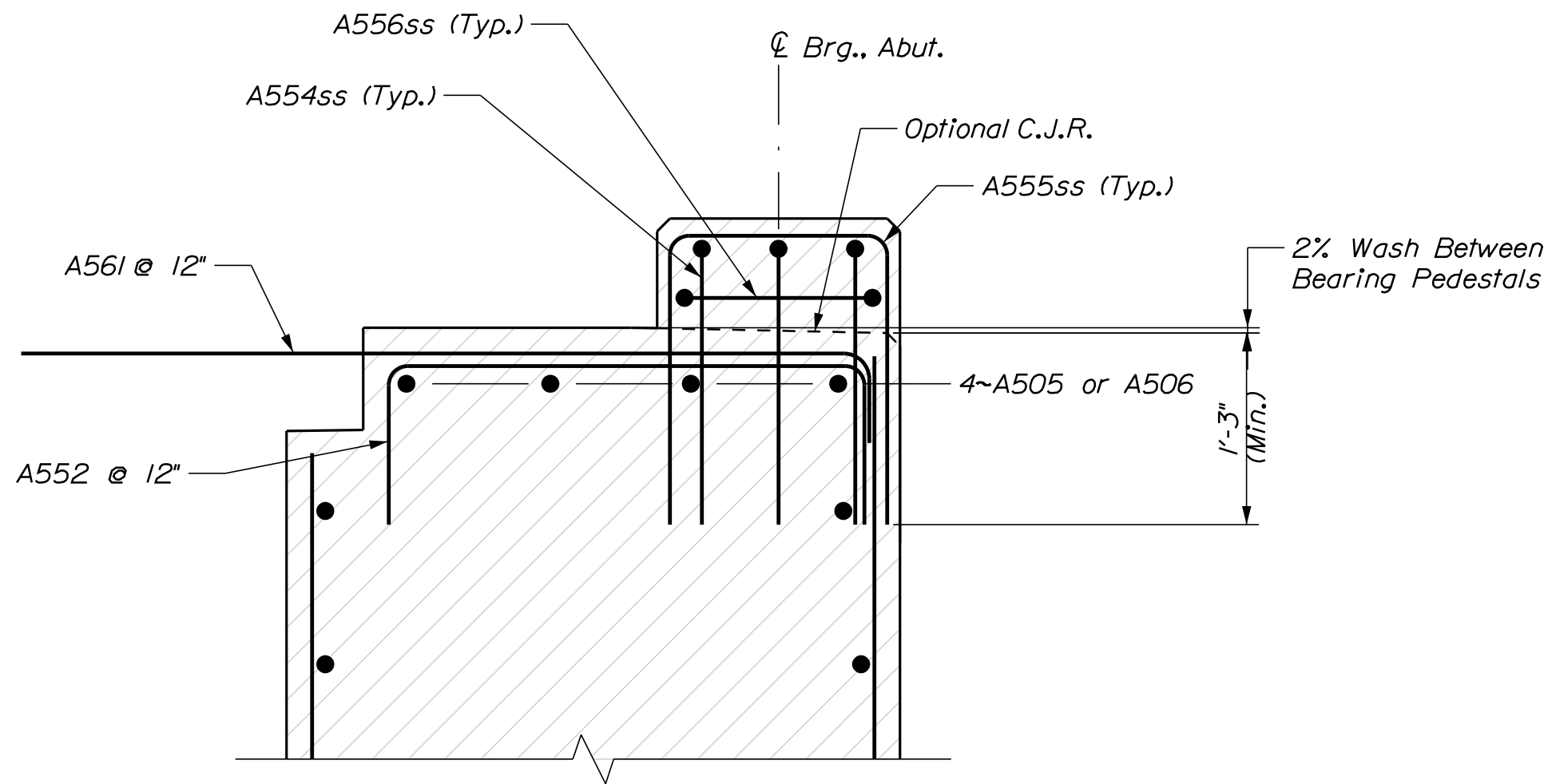


PARTIAL SECTION B-B

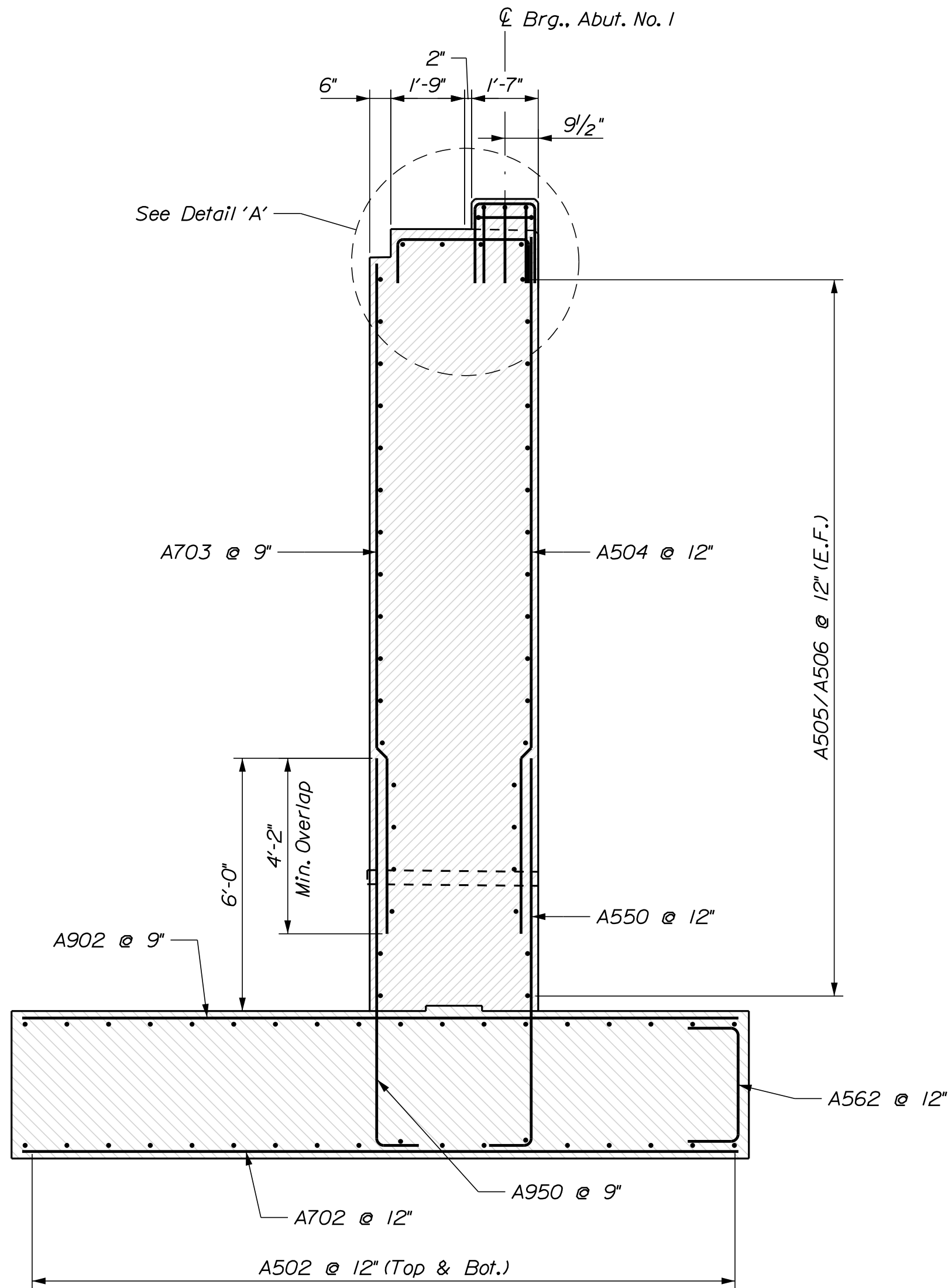


PARTIAL SECTION A-A

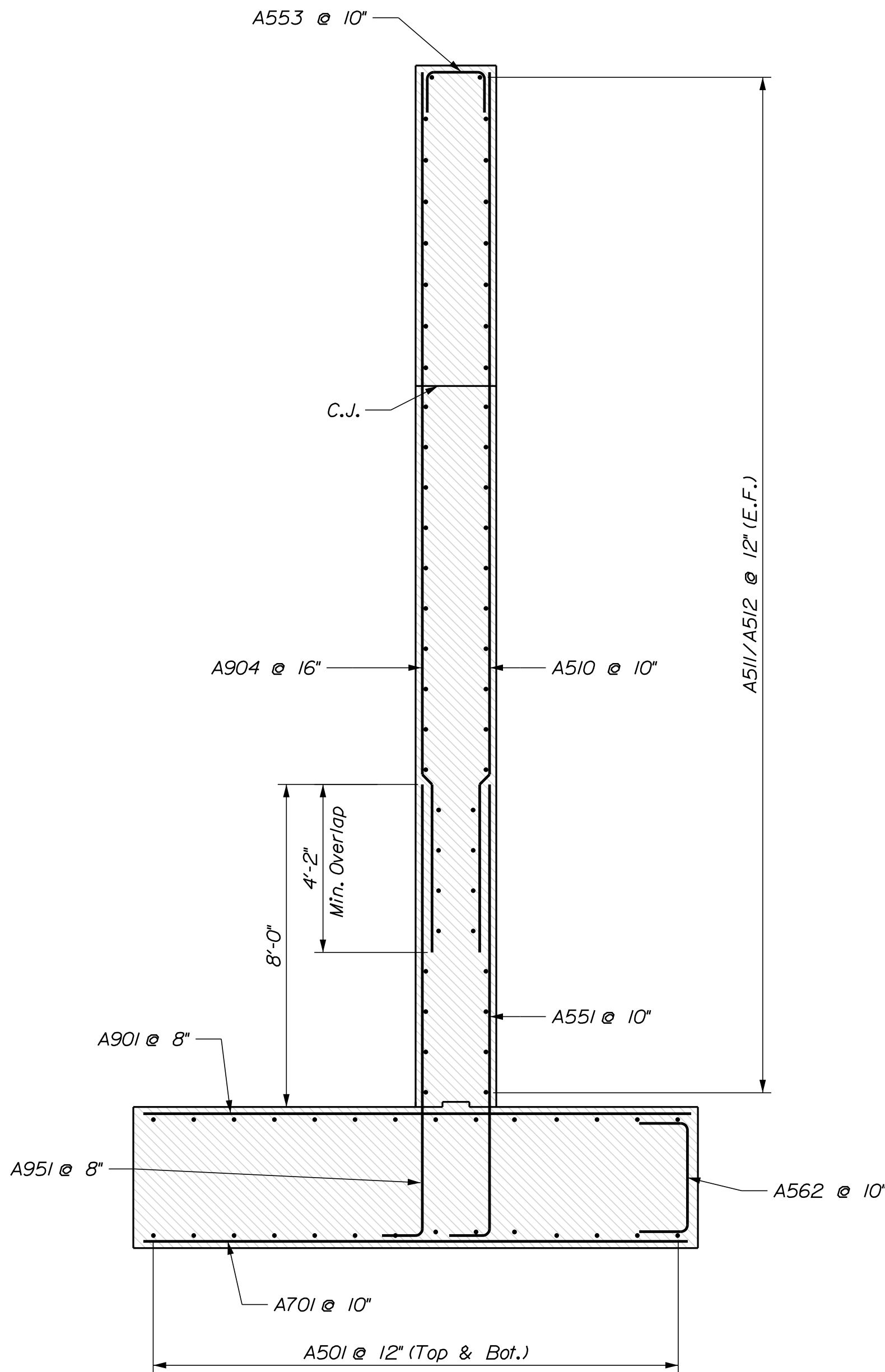
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| STATE OF MAINE | | DEPARTMENT OF TRANSPORTATION | | 2229600 | | WIN | | BRIDGE NO. 2273 | | 22296.00 | | BRIDGE PLANS | |
| FARMINGTON FALLS BRIDGE | | SANDY RIVER | | CHESTERVILLE-FARMINGTON FRANKLIN COUNTY | | ABUTMENT NO. 1 | | REINFORCEMENT ELEVATION | | SHEET NUMBER | | 47 | |
| PROJ. MANAGER | | MICHAEL WIGHT | | BY | | DATE | | SIGNATURE | | P.E. NUMBER | | DATE | |
| DESIGN-DETAILED | | MYLENE | | R. PARKER | | 6/2021 | | | | | | | |
| CHECKED-REVIEWED | | C. SICHAK | | C. SICHAK | | 6/2021 | | | | | | | |
| DESIGNS DETAILLED | | | | | | | | | | | | | |
| DESIGNS DETAILLED | | | | | | | | | | | | | |
| REVISIONS 1 | | | | | | | | | | | | | |
| REVISIONS 2 | | | | | | | | | | | | | |
| REVISIONS 3 | | | | | | | | | | | | | |
| REVISIONS 4 | | | | | | | | | | | | | |
| FIELD CHANGES | | | | | | | | | | | | | |



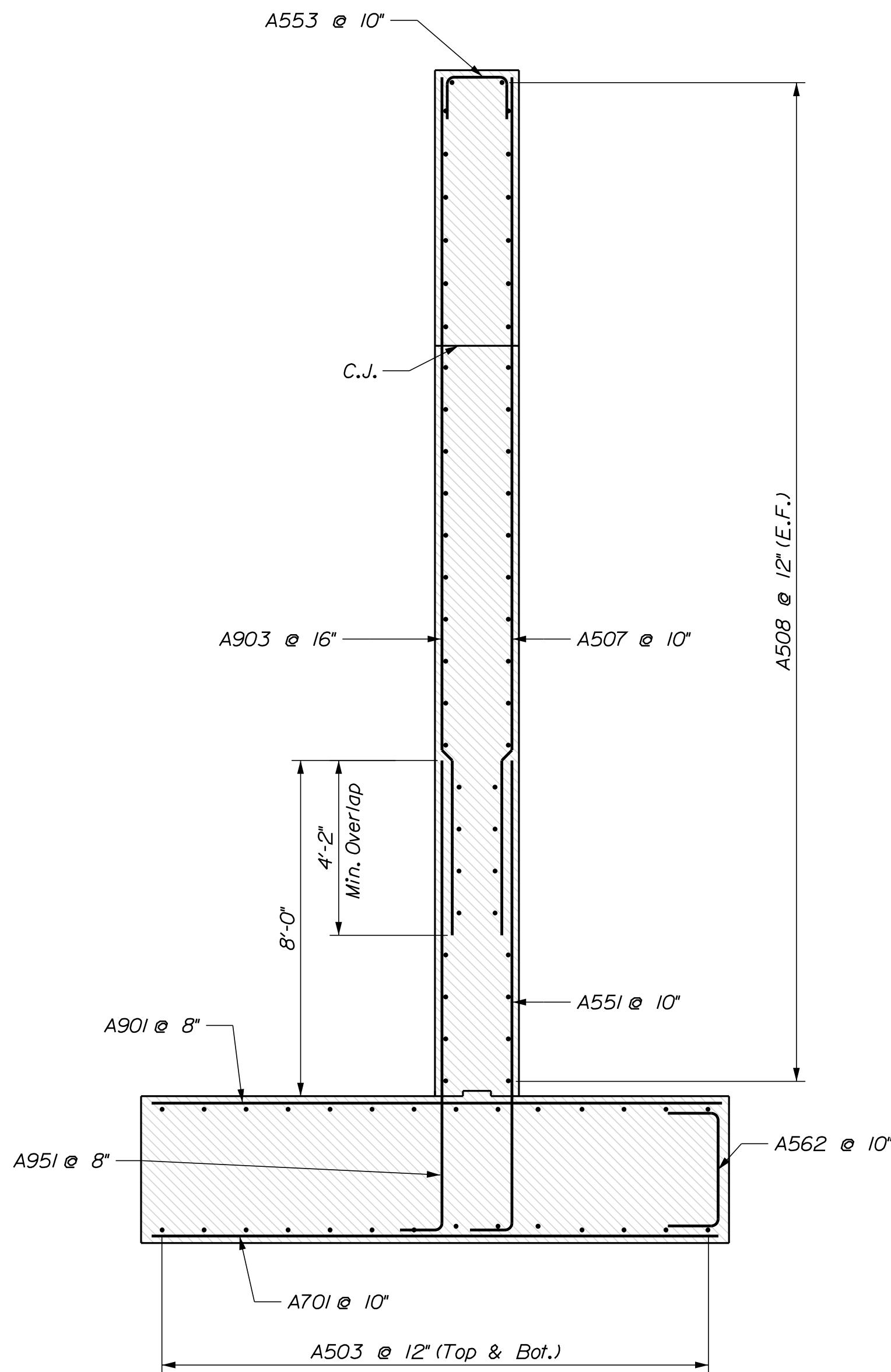
DETAIL 'A'



TYPICAL ABUTMENT SECTION A-A



TYPICAL SE WINGWALL SECTION B-B



TYPICAL SW WINGWALL SECTION C-C

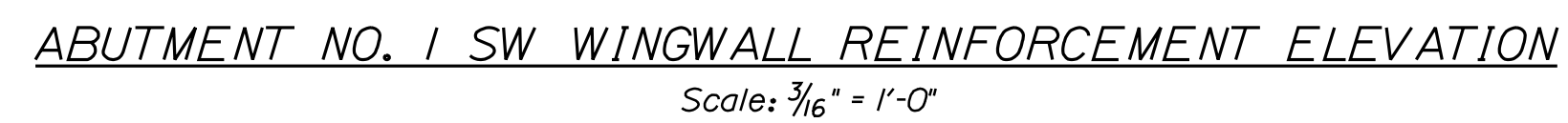
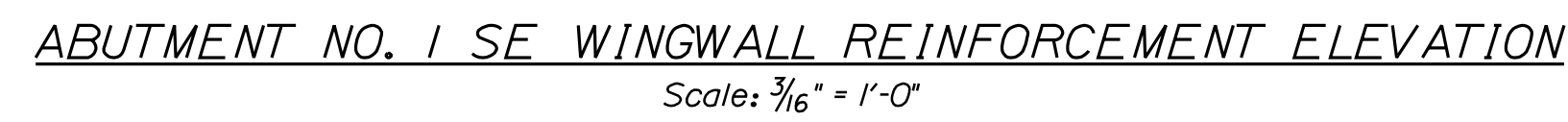
LEGEND:

C.J.R. = Construction Joint, Roughen Surface 1/4" profile min. (Typ.)

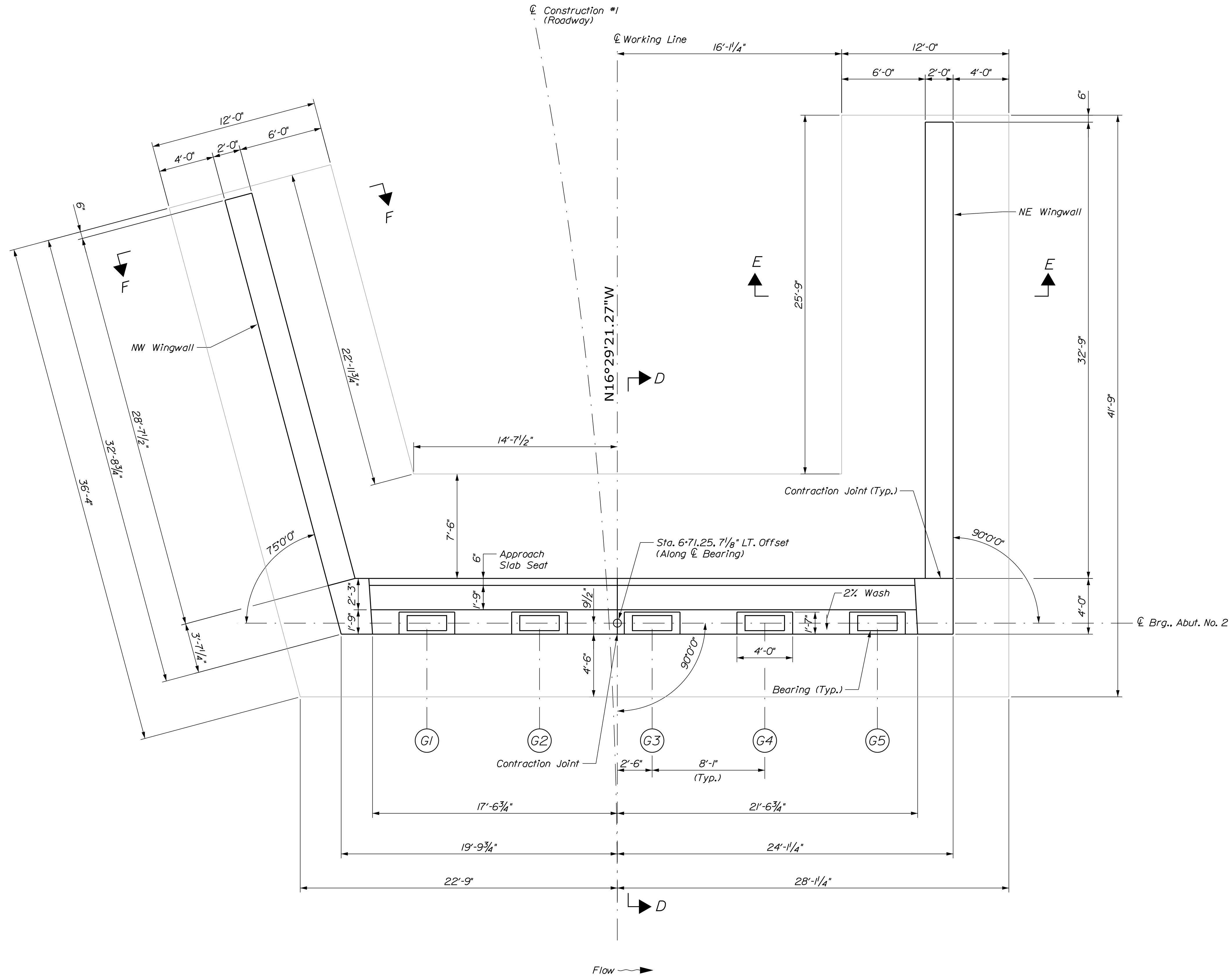


| PROJ. MANAGER | MICHAEL WIGHT | BY | DATE |
|-------------------|---------------|-----------|--------|
| CHECKED-DETAILED | MYLENE | R. PARKER | 6/2021 |
| CHECKED-REVIEWED | C. SICHAK | C. SICHAK | 6/2021 |
| DESIGNS DETAILLED | | | |
| DESIGNS 1 | | | |
| REVISIONS 2 | | | |
| REVISIONS 3 | | | |
| REVISIONS 4 | | | |
| FIELD CHANGES | | | |

| SIGNATURE | P.E. NUMBER | DATE |
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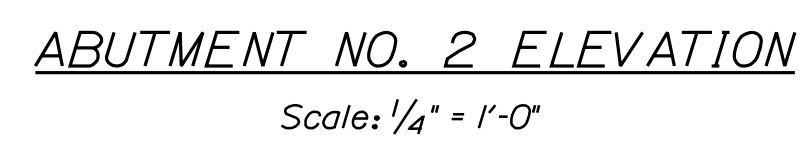


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Scale: $1/4" = 1'-0"$

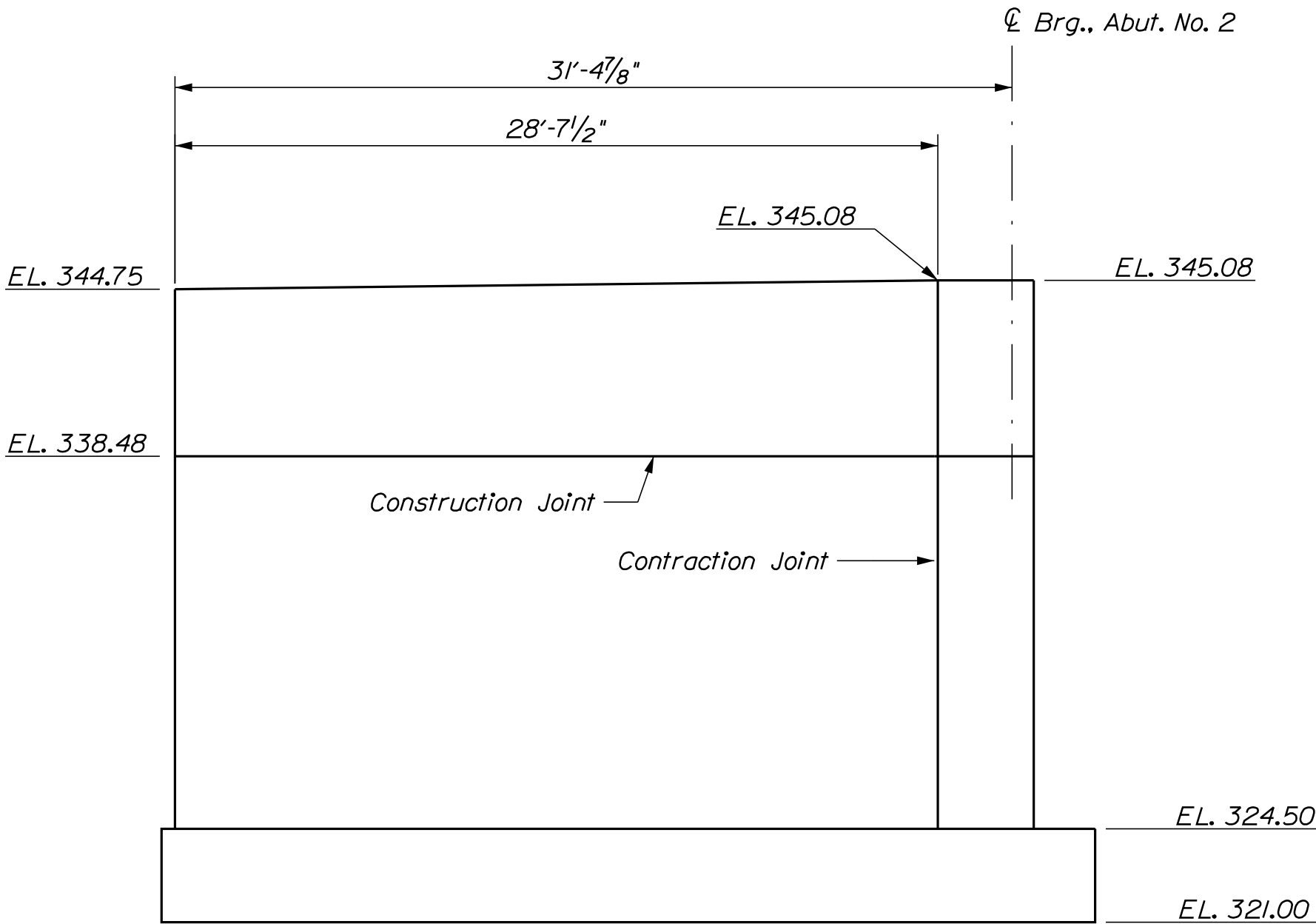
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| FARMINGTON FALLS BRIDGE SANDY RIVER CHESTERVILLE-FARMINGTON FRANKLIN COUNTY ABUTMENT NO. 2 PLAN | PROJ. MANAGER | MICHAEL WIGHT | BY | DATE |
| | DESIGN-DETAILED | MYLENE NB | R. PARKER | 6/20/21 |
| | CHECKED-REVIEWED | C. SICHAK | C. SICHAK | 6/20/21 |
| | DESIGN2-DETAILED2 | | | SIGNATURE |
| | DESIGN3-DETAILED3 | | | P.E. NUMBER |
| | REVISIONS 1 | | | |
| | REVISIONS 2 | | | |
| | REVISIONS 3 | | | |
| | REVISIONS 4 | | | DATE |
| | FIELD CHANGES | | | |
| STATE OF MAINE DEPARTMENT OF TRANSPORTATION 2229600 | | | | BRIDGE NO. 2273 WIN 22296.00 BRIDGE PLANS |



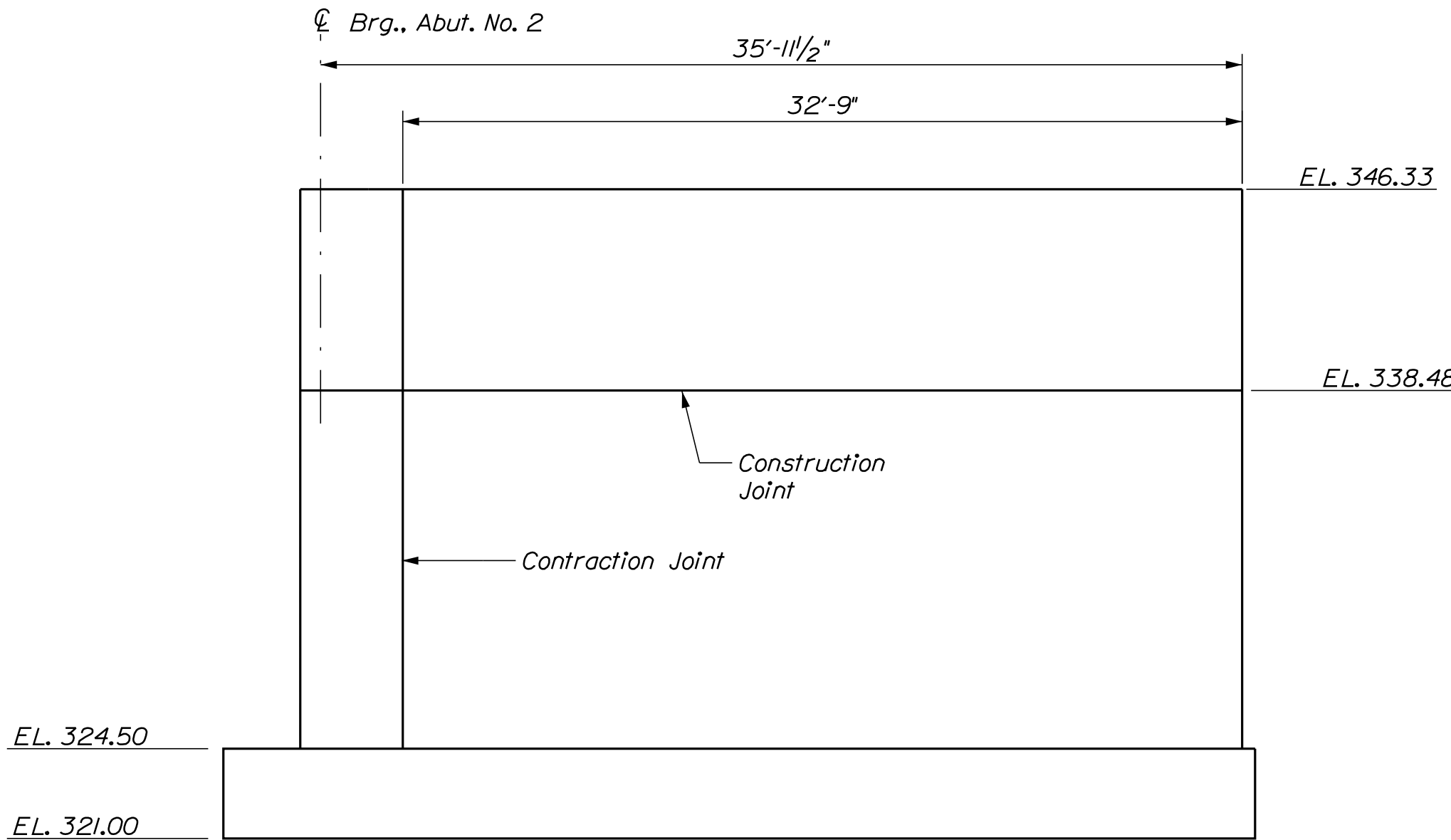
C.J.R. = Construction Joint, Roughen
Surface 1/4" profile min. (Typ.)

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| STATE OF MAINE | |
| DEPARTMENT OF TRANSPORTATION | |
| 2229600 | |
| BRIDGE NO. 2273 | WIN 22296.00 |
| BRIDGE PLANS | |

BB-FCSR-201
CL CONSTRUCTION NO. 1
STA. 6+60.4, 2.8 RT.
EL. 321.1



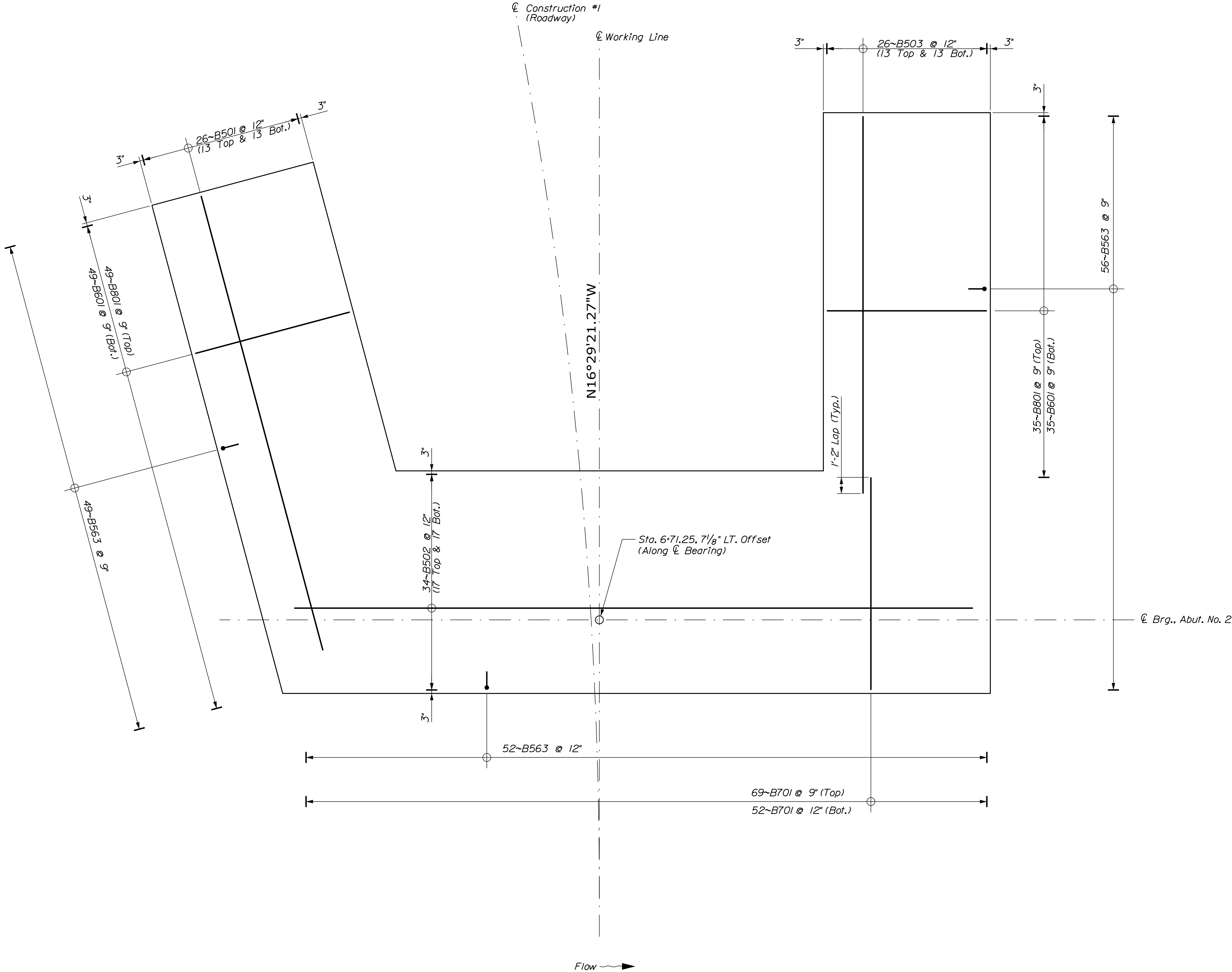
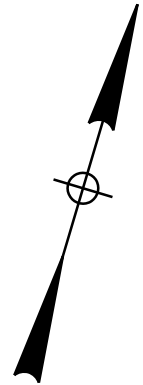
ABUTMENT NO. 2 NW WINGWALL ELEVATION
Scale: 3/16" = 1'-0"



ABUTMENT NO. 2 NE WINGWALL ELEVATION
Scale: 3/16" = 1'-0"

BB-FCSR-201
CL CONSTRUCTION NO. 1
STA. 6+60.4, 2.8 RT.
EL. 321.1

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|--|--|--|--|--|--|--|--|--|--|------------------|--|----------------|-----------|--------|-----------------|--|------------------------------|--|----------|--|--------------|--|--|--|--|--|--|
| FARMINGTON FALLS BRIDGE | | | | | | | | | | PROJ. MANAGER | | MICHAEL WRIGHT | BY | DATE | STATE OF MAINE | | | | | | | | | | | | |
| SANDY RIVER | | | | | | | | | | DESIGN-DETAILED | | MYLENE MB | R. PARKER | 6/2021 | | | DEPARTMENT OF TRANSPORTATION | | | | | | | | | | |
| CHESTERVILLE-FARMINGTON FRANKLIN COUNTY | | | | | | | | | | CHECKED-REVIEWED | | C. SICHAK | C. SICHAK | 6/2021 | SIGNATURE | | | | | | | | | | | | |
| ABUTMENT NO.2 WINGWALL PLANS AND ELEVATIONS | | | | | | | | | | DESIGN-DETAILED2 | | | | | | | P.E. NUMBER | | | | | | | | | | |
| | | | | | | | | | | DESIGN-DETAILED3 | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | REVISIONS 1 | | | | | | | | | | | | | | | | | |
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| | | | | | | | | | | REVISIONS 3 | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | REVISIONS 4 | | | | | DATE | | | | | | | | | | | | |
| | | | | | | | | | | FIELD CHANGES | | | | | | | | | | | | | | | | | |
| SHEET NUMBER | | | | | | | | | | | | | | | BRIDGE NO. 2273 | | WIN | | 22296.00 | | BRIDGE PLANS | | | | | | |
| 52 | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| OF 76 | | | | | | | | | | | | | | | | | | | | | | | | | | | |

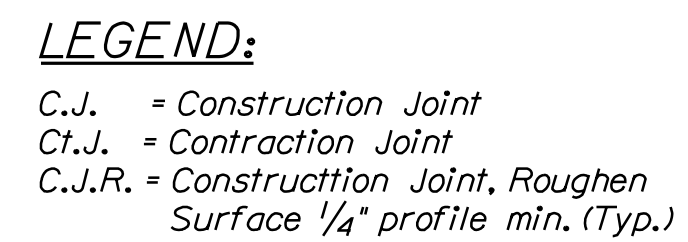


ABUTMENT NO. 2 FOOTING REINFORCEMENT PLAN

Scale: 1/4" = 1'-0"



| | | | | | | | | | |
|---|--|--|--|-------------------|---------------|-----------|--------|---|--|
| FARMINGTON FALLS BRIDGE SANDY RIVER CHESTERVILLE-FARMINGTON FRANKLIN COUNTY | | | | PROJ. MANAGER | MICHAEL WIGHT | BY | DATE | STATE OF MAINE DEPARTMENT OF TRANSPORTATION 2229600 WIN 22296.00 BRIDGE NO. 2273 BRIDGE PLANS | |
| ABUTMENT NO. 2 FOOTING REINFORCEMENT PLAN | | | | DESIGN-DETAILED | MYLENE MB | R. PARKER | | | |
| | | | | CHECKED-REVIEWED | C. SICHAK | C. SICHAK | 6/2021 | | |
| | | | | DESIGN2-DETAILED2 | | | | | |
| | | | | DESIGN3-DETAILED3 | | | | | |
| | | | | REVISIONS 1 | | | | P.E. NUMBER | |
| | | | | REVISIONS 2 | | | | | |
| | | | | REVISIONS 3 | | | | | |
| | | | | REVISIONS 4 | | | | DATE | |
| | | | | FIELD CHANGES | | | | | |
| SHEET NUMBER | | | | | | | | | |
| 53 | | | | | | | | | |
| OF 76 | | | | | | | | | |



| | | | | | | | | | |
|---|--|--|--|-------------------|---------------|-----------|--------|--|-----------------|
| FARMINGTON FALLS BRIDGE SANDY RIVER CHESTERVILLE-FARMINGTON FRANKLIN COUNTY | | | | PROJ. MANAGER | MICHAEL WIGHT | BY | DATE | STATE OF MAINE DEPARTMENT OF TRANSPORTATION | |
| ABUTMENT NO. 2 REINFORCEMENT ELEVATION | | | | DESIGN-DETAILED | MYEVE NB | R. PARKER | 6/2021 | | 2229600 |
| | | | | CHECKED-REVIEWED | C. SICHAK | C. SICHAK | 6/2021 | SIGNATURE | |
| | | | | DESIGN2-DETAILED2 | | | | | |
| | | | | DESIGN3-DETAILED3 | | | | P.E. NUMBER | |
| | | | | REVISIONS 1 | | | | | |
| | | | | REVISIONS 2 | | | | | WIN 22296.00 |
| | | | | REVISIONS 3 | | | | | |
| | | | | REVISIONS 4 | | | | DATE | |
| | | | | FIELD CHANGES | | | | | |
| | | | | | | | | BRIDGE NO. 2273 | BRIDGE PLANS |

B556ss (Typ.)

B554ss (Typ.)

B555ss (Typ.)

B562 @ 12"

B552 @ 12"

C Brg., Abut.

Optional C.J.R.

2% Wash Between Bearing Pedestals

1'-3" (Min.)

4-B505 or B506

DETAIL 'A'

The diagram illustrates a typical abutment section, labeled 'TYPICAL ABUTMENT SECTION D-D'. It shows a cross-section of a bridge abutment with various reinforcement details and dimensions. The main structure is a vertical wall with a base. The reinforcement includes:

- Top Reinforcement:** Brg., Abut. No. 1 (Bridge Abutment No. 1) is shown at the top. Dimensions include 6" for the top width, 2" for the top reinforcement width, 1'-9" for the top reinforcement length, 1'-7" for the top reinforcement length, and 9 1/2" for the top reinforcement length.
- Vertical Wall Reinforcement:** B504 @ 12" (vertical bars), B602 @ 9" (horizontal bars), B505/B506 @ 12" (E.F.) (vertical bars), B551 @ 12" (vertical bars), and B563 @ 12" (vertical bars).
- Base Reinforcement:** B701 @ 9" (horizontal bars), B851 @ 9" (horizontal bars), B701 @ 12" (horizontal bars), and B502 @ 12" (Top & Bot.) (horizontal bars).
- Dimensions:** 6'-0" (total height), 4'-2" (height of the main wall), and Min. Overlap (minimum overlap).
- Other Details:** See Detail 'A' (reference to another detail), 9 1/2" (dimension), and 12" (dimension).

The diagram illustrates a typical NE Wingwall Section E-E. It shows a vertical wall section with a horizontal base. The reinforcement details are as follows:

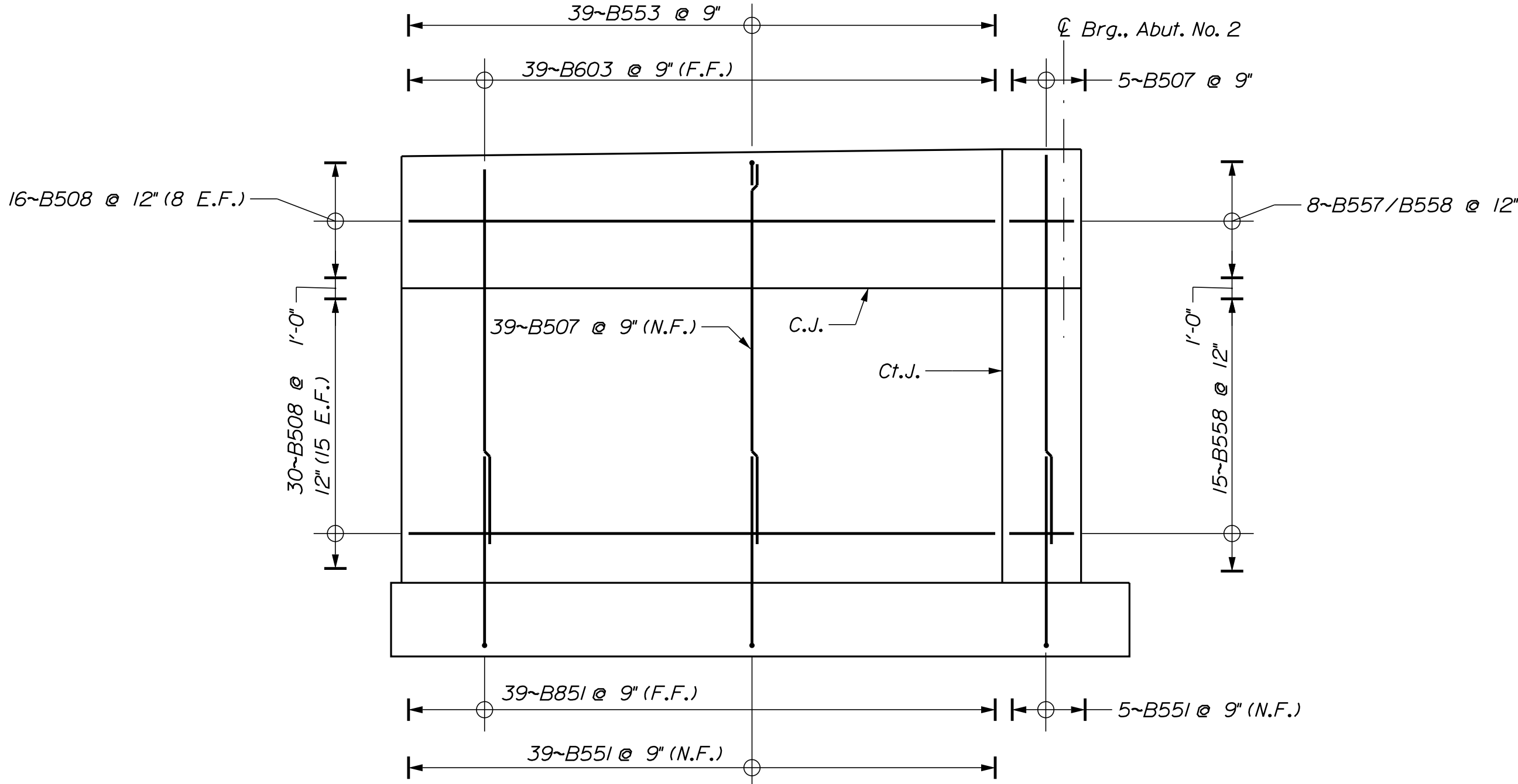
- Top Reinforcement:** B553 @ 9" (top longitudinal bar).
- Vertical Wall Reinforcement:**
 - B603 @ 9" (left vertical bars).
 - B510 @ 9" (right vertical bars).
 - B511 @ 12" (E.F.) (right vertical bars, extending full height).
- Base Reinforcement:**
 - B801 @ 9" (left horizontal bars).
 - B851 @ 9" (left horizontal bars).
 - B601 @ 9" (bottom horizontal bars).
 - B503 @ 12" (Top & Bot.) (bottom horizontal bars).
 - B563 @ 9" (right horizontal bars).
- Other Details:**
 - C.J. (Centerline) is indicated on the vertical wall.
 - Min. Overlap is shown for the vertical bars.
 - Dimensions: 6'-0" for the vertical wall height, 4'-2" for the minimum overlap.

TYPICAL NE WINGWALL SECTION E-E

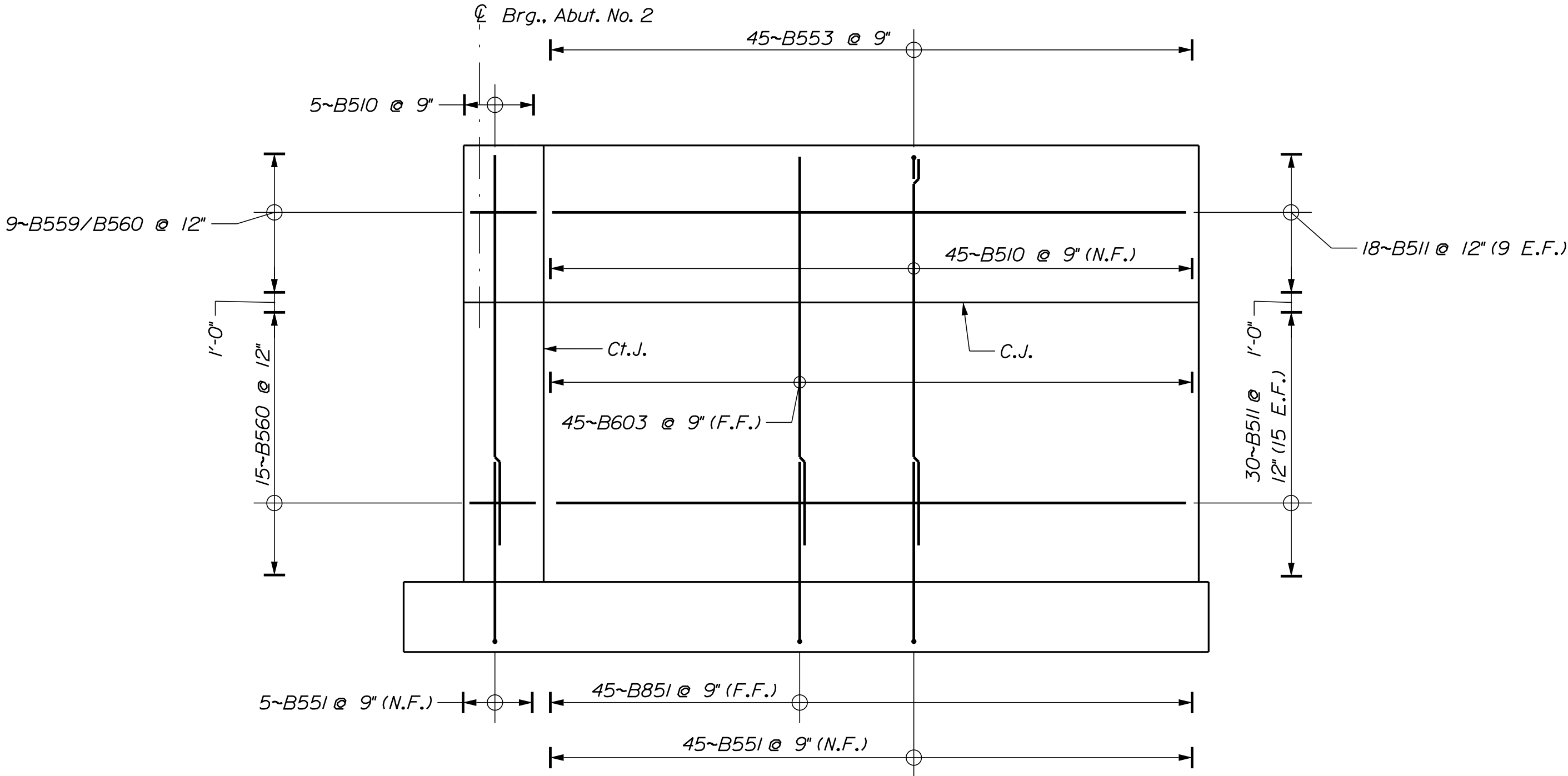
The diagram illustrates the cross-section of a typical NW wingwall, labeled 'TYPICAL NW WINGWALL SECTION F-F'. The structure is an L-shaped concrete member. The vertical stem is reinforced with B553 bars at 9 inches on the top face and B507 bars at 9 inches on the side face. The horizontal base is reinforced with B801 bars at 9 inches on the top face, B601 bars at 9 inches on the bottom face, and B563 bars at 9 inches on the side face. A central vertical core is reinforced with B603 bars at 9 inches on the left and B551 bars at 9 inches on the right. A 'C.J.' (Centerline) is indicated at the top of the stem. Vertical dimensions include a total height of 6'-0" and a section height of 4'-2" with a 'Min. Overlap' requirement. Horizontal dimensions include a base width of 6'-0" and a reinforcement spacing of B501 at 12 inches (Top & Bot.). A vertical dimension of B508 at 12 inches (E.F.) is also shown on the right side.

LEGEND:

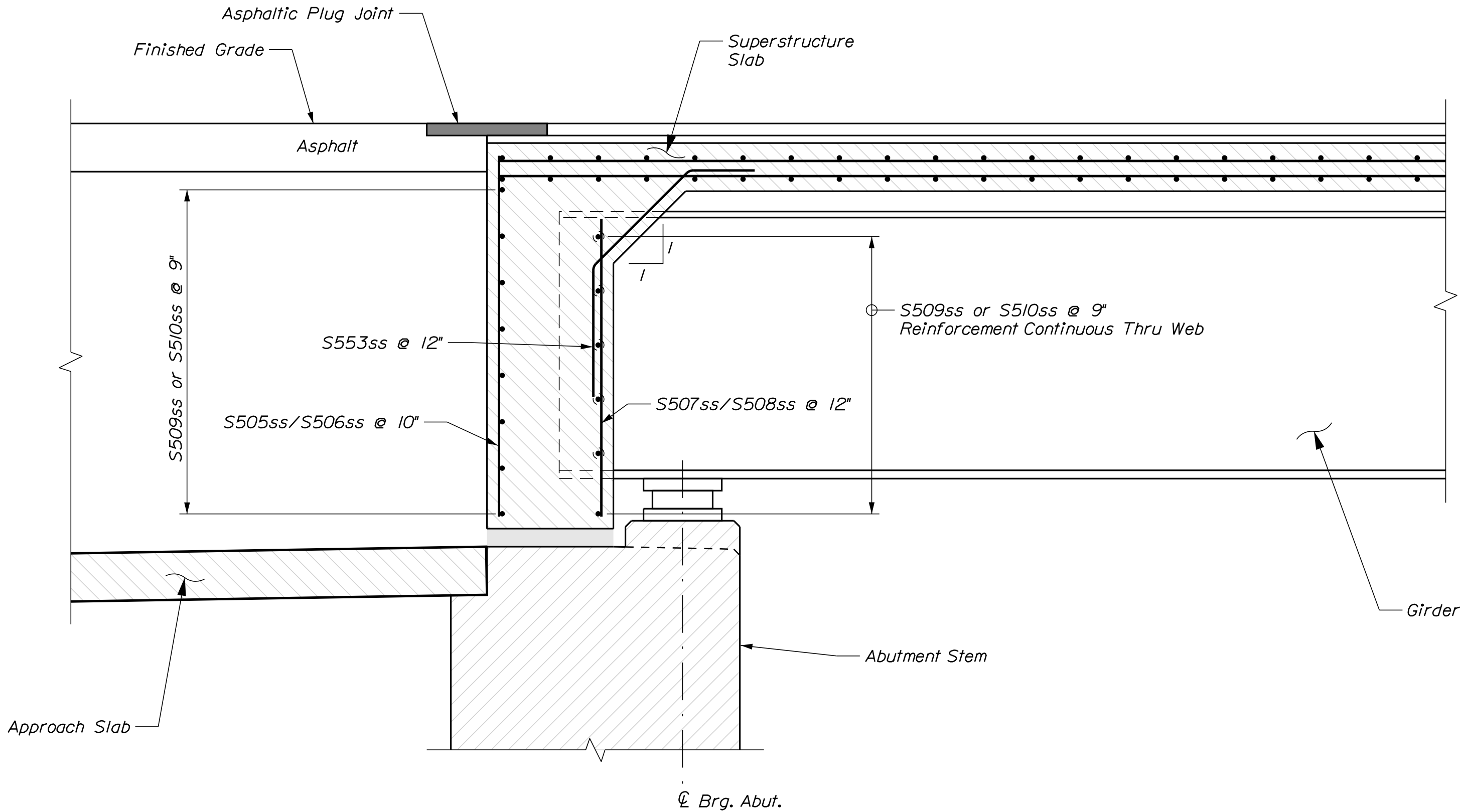
C.J. = Construction Joint
Ct.J. = Contraction Joint
C.J.R. = Construction Joint, Roughen
Surface 1/4" profile min. (Typ.)



ABUTMENT NO. 2 NW WINGWALL REINFORCEMENT ELEVATION
Scale: 3/16" = 1'-0"



ABUTMENT NO. 2 NE WINGWALL REINFORCEMENT ELEVATION
Scale: 3/16" = 1'-0"

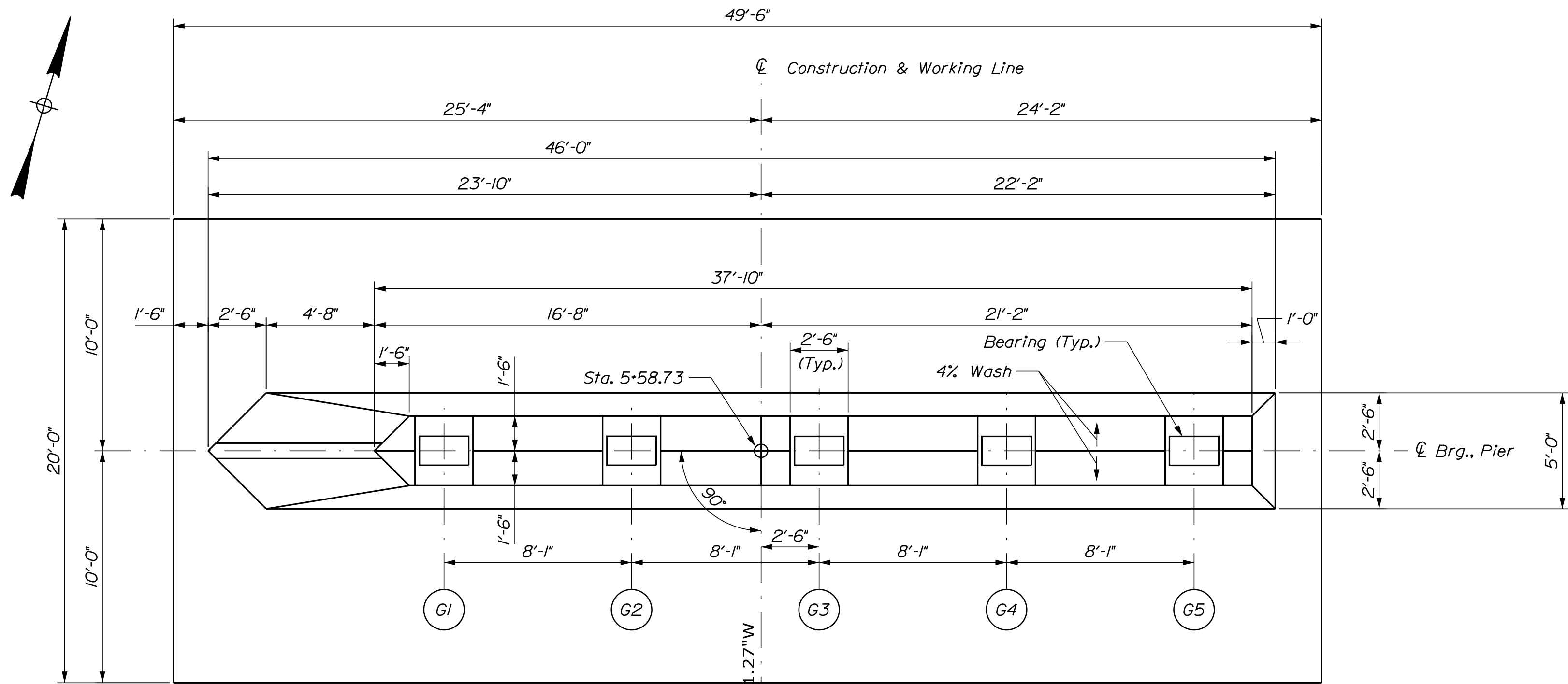


TYPICAL ABUTMENT BACKWALL SECTION DETAIL



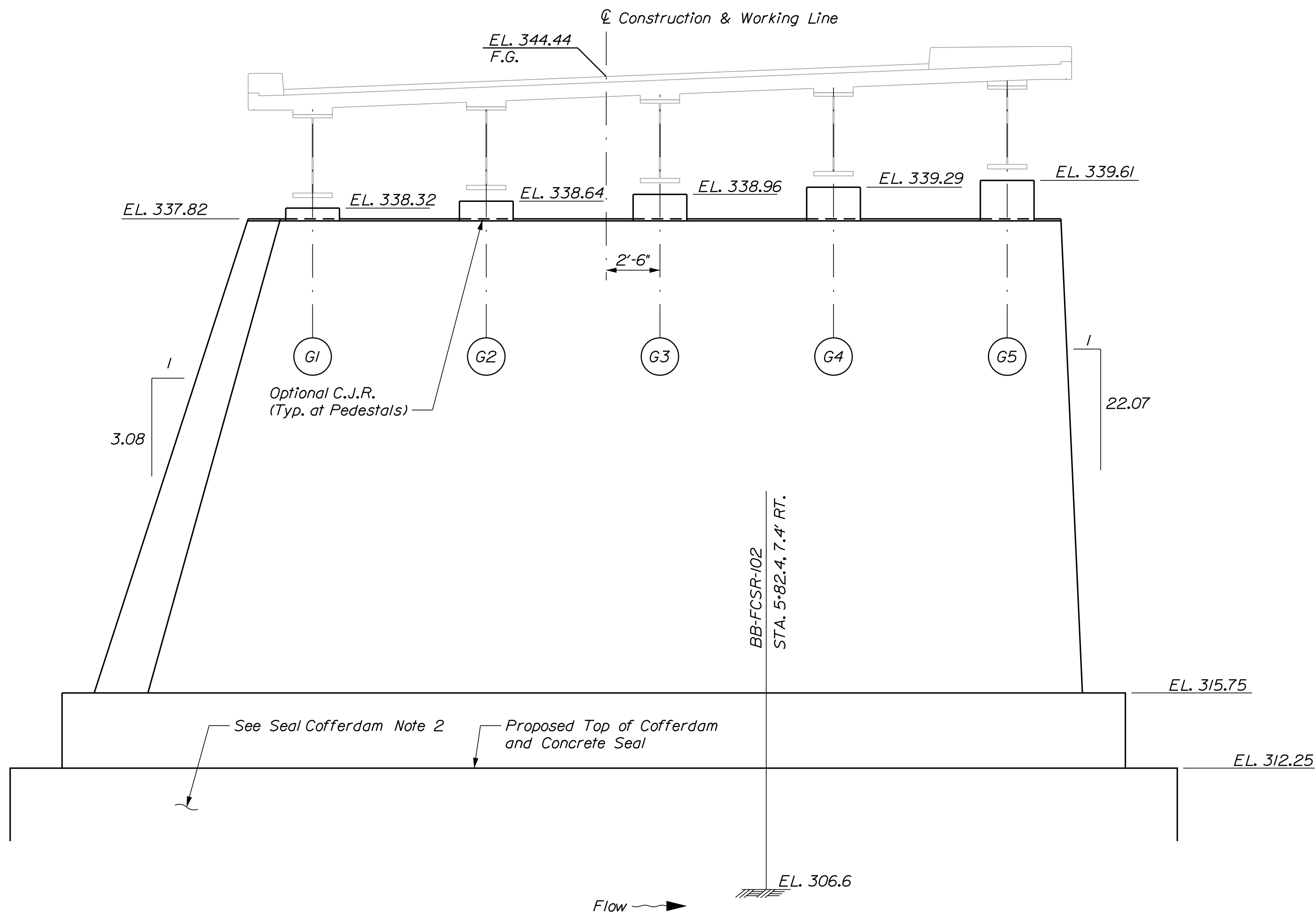
| | | | |
|------------------|---------------|-----------|--------|
| PROJ. MANAGER | MICHAEL WIGHT | BY | DATE |
| DESIGN-DETAILED | MYLENE | R. PARKER | 6/2021 |
| CHECKED-REVIEWED | C. SCHAK | C. SCHAK | 6/2021 |
| DESIGN-DETAILED | | | |
| REVISIONS 1 | | | |
| REVISIONS 2 | | | |
| REVISIONS 3 | | | |
| REVISIONS 4 | | | |
| FIELD CHANGES | | | |

| | |
|-------------|--|
| SIGNATURE | |
| P.E. NUMBER | |
| DATE | |



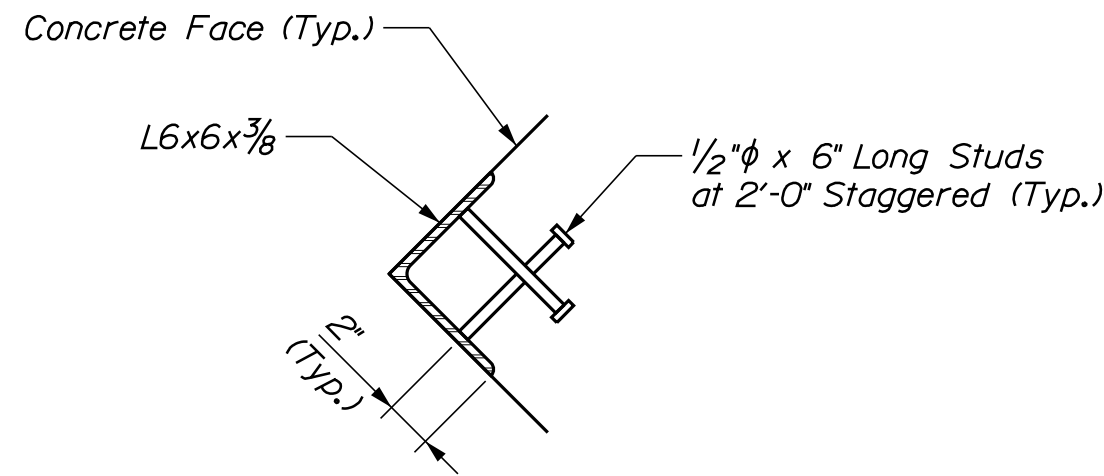
Flow →

PIER PLAN



Flow →

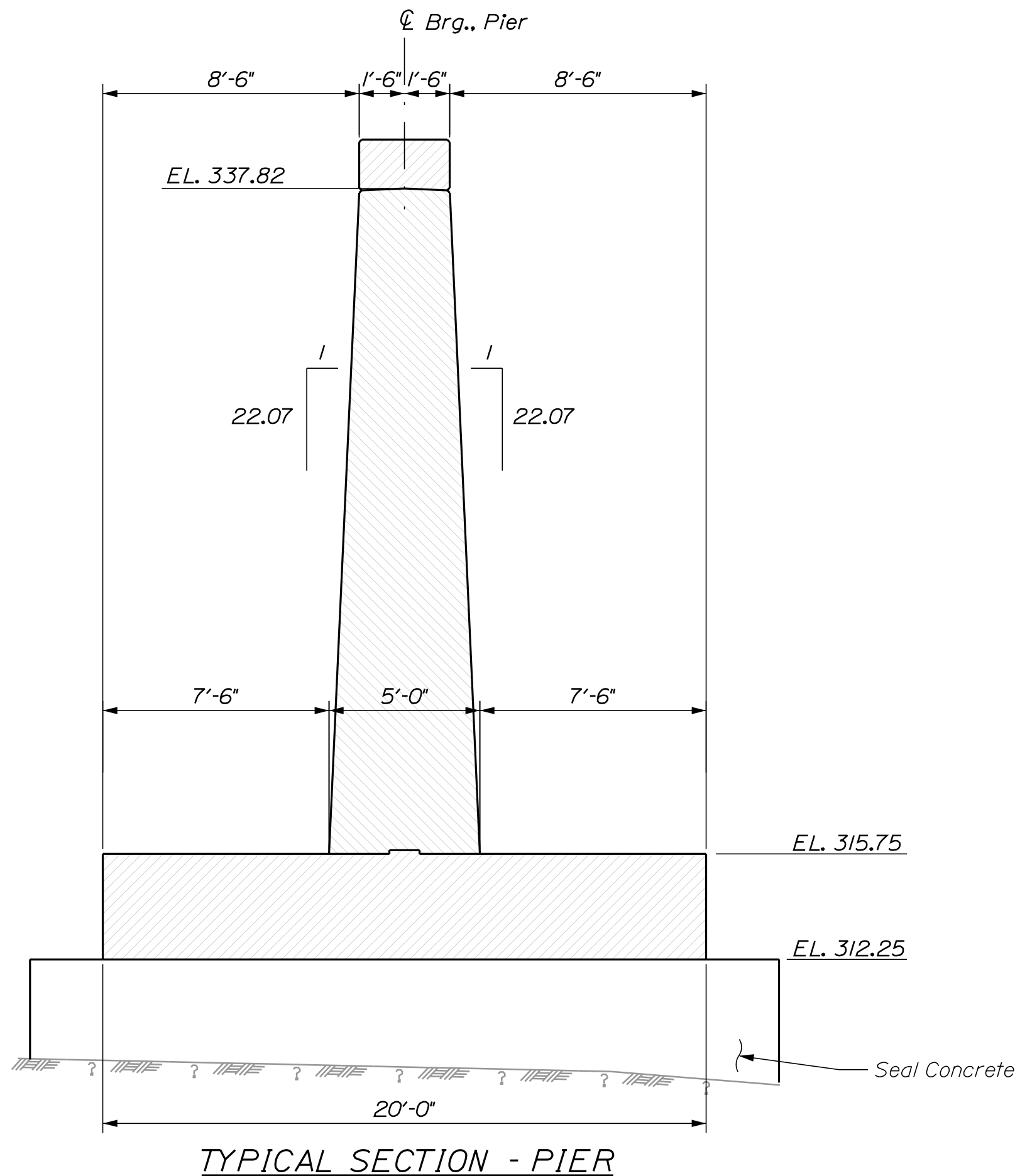
PIER ELEVATION



PIER NOSE ARMOR DETAIL

PIER NOSE ARMOR NOTES

1. Payment for furnishing and placing the concrete anchors and pier nose shall be incidental to the unit price bid for the concrete pier.
2. Pier nose protection shall be extended from top of footing to EL. 337.78.
3. Nose armor, including anchor studs, shall be cleaned and galvanized in accordance with Section 506 of the Standard Specifications and included in the Pay Item 502.239 - Structural Concrete Piers.

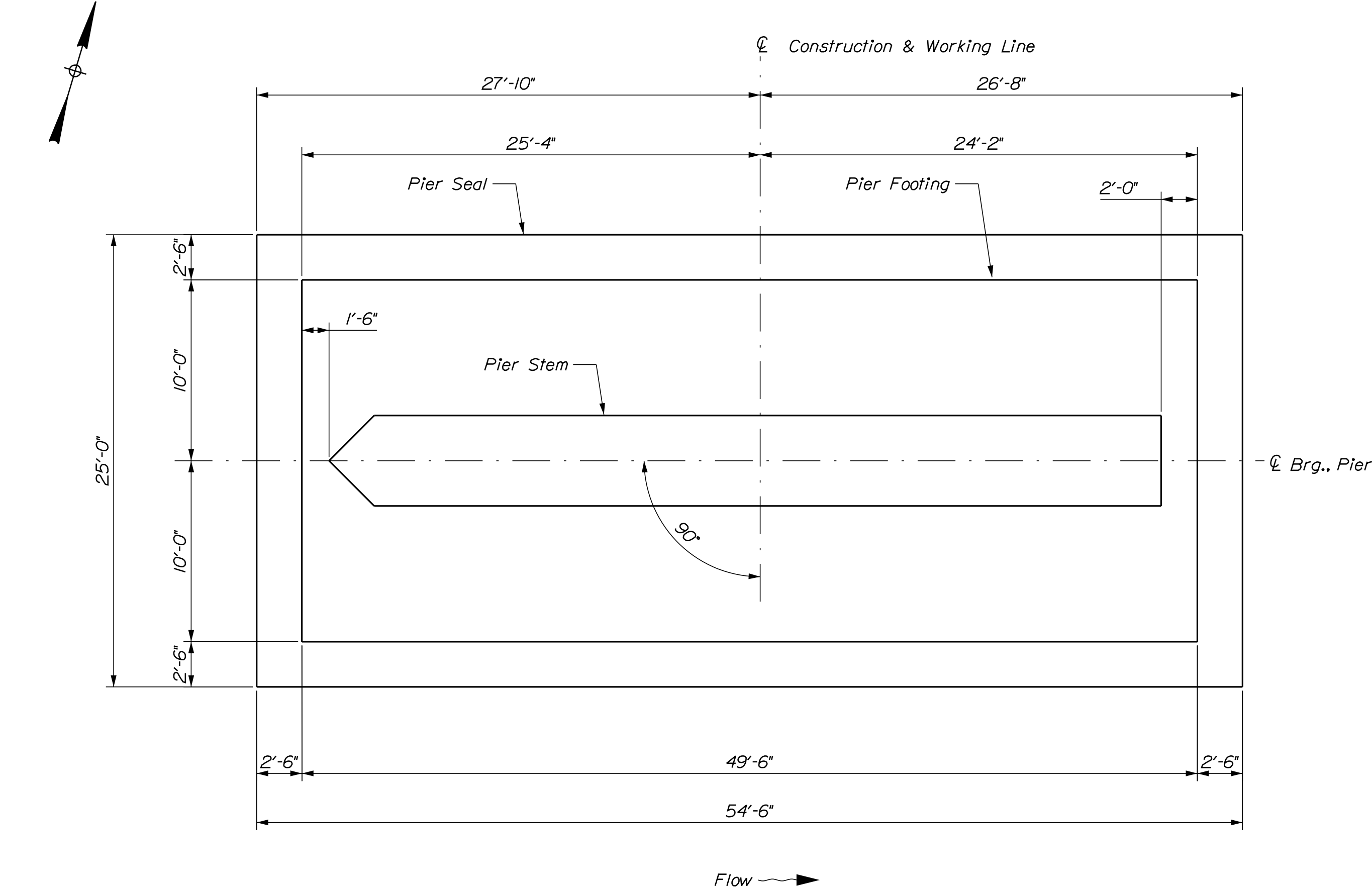


TYPICAL SECTION - PIER

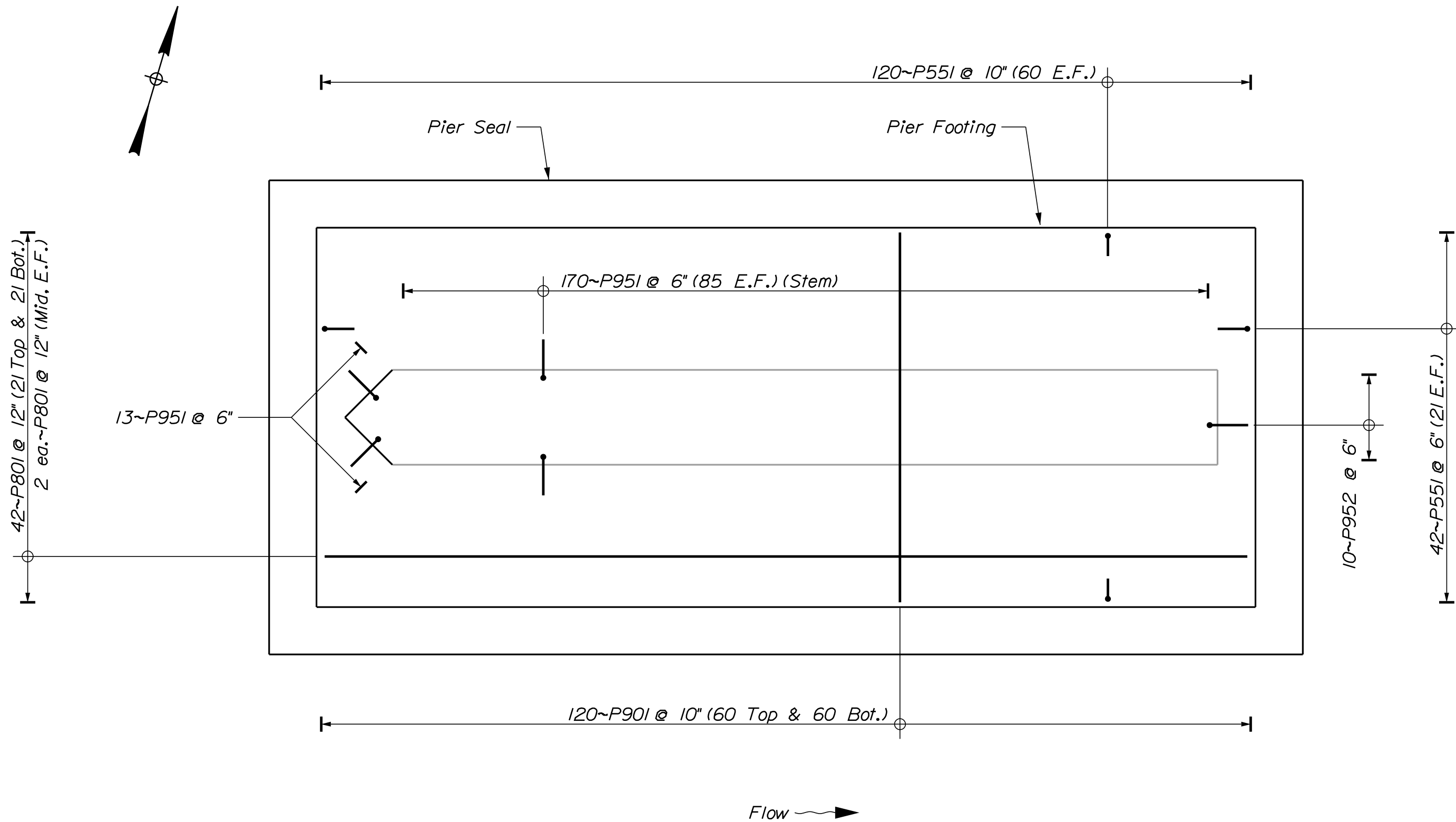
LEGEND:
C.J.R. = Construction Joint, Roughen Surface 1/4" profile min. (Typ.)

**ERDMAN
ANTHONY**

| | | | | | | | |
|---|--|------------------------------|--|---------------|--|---------------|--|
| STATE OF MAINE | | DEPARTMENT OF TRANSPORTATION | | 2229600 | | BRIDGE PLANS | |
| FARMINGTON FALLS BRIDGE | | SANDY RIVER | | PIER PLAN | | SHEET NUMBER | |
| CHESTERVILLE-FARMINGTON FRANKLIN COUNTY | | PIER PLAN | | AND ELEVATION | | 58 | |
| PROJECT NO. 2273 | | WIN | | 22296.00 | | OF 76 | |
| DATE | | SIGNATURE | | P.E. NUMBER | | DATE | |
| BY | | DATE | | REVISIONS 1 | | REVISIONS 2 | |
| CHECKED | | DESIGNED | | REVISIONS 3 | | REVISIONS 4 | |
| DESIGNED | | REVISIONS 1 | | REVISIONS 2 | | REVISIONS 3 | |
| REVISIONS 1 | | REVISIONS 2 | | REVISIONS 3 | | REVISIONS 4 | |
| REVISIONS 2 | | REVISIONS 3 | | REVISIONS 4 | | FIELD CHANGES | |
| REVISIONS 3 | | REVISIONS 4 | | FIELD CHANGES | | | |
| REVISIONS 4 | | FIELD CHANGES | | | | | |
| FIELD CHANGES | | | | | | | |



PIER SEAL AND FOOTING PLAN



PIER SEAL AND FOOTING REINFORCEMENT PLAN

PIER DESIGN CRITERIA

1. Critical AASHTO Load Combination - Maine Modified Strength I.
2. Buoyancy: Water level assumed at elevation 325.77 at Q1.I and elevation 335.30 at Q10.
3. Stream flow: Maximum Velocity of 3.32 fps at Q1.I and 5.34 fps at Q10 skewed at 0° to longitudinal centerline of pier.
4. Wind: 115 mph.
5. Ice: Thickness 1.5 feet, pressure 28.8 ksf at elevation 325.77 for Q1.I or pressure 14.4 ksf at elevation 340.61 for Q50. 30% of nose force applied transverse to pier.

PIER NOTES

1. Reinforcing steel shall have a minimum concrete cover of 3 inches unless otherwise noted.
2. Maximum calculated footing pressure is 6.6 ksf for Maine Modified Strength I.
3. At the option of the Resident, bedrock which protrudes above a horizontal plane 12 inches below the proposed bottom of footing elevation may be removed. Payment for bedrock removal shall be made under Item No. 206.092 Structural Rock Excavation - Major Structures.
4. Pier footing/seal concrete shall be placed on bedrock cleaned of all weathered rock, loose fractured rock and soil. The bedrock subgrade shall be confirmed to be relatively level. Where the bedrock slope exceeds 4H:1V, the bedrock surface shall be benched to create level steps or made completely level. The Resident shall approve the bedrock subgrade prior to the placement of the pier footing concrete.

PIER SEAL COFFERDAM NOTES

1. The seal concrete placement dimensions represent the minimum seal size necessary to meet design requirements and are not based on the use of any particular sheet pile section.
2. The horizontal pay limit for seal concrete shall be the dimensions shown on the plans. No additional payment will be made for concrete placed outside of these limits.
3. When sheet piling is used for seal cofferdams, appropriate rolled corners shall be used, and the inside face of the sheet piling shall be at or outside the seal concrete dimensions shown.
4. The assumed 5'-8" depth of the seal is set for a water elevation of 325.77. If the water elevation at the time of construction is higher, the depth of the seal shall be adjusted.
5. Seal concrete shall be placed on bedrock cleaned of weathered rock, loose fractured bedrock, boulders and soil. Where the bedrock surface slope exceeds 4H:1V, the bedrock surface shall be benched in level steps or made completely level. Cofferdam excavation inspection and reporting shall be in accordance with Standard Specifications Section 511 - Cofferdams. At a minimum, the inspection shall include bedrock elevation measurements and sediment measurements, taken at a minimum of 4 feet evenly distributed plan locations.
6. The bedrock may vary in nature, slope and degree of fracturing. Actual rock elevations may vary. After the foundation excavation is complete and all unsound bedrock is removed, the Contractor shall obtain foundation bedrock elevations in accordance with the Contractor's written procedure per Section 511 of the Standard Specifications.
7. Each seal shall be cored full depth in at least 4 locations to ensure that the seal is satisfactorily placed. The final core run shall sample the bedrock interface and a minimum of 1 foot of bedrock. Seal core locations will be approved by the Department. Seal concrete cores will be a minimum 3 inch outer diameter and be stored in boxes labeled. In the event that voids or other defects are indicated, the Contractor shall correct the defects in a manner approved by the Department. For each core that reveals a defect, 2 additional cores shall be taken in approximately the same area in locations approved by the Department. All core holes shall be filled using non-shrink grout selected from the MaineDOT Qualified Products List of Grout Materials. The cost of coring and repairs will be considered incidental to related pay items.

SHEET NUMBER

60

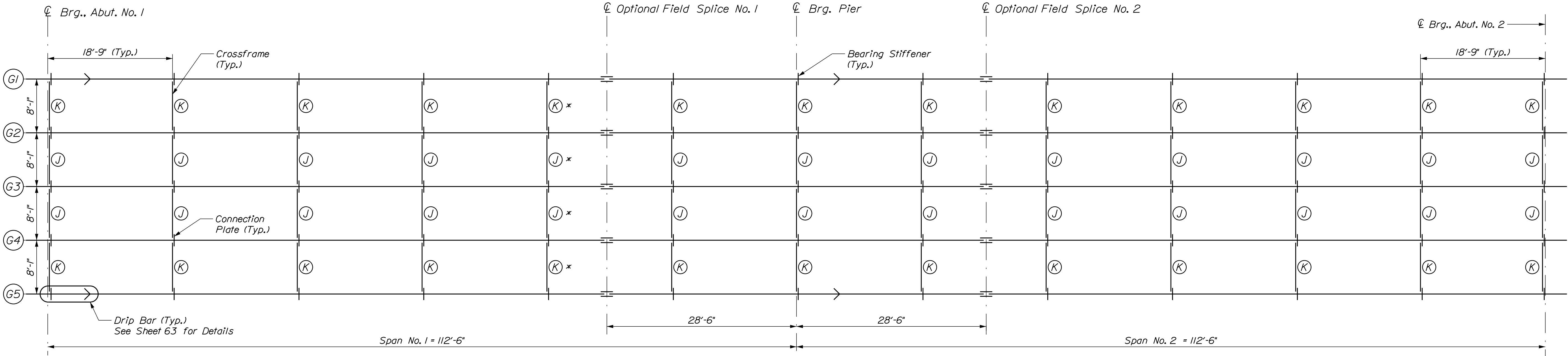
OF 76

Date:6/29/2021

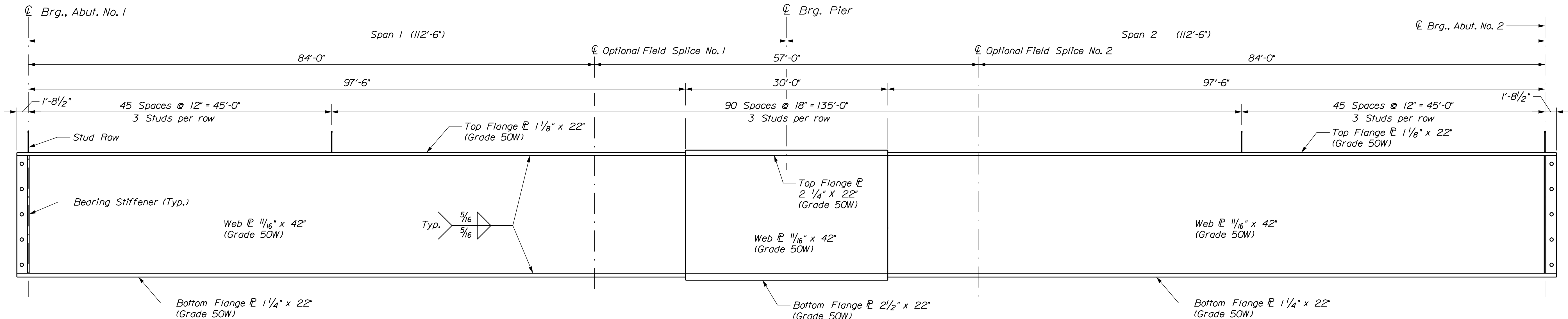
Username: LindoT

Division: BRIDGE

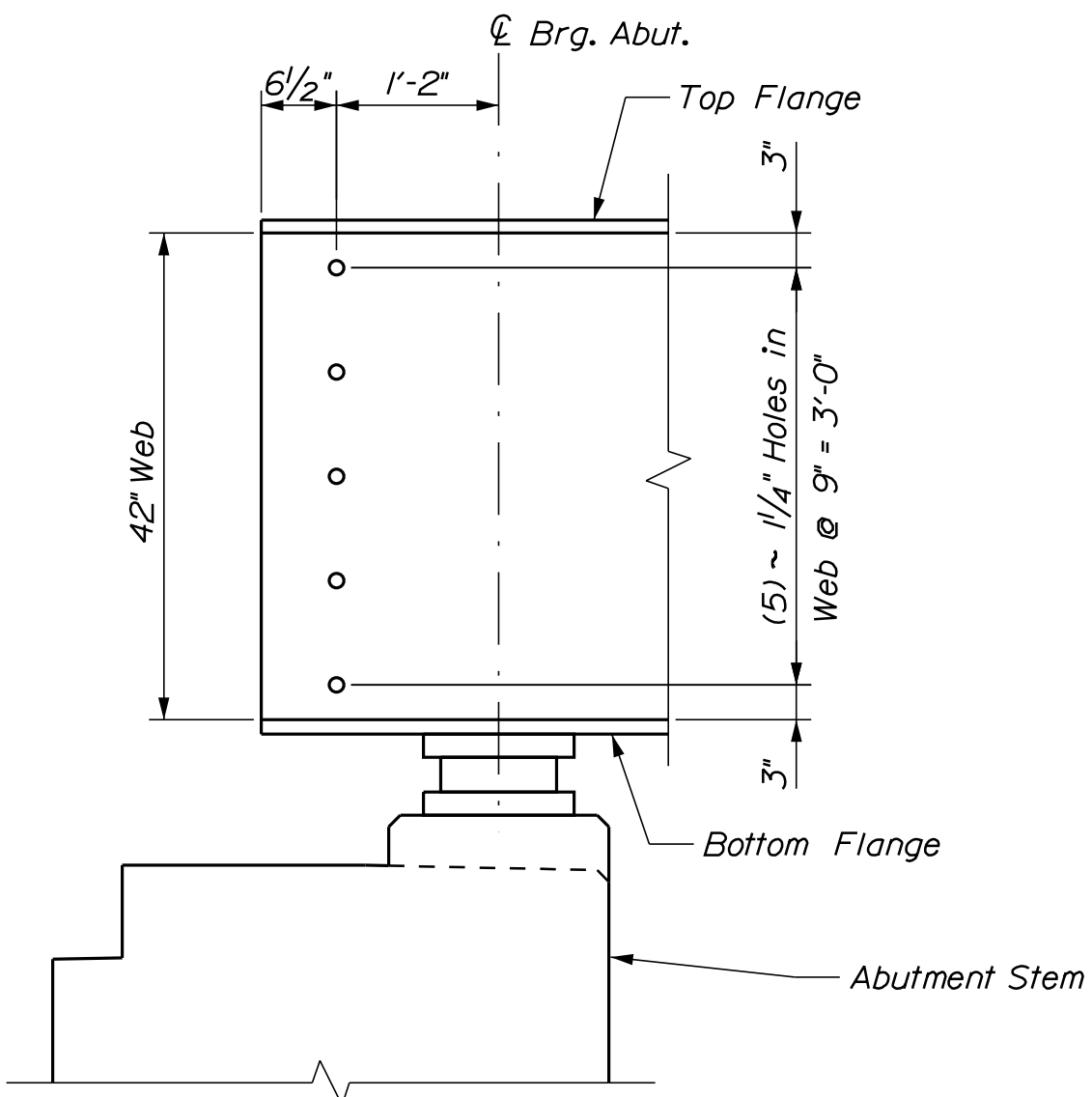
Filename: ... \MSTAN061_Framing-Plan.dgn



FRAMING PLAN



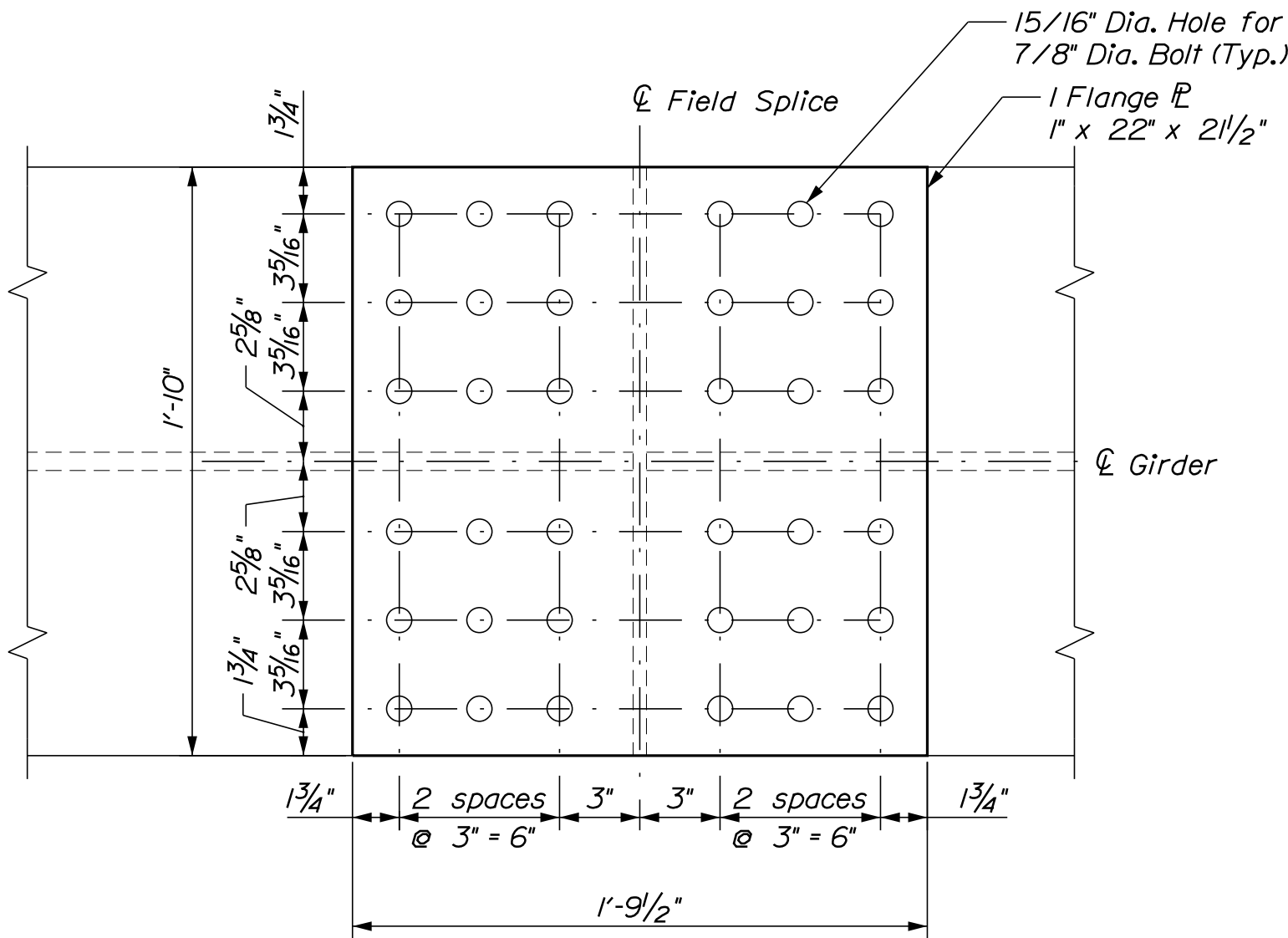
GIRDER ELEVATION
Vertical Scale Exaggerated for Clarity



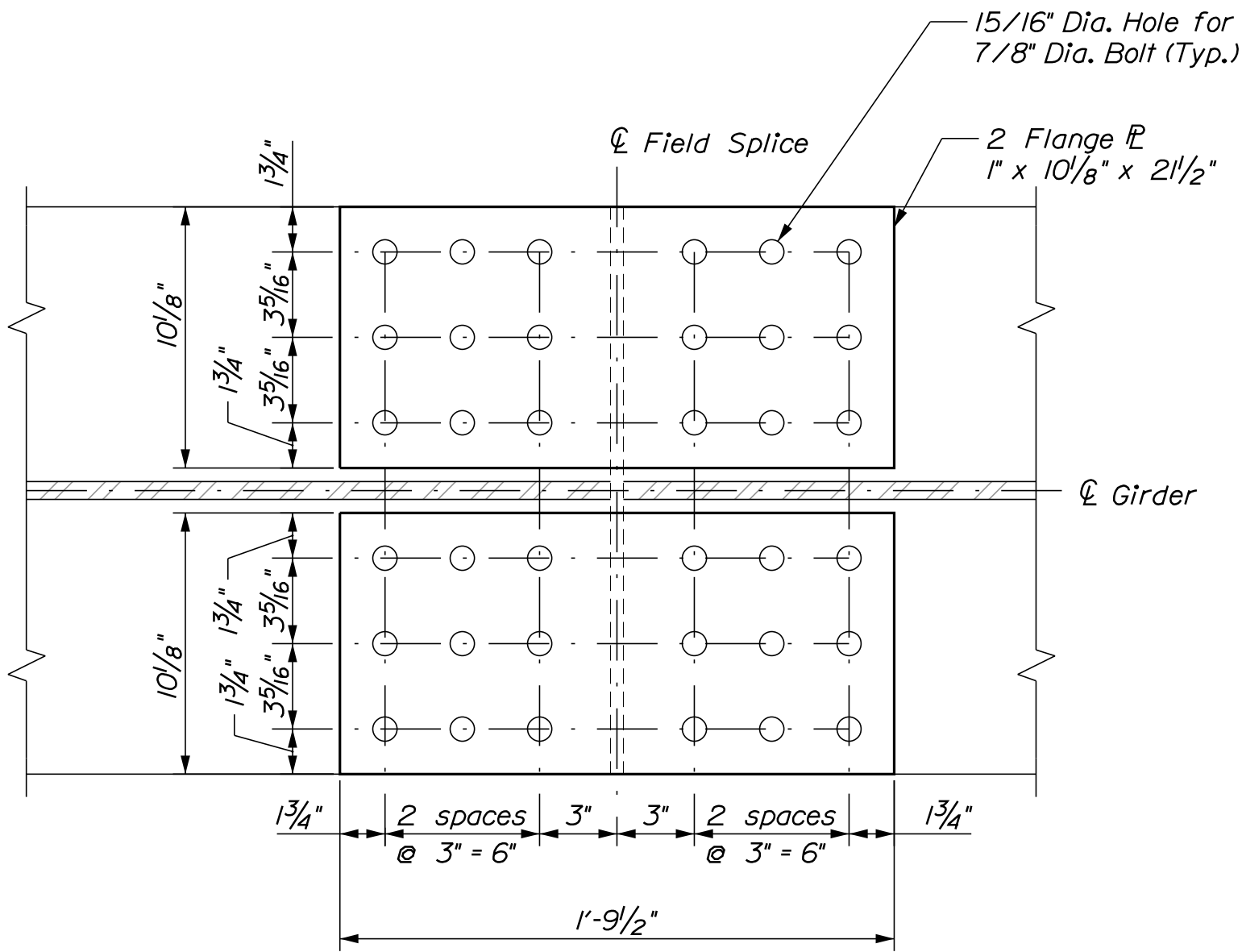
GIRDER END DETAIL

NOTES:

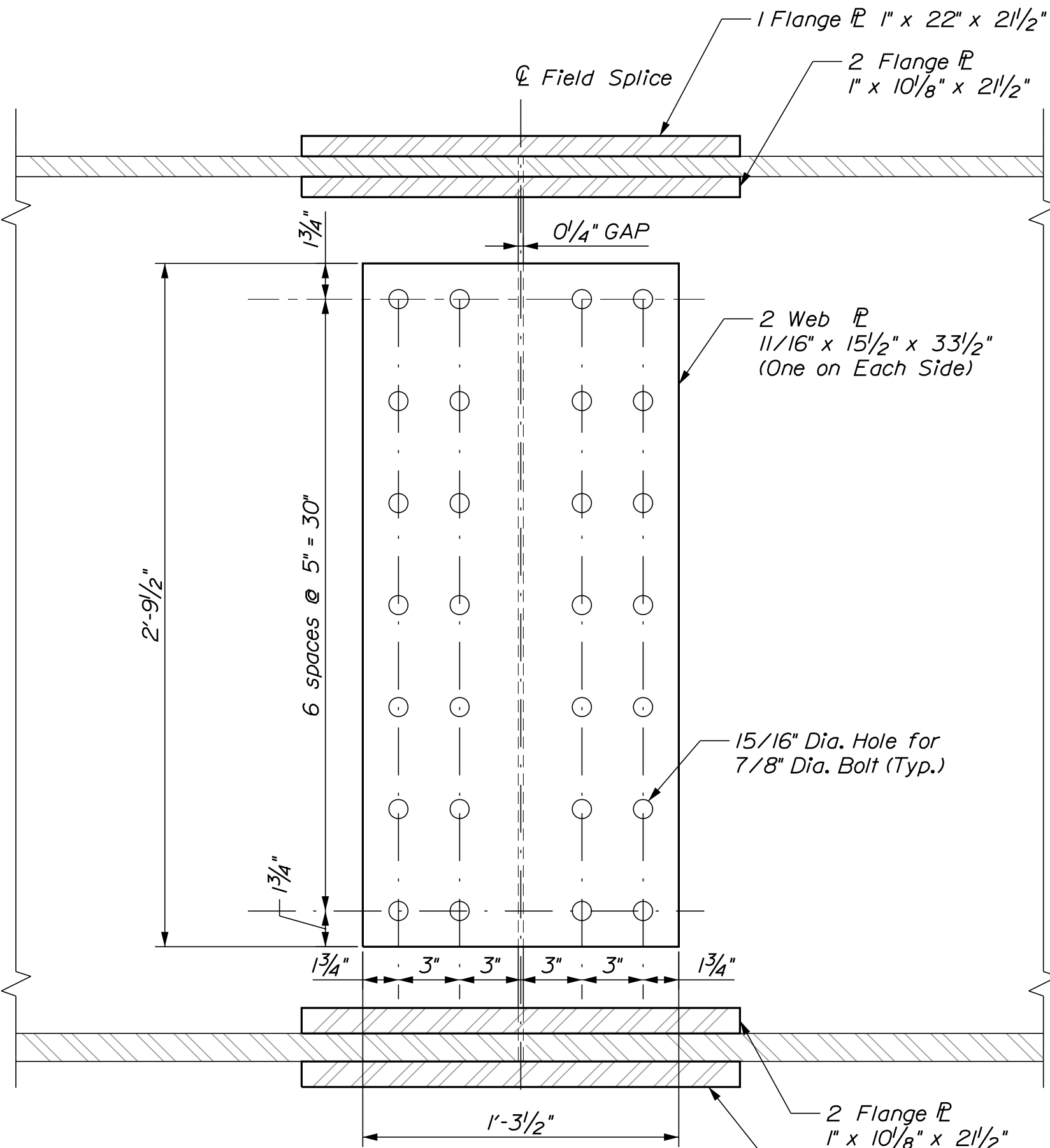
1. At locations marked with an asterisk (*), the designated diaphragms/cross frames shall be changed to a Type C1 diaphragm as required to accommodate the Contractor's deck placement sequence. No extra compensation will be allowed for any diaphragms so substituted, and any additional costs will be considered incidental to Contract items.



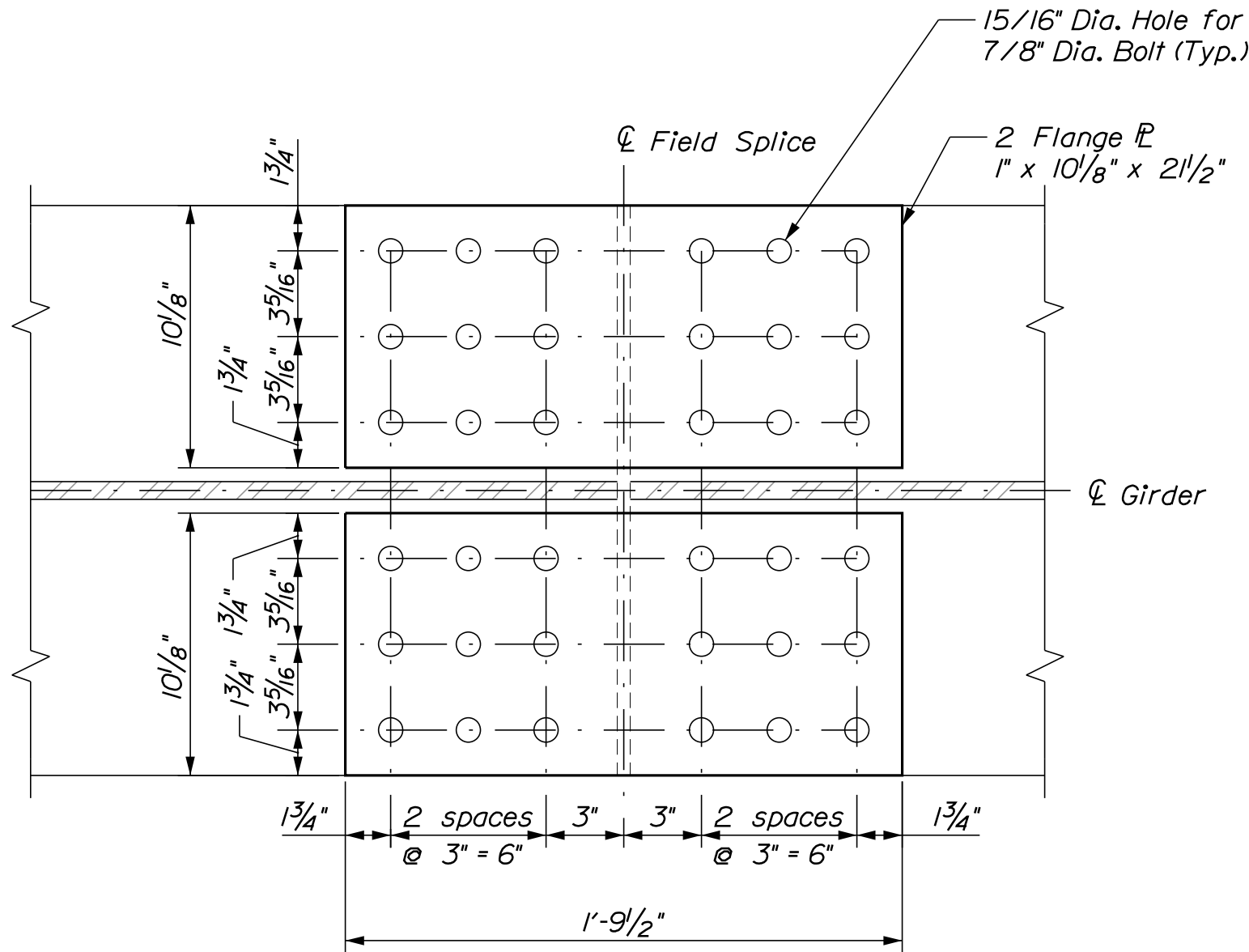
TOP SPLICE PLATE OUTSIDE



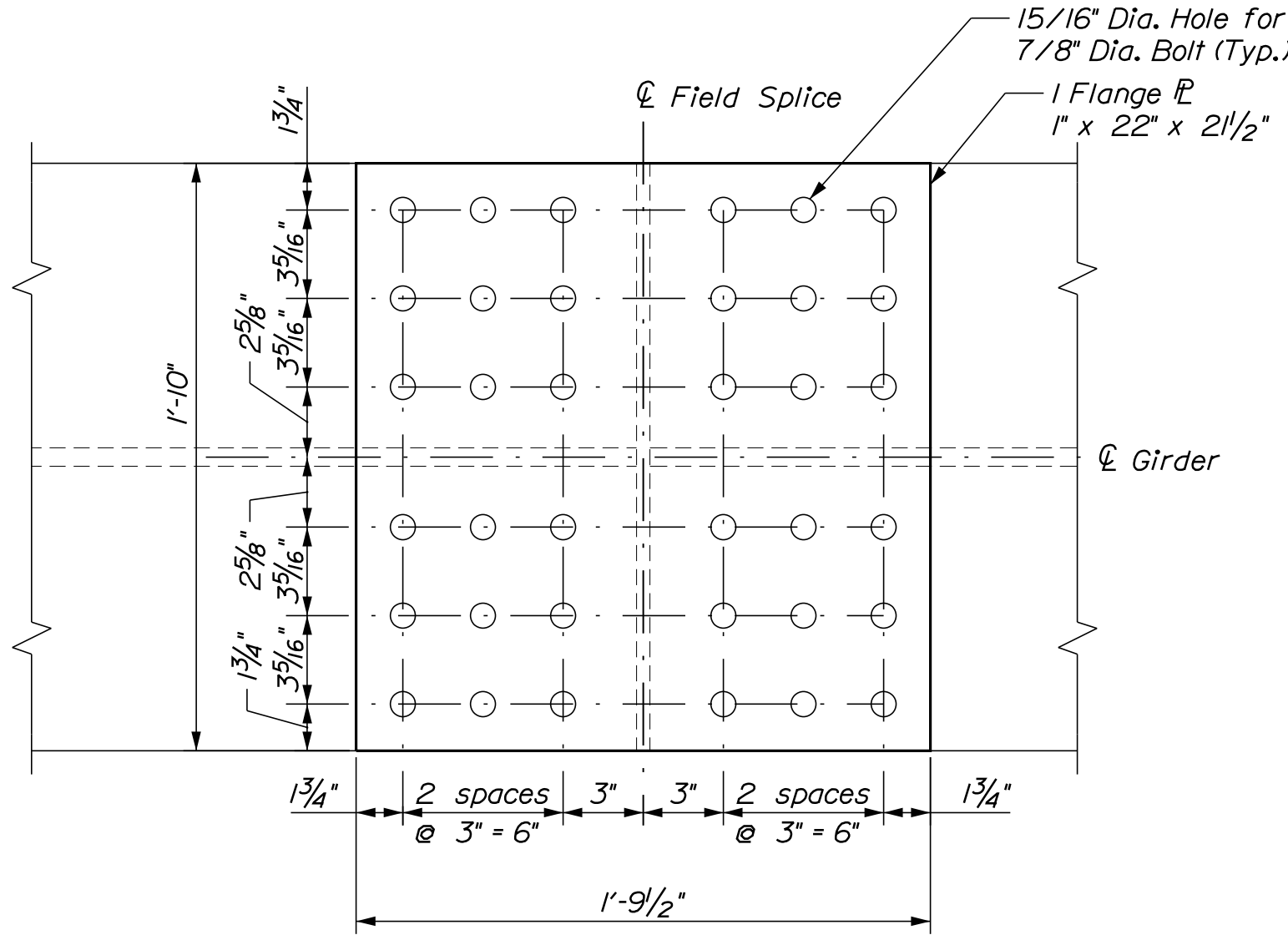
TOP SPLICE PLATE INSIDE



SPLICE ELEVATION VIEW



BOTTOM SPLICE PLATE INSIDE



BOTTOM SPLICE PLATE OUTSIDE

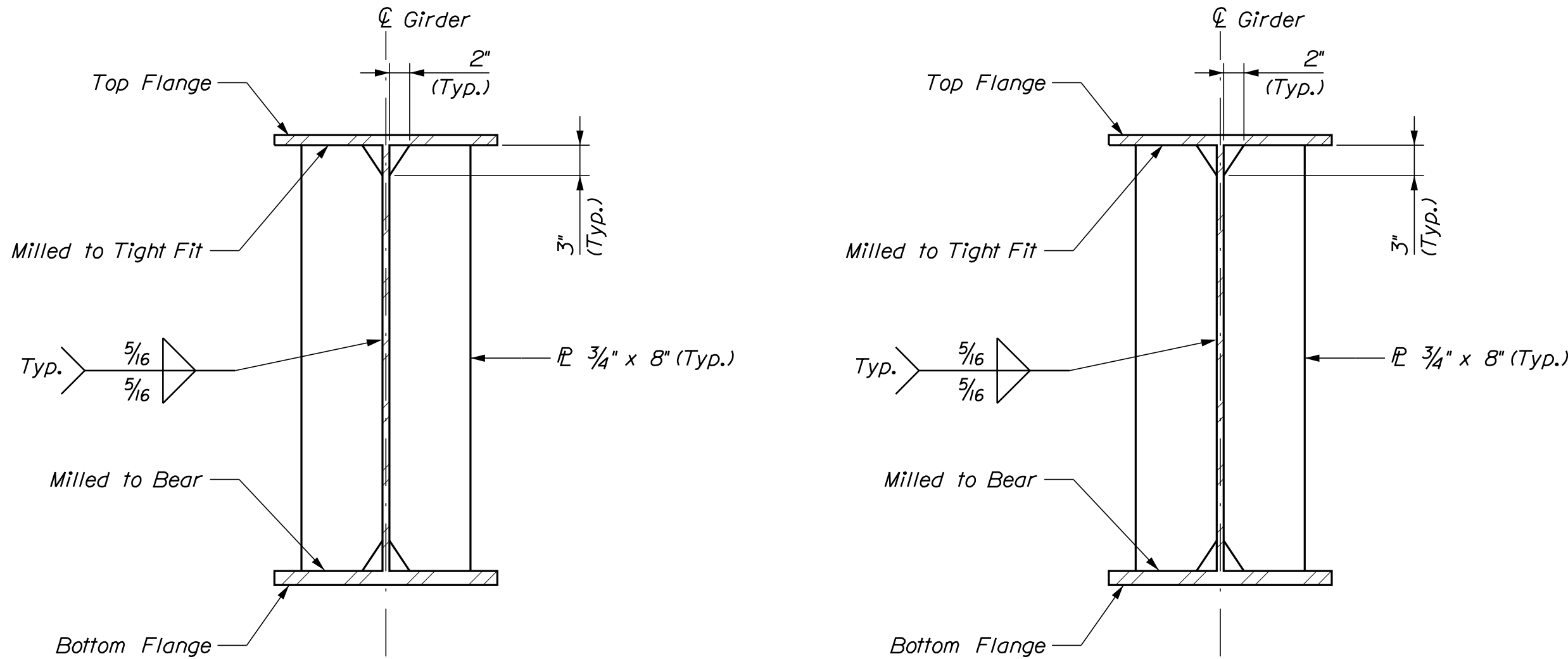


| PROJ. MANAGER | MICHAEL WIGHT | BY | DATE |
|-------------------|---------------|-----------|--------|
| CHECKED-DETAILED | MYLENE | R. PARKER | 6/2021 |
| DESIGNED-DETAILED | C. SICHAK | C. SICHAK | 6/2021 |
| DESIGNED-DETAILED | | | |
| REVISIONS 1 | | | |
| REVISIONS 2 | | | |
| REVISIONS 3 | | | |
| REVISIONS 4 | | | |
| FIELD CHANGES | | | |

SIGNATURE

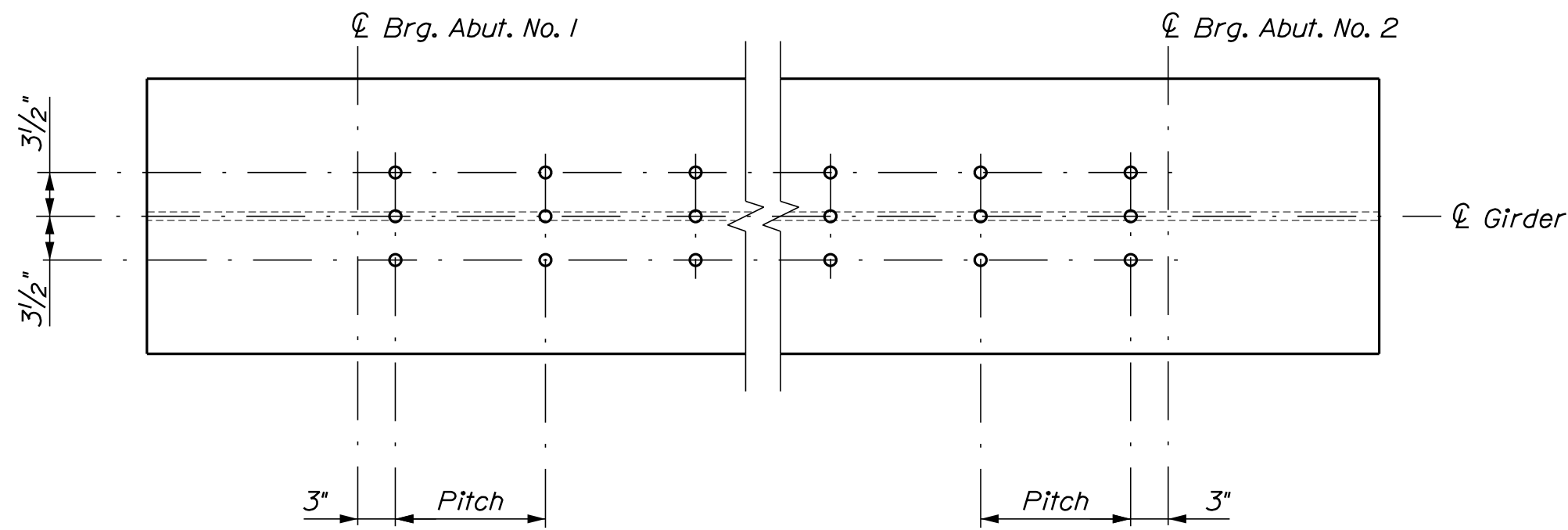
P.E. NUMBER

DATE



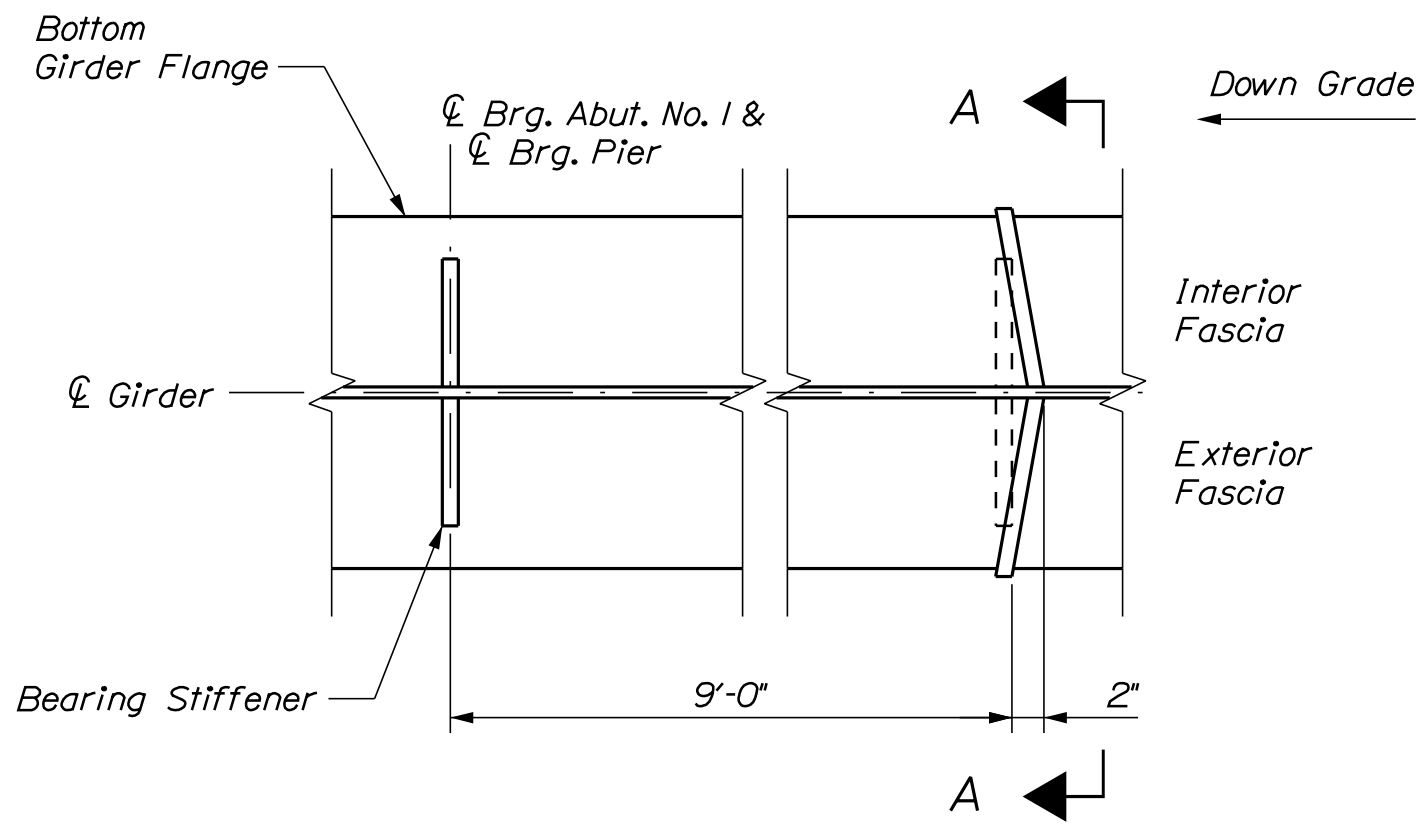
BEARING STIFFENER AT ABUTMENT

BEARING STIFFENER AT PIER

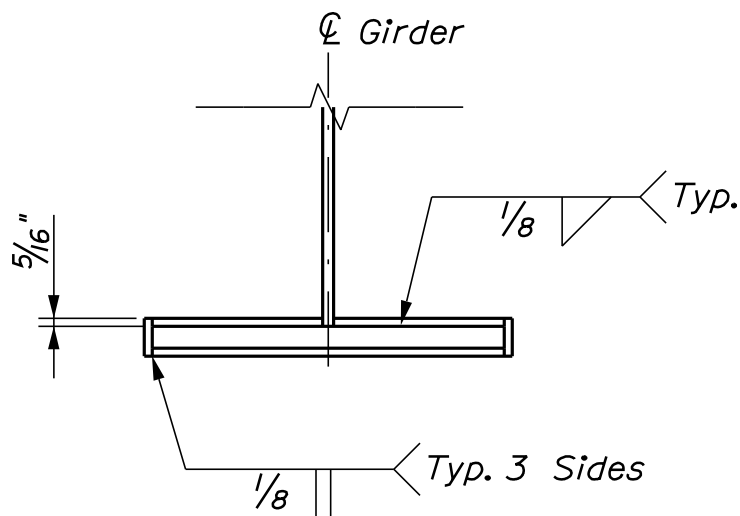


Note:
1. See Girder Elevation on Sheet No. 61 for shear connector pitch.

STUD LAYOUT PLAN



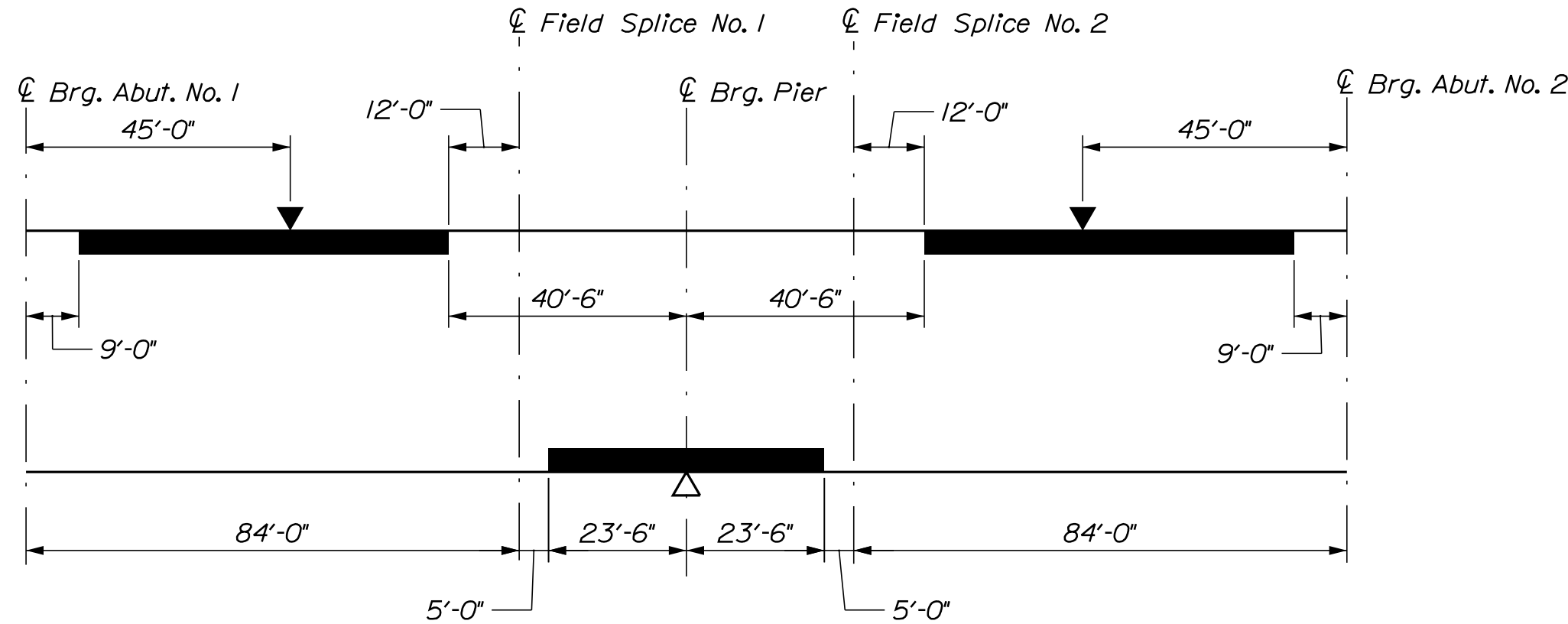
PLAN



SECTION A-A

DRIP BAR DETAIL

(Abutment No. 1 and Pier Typical)



STRESS DIAGRAM

(Shaded areas are always in compression,
others are in tension or have stress reversals)
(Girder 5 shown, others similar)

STRUCTURAL STEEL NOTES

1. Camber ordinates, as shown on Sheet 64, are computed to compensate for all dead load deflections and for the finished grade profile.

2. No transverse butt weld splices will be allowed in the flange plates or web plates within 10 feet or 10 percent of the span length (whichever is greater) from the points of maximum negative moment or maximum positive moment. Butt weld splices in flanges shall be not less than three feet from transverse butt welds in the web plates and no transverse web or flange butt welds shall be located within three feet of other transverse welds (e.g. connection plates to web welds) on either flange or web. No transverse butt weld splices will be allowed in areas of stress reversal.

3. Sections of flange plates or web plates between transverse shop splices or between a transverse shop splice and a field splice shall be not less than 10 feet in length unless otherwise shown on the plans.

4. Bearing stiffeners shall be plumb after erection and dead loading of the structure. Intermediate web stiffeners may be either plumb or normal to the top flange.

5. Cross frame connection plates may be either plumb or normal to the top flange.

6. Filler plates shall be weathering steel conforming to the requirements of ASTM A 709, Grade 50W.

7. For cross frame details, see Standard Details 504(04) and 504(07).

8. Girder ends and diaphragms or cross frames within 10 feet of the centerline of bearing at the abutments shall be coated with a Zinc Rich Coating System, in accordance with Standard Specifications Section 506, Shop Applied Protective Coating-Steel. The color shall be Federal Standard 595B, Color Number 30045 (Brown). NEPCOAT Qualified Products List C may be used.

9. Bolted field splice connections shall be made using 7/8" diameter ASTM A325 Type 3 H.S. bolts. Hole size shall be 15/16" diameter unless otherwise shown. Bolt threads shall be excluded from the shear plane of field splice connections.

10. Bolted cross frame connection shall be made using 7/8" diameter ASTM A325 Type 3 H.S. bolts. Hole size shall be 15/16" diameter. The minimum edge distance shall be 1 1/2" unless otherwise shown. Oversized or short-slotted holes are not permitted for use in cross frame connections. Bolt threads shall be excluded from the shear plane of cross frame connections.

11. Prior to structural steel erection, the Contractor shall submit an erection plan sequence to the Engineer for approval.

12. All web, flange, field splice plates and diaphragms (including connection plates) in tension or stress reversal areas shall conform to zone 2 Charpy V-notch impact test requirements of AASHTO M270.

13. Girder webs shall be vertical under full dead load.

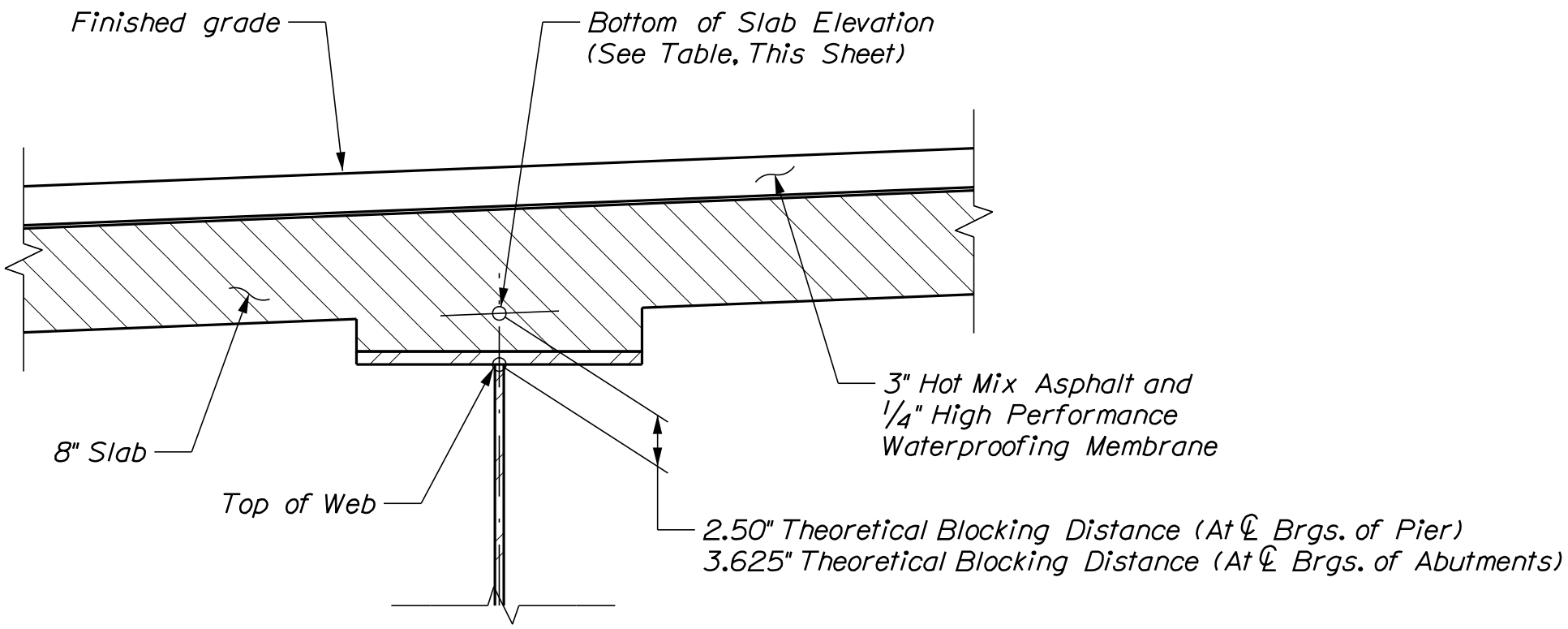
14. Holes in web shall be vertical (plumb) after superstructure slab placement.

15. The structural steel was designed with a vertical construction load of 50 psf and a lateral wind velocity of 115 mph.

16. After placement of the superstructure concrete, thoroughly clean the abutments and pier of all stains with a method approved by the Resident. Payment will be considered incidental to related Contract items.



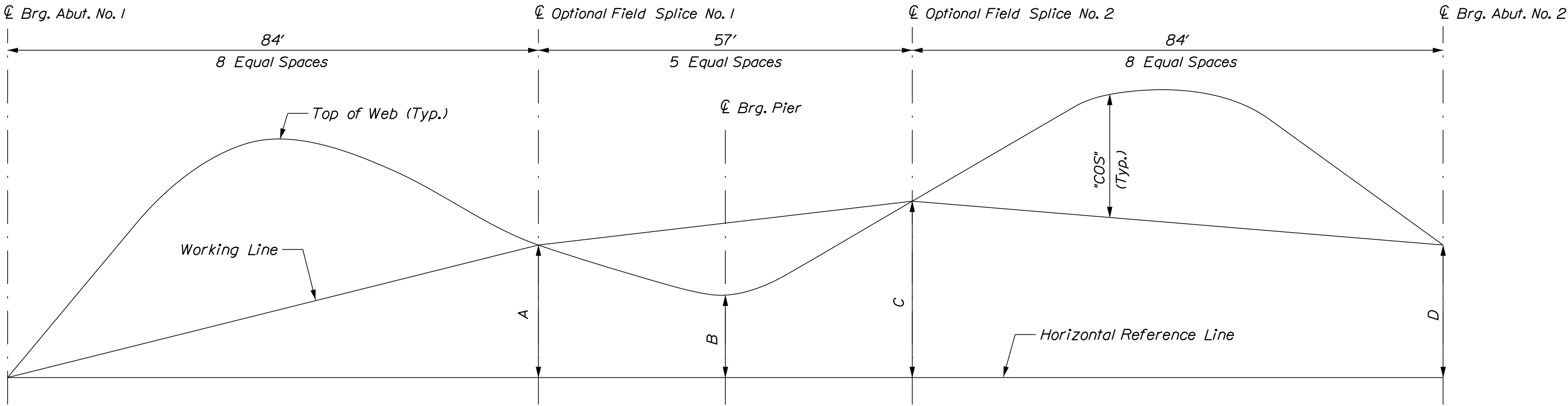
| BOTTOM OF SLAB ELEVATION TABLE (w/DEFLECTIONS) | | | | | | | | | | | | | | | | | | | | | | |
|--|-----------------------|---------|---------|---------|---------|---------|---------|---------|---------|---------|----------------|---------|---------|---------|---------|---------|---------|---------|---------|---------|-----------------------|--|
| Span 1 | | | | | | | | | | | | Span 2 | | | | | | | | | | |
| GIRDER | ℄ Brg. Abut. No. 1 | .1 | .2 | .3 | .4 | .5 | .6 | .7 | .8 | .9 | ℄ Brg. Pier | .1 | .2 | .3 | .4 | .5 | .6 | .7 | .8 | .9 | ℄ Brg. Abut. No. 2 | |
| G1 | 342.336 | 342.426 | 342.510 | 342.584 | 342.648 | 342.701 | 342.744 | 342.780 | 342.814 | 342.854 | 342.856 | 342.966 | 343.037 | 343.114 | 343.189 | 343.257 | 343.315 | 343.362 | 343.399 | 343.425 | 343.446 | |
| G2 | 342.659 | 342.751 | 342.836 | 342.912 | 342.976 | 343.028 | 343.069 | 343.103 | 343.134 | 343.172 | 343.179 | 343.283 | 343.357 | 343.437 | 343.515 | 343.584 | 343.644 | 343.690 | 343.725 | 343.750 | 343.769 | |
| G3 | 342.983 | 343.078 | 343.167 | 343.244 | 343.310 | 343.361 | 343.401 | 343.431 | 343.459 | 343.494 | 343.503 | 343.606 | 343.682 | 343.765 | 343.846 | 343.918 | 343.977 | 344.022 | 344.056 | 344.077 | 344.093 | |
| G4 | 343.306 | 343.408 | 343.503 | 343.585 | 343.652 | 343.704 | 343.741 | 343.768 | 343.792 | 343.824 | 343.826 | 343.935 | 344.015 | 344.102 | 344.186 | 344.260 | 344.320 | 344.363 | 344.392 | 344.408 | 344.416 | |
| G5 | 343.629 | 343.742 | 343.845 | 343.932 | 344.002 | 344.053 | 344.089 | 344.112 | 344.133 | 344.162 | 344.149 | 344.273 | 344.356 | 344.446 | 344.534 | 344.610 | 344.669 | 344.710 | 344.734 | 344.742 | 344.739 | |



BLOCKING DETAIL

| TABLE OF CAMBER ORDINATES PER PIECE ("COP" in) | | | | | | | | | | | | | | | | | | | | | | |
|--|--------------------|------|------|------|------|------|------|------|------------------|-------|-------|-------|-------|------------------|------|------|------|------|------|------|------|--------------------|
| GIRDER | ℄ Brg. Abut. No. 1 | 1/8 | 2/8 | 3/8 | 4/8 | 5/8 | 6/8 | 7/8 | ℄ Field Splice 1 | 1/5 | 2/5 | 3/5 | 4/5 | ℄ Field Splice 2 | 1/8 | 2/8 | 3/8 | 4/8 | 5/8 | 6/8 | 7/8 | ℄ Brg. Abut. No. 2 |
| G1 | 0.00 | 0.47 | 0.85 | 1.08 | 1.16 | 1.06 | 0.81 | 0.43 | 0.00 | -0.33 | -0.63 | -0.64 | -0.34 | 0.00 | 0.44 | 0.82 | 1.07 | 1.17 | 1.09 | 0.86 | 0.47 | 0.00 |
| G2 | 0.00 | 0.49 | 0.89 | 1.14 | 1.23 | 1.13 | 0.86 | 0.46 | 0.00 | -0.37 | -0.63 | -0.64 | -0.37 | 0.00 | 0.47 | 0.87 | 1.14 | 1.24 | 1.15 | 0.90 | 0.50 | 0.00 |
| G3 | 0.00 | 0.53 | 0.96 | 1.23 | 1.33 | 1.23 | 0.94 | 0.50 | 0.00 | -0.40 | -0.67 | -0.68 | -0.41 | 0.00 | 0.51 | 0.95 | 1.24 | 1.34 | 1.24 | 0.97 | 0.53 | 0.00 |
| G4 | 0.00 | 0.59 | 1.08 | 1.38 | 1.48 | 1.37 | 1.05 | 0.56 | 0.00 | -0.45 | -0.78 | -0.79 | -0.45 | 0.00 | 0.57 | 1.05 | 1.38 | 1.49 | 1.38 | 1.08 | 0.59 | 0.00 |
| G5 | 0.00 | 0.68 | 1.23 | 1.56 | 1.68 | 1.54 | 1.17 | 0.63 | 0.00 | -0.48 | -0.93 | -0.94 | -0.49 | 0.00 | 0.64 | 1.18 | 1.55 | 1.68 | 1.57 | 1.23 | 0.68 | 0.00 |

| TABLE OF CAMBER DIMENSIONS (ft) | | | | |
|---------------------------------|------|------|------|------|
| GIRDER | A | B | C | D |
| G1 | 0.78 | 0.55 | 0.80 | 1.11 |
| G2 | 0.74 | 0.55 | 0.76 | 1.11 |
| G3 | 0.77 | 0.55 | 0.79 | 1.11 |
| G4 | 0.92 | 0.55 | 0.93 | 1.11 |
| G5 | 1.17 | 0.55 | 1.19 | 1.11 |



CAMBER & DEFLECTION NOTES

CAMBER DIAGRAM BY PIECE

1. Camber ordinates, as shown, are computed to compensate for all dead load deflections and for the curvature of the finished grade profile.



STATE OF MAINE
DEPARTMENT OF TRANSPORTATION

2229600

WIN
22296.00

BRIDGE NO. 2273

BRIDGE PLANS

PROJ. MANAGER
MICHAEL WIGHT

CHECKED-REVIEWED
M. WIGHT
C. SICHAK

DESIGNED-DETAILED
C. SICHAK

REVISIONS
1
2
3
4

DATE
6/2021
6/2021

BY
R. PARKER
C. SICHAK

SIGNATURE

P.E. NUMBER

DATE

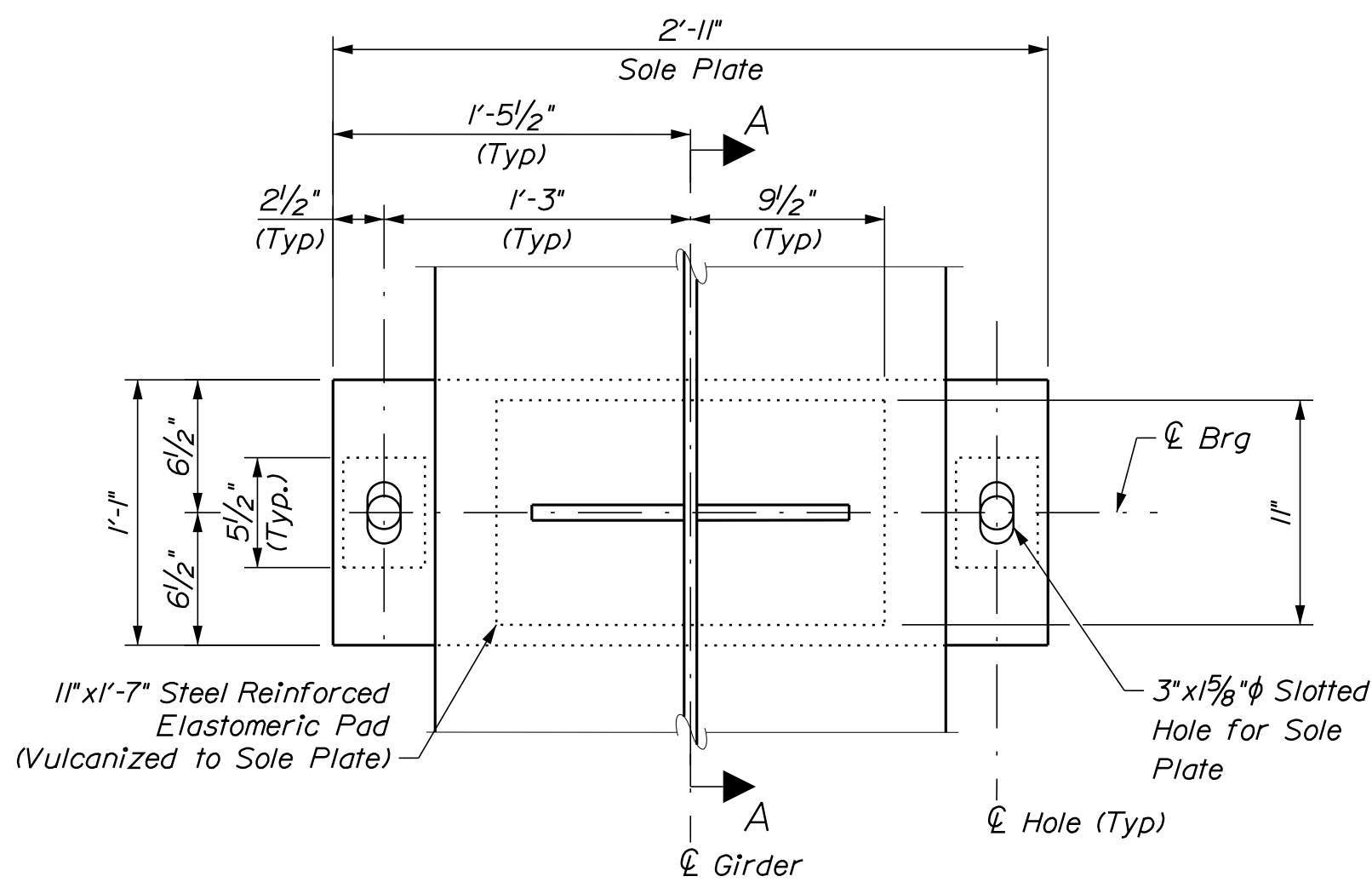
FARMINGTON FALLS BRIDGE
SANDY RIVER
CHESTERVILLE-FARMINGTON FRANKLIN COUNTY

CAMBER DIAGRAM

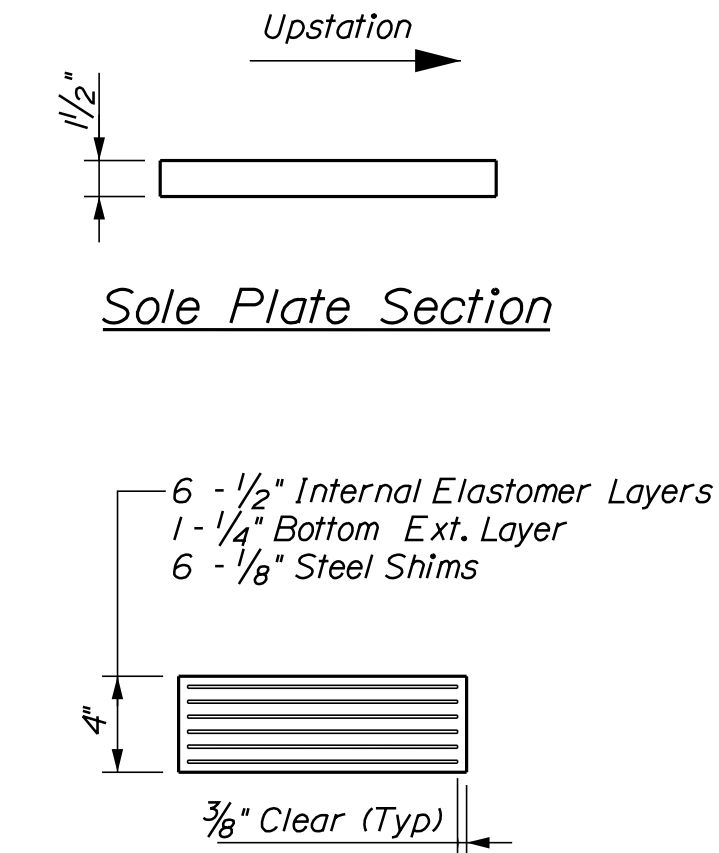
SHEET NUMBER

64

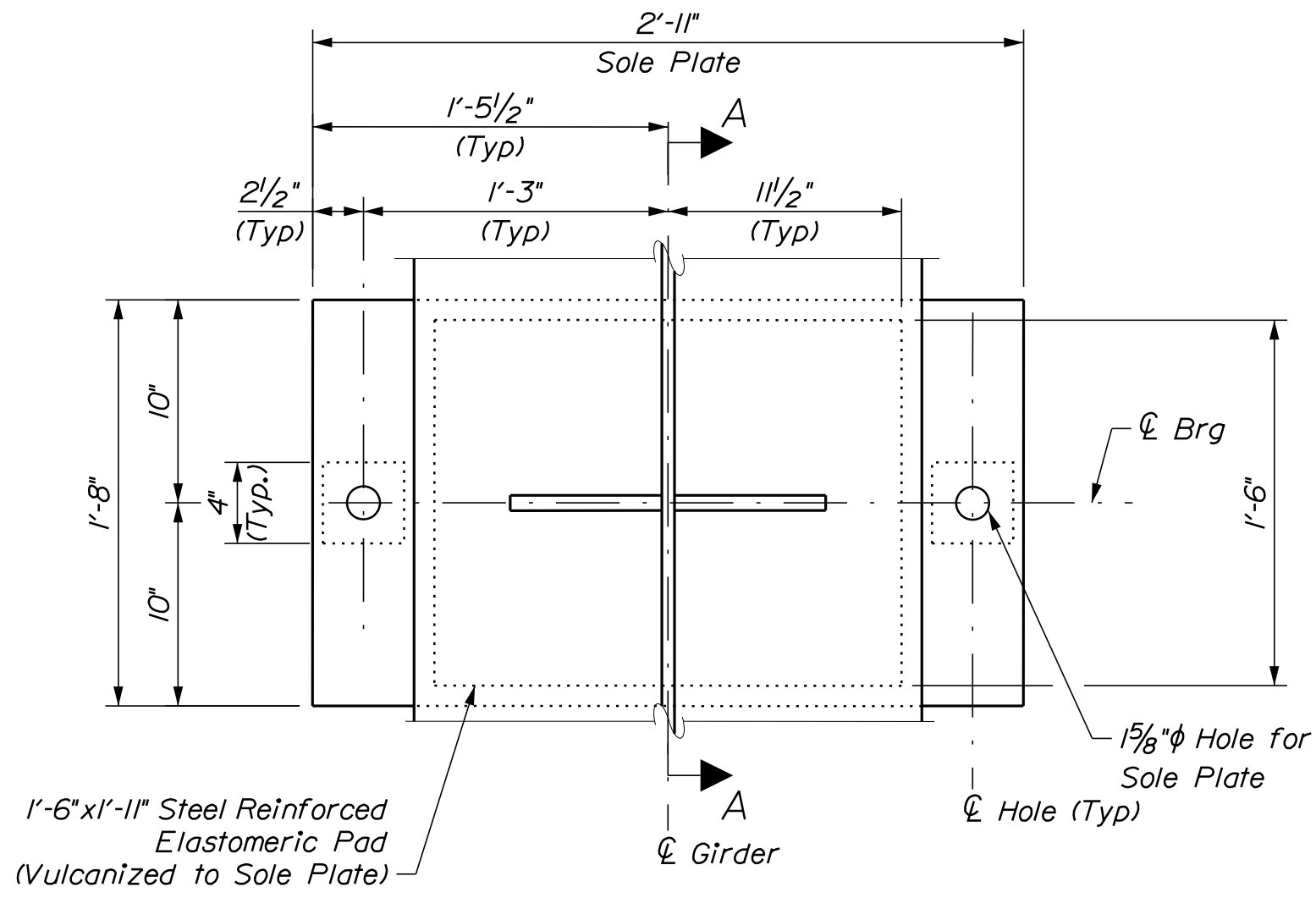
OF 76



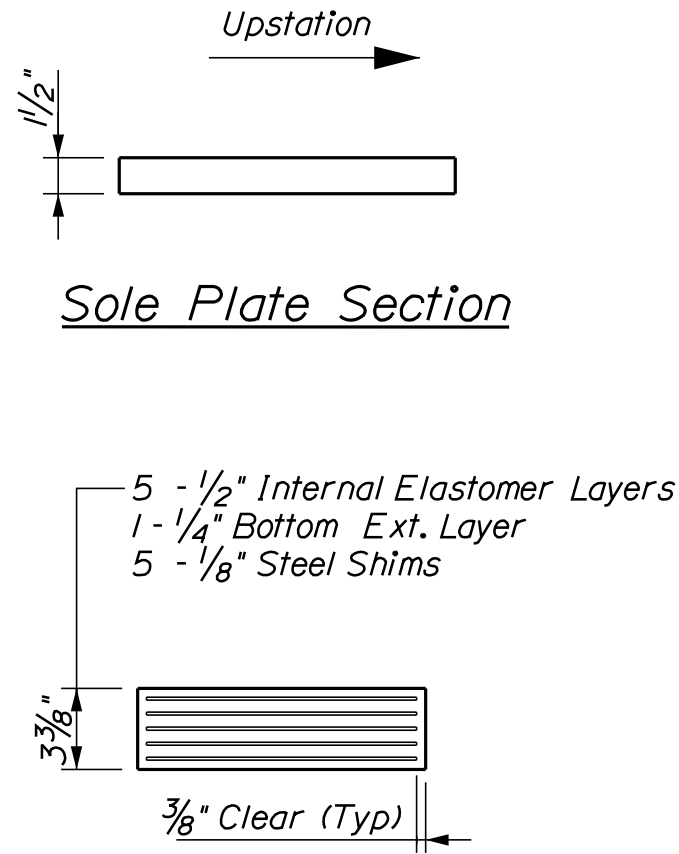
Plan



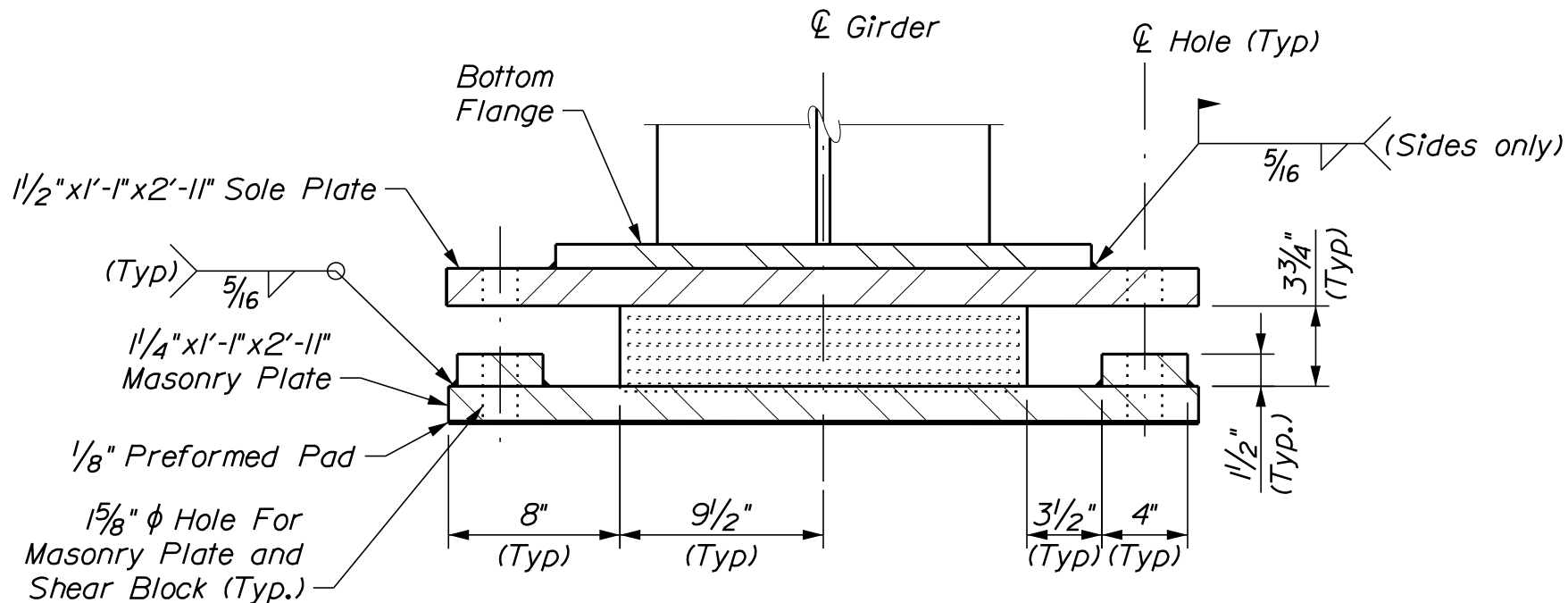
Elastomeric Pad Section



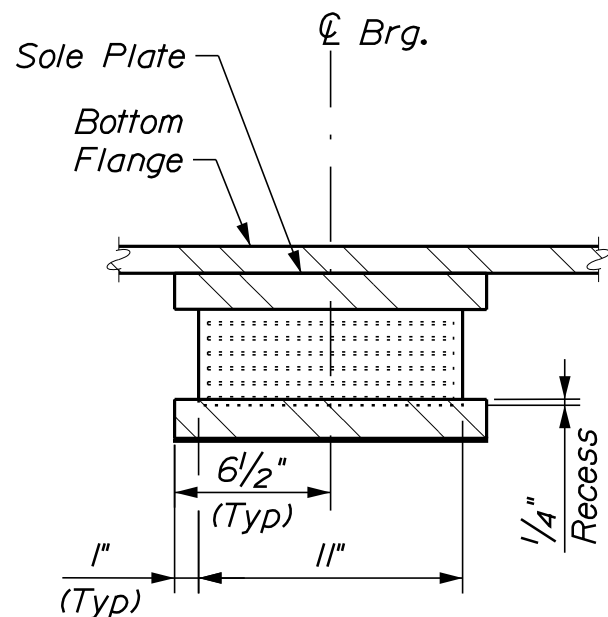
Plan



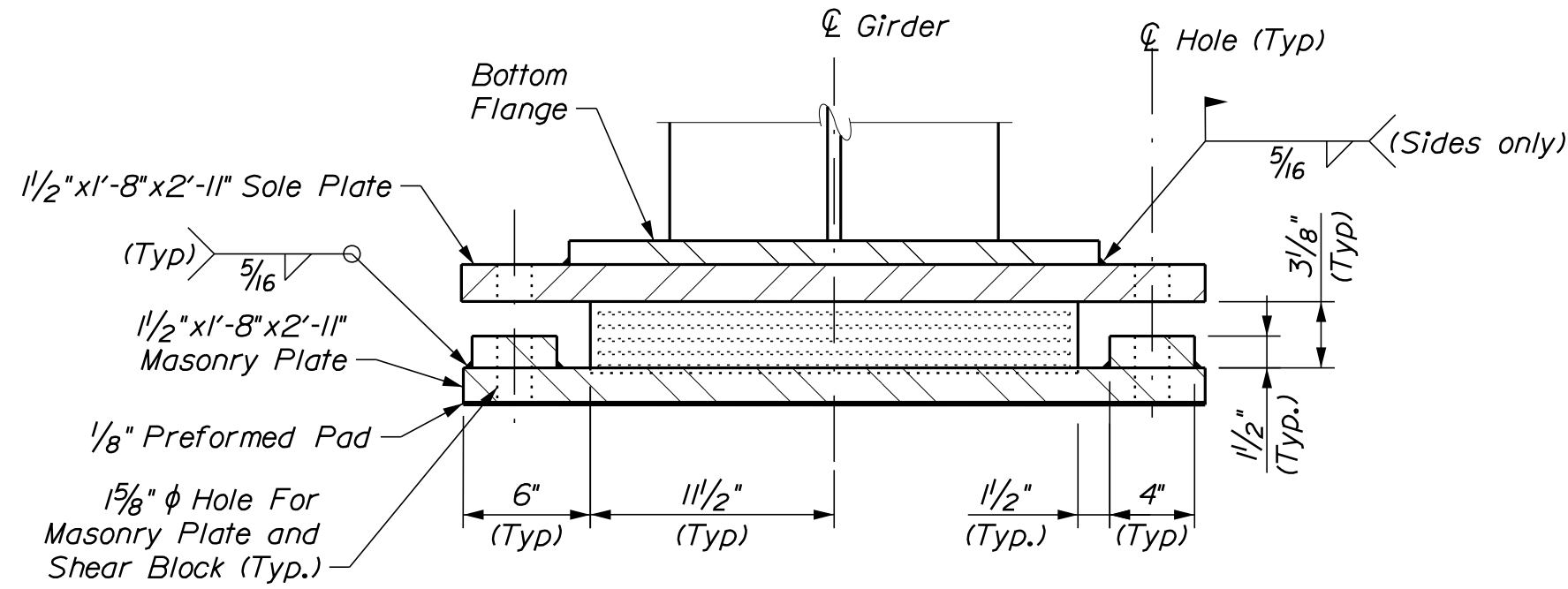
Elastomeric Pad Section



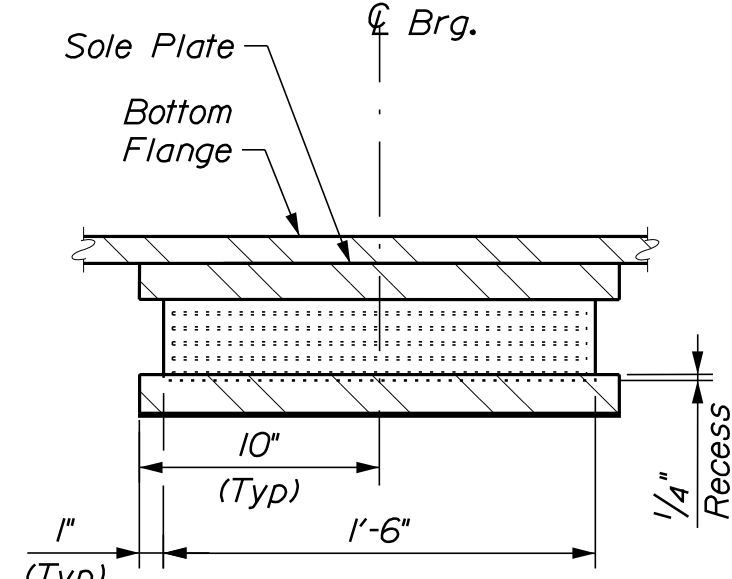
Elevation



Section A-A



Elevation

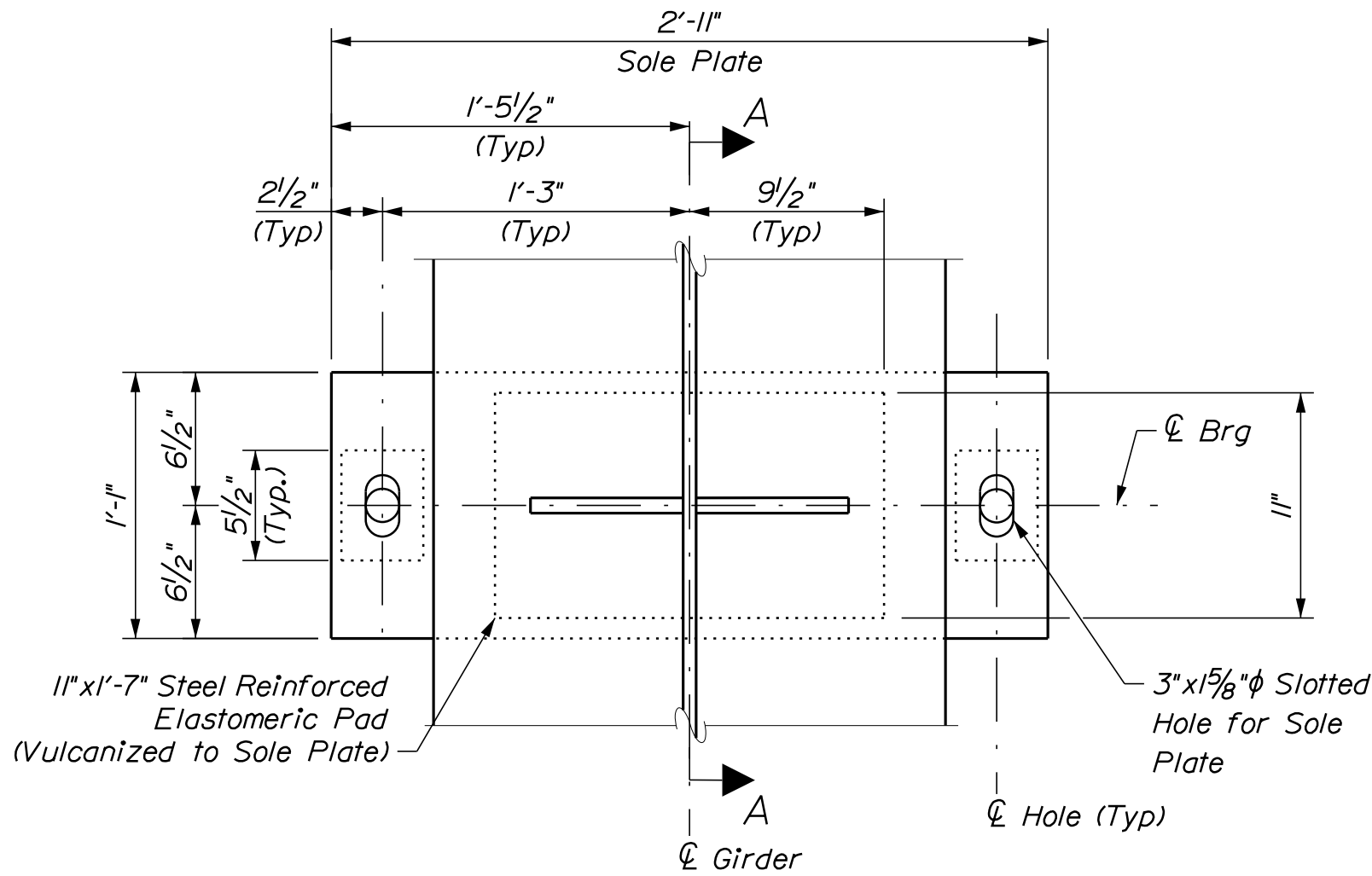


Section A-A

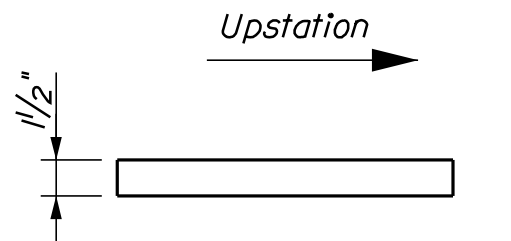
ABUTMENT I BEARING ASSEMBLY (EXPANSION)

PIER BEARING ASSEMBLY (FIXED)

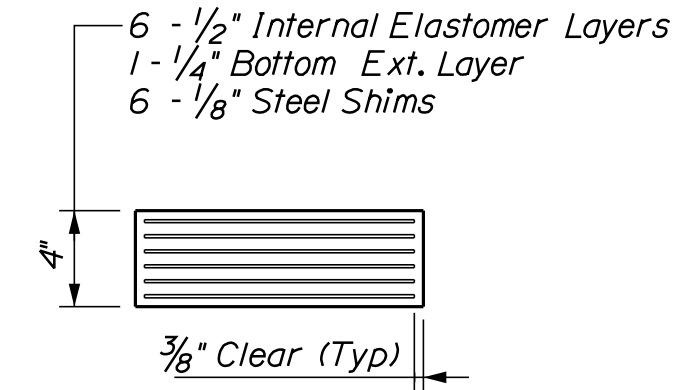
| BEARING DESIGN LOADS AND MOVEMENTS (UNFACTORED) | | | | | | | |
|---|---------------------------|-----------|-------|----------------------------|-----------------------------|-----------------|--------|
| Brg. Type | VERTICAL REACTIONS (kips) | | | Horiz. Load - Long. (kips) | Horiz. Load - Trans. (kips) | MOVEMENTS (in.) | |
| | Dead Load | Live Load | Total | | | Long. | Trans. |
| Exp. @ Abut. 1 | 101 | 68 | 169 | 9.3 | -- | 0.875 | 0 |
| Fixed @ Pier | 334 | 132 | 467 | 0 | -- | 0 | 0 |
| Exp. @ Abut. 2 | 101 | 68 | 169 | 9.3 | -- | 0.875 | 0 |



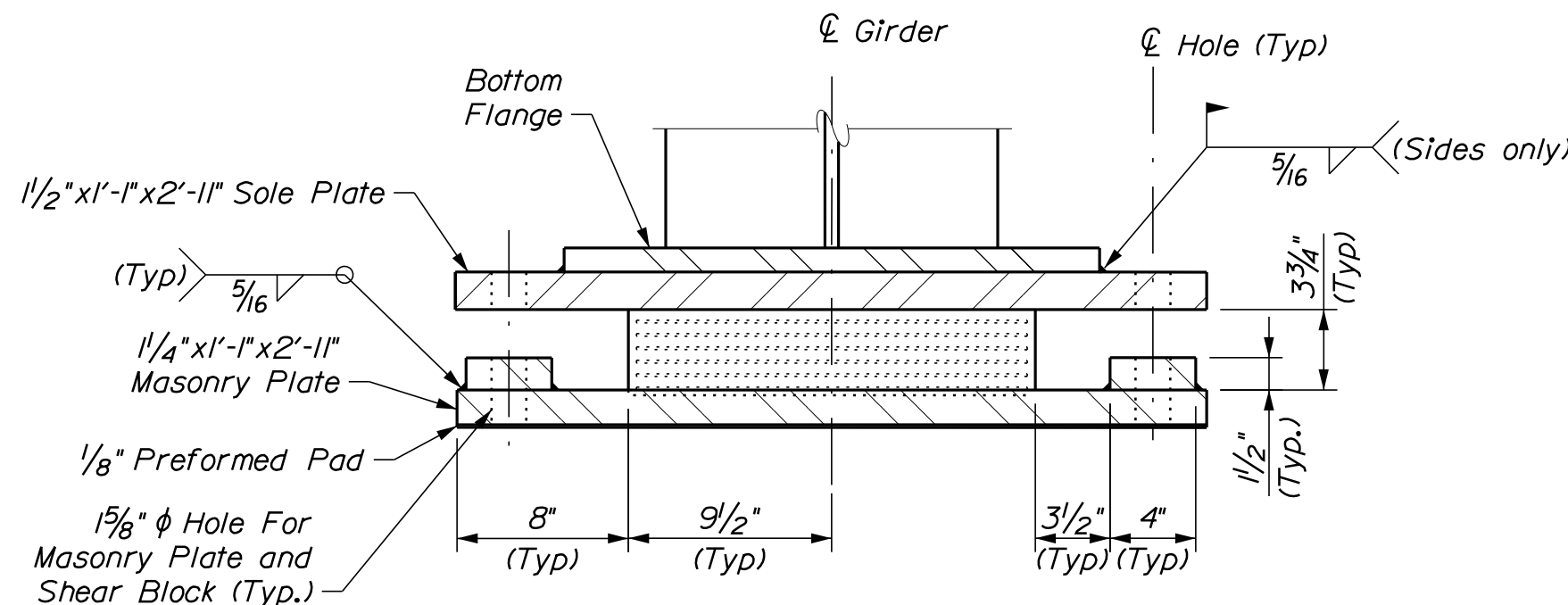
Plan



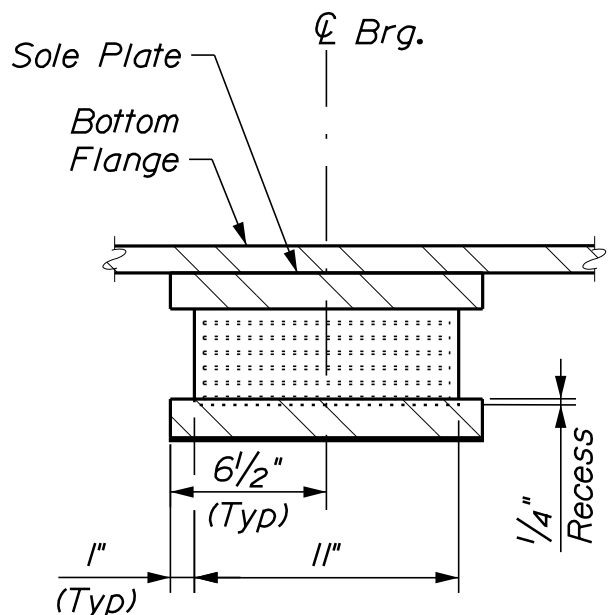
Sole Plate Section



Elastomeric Pad Section

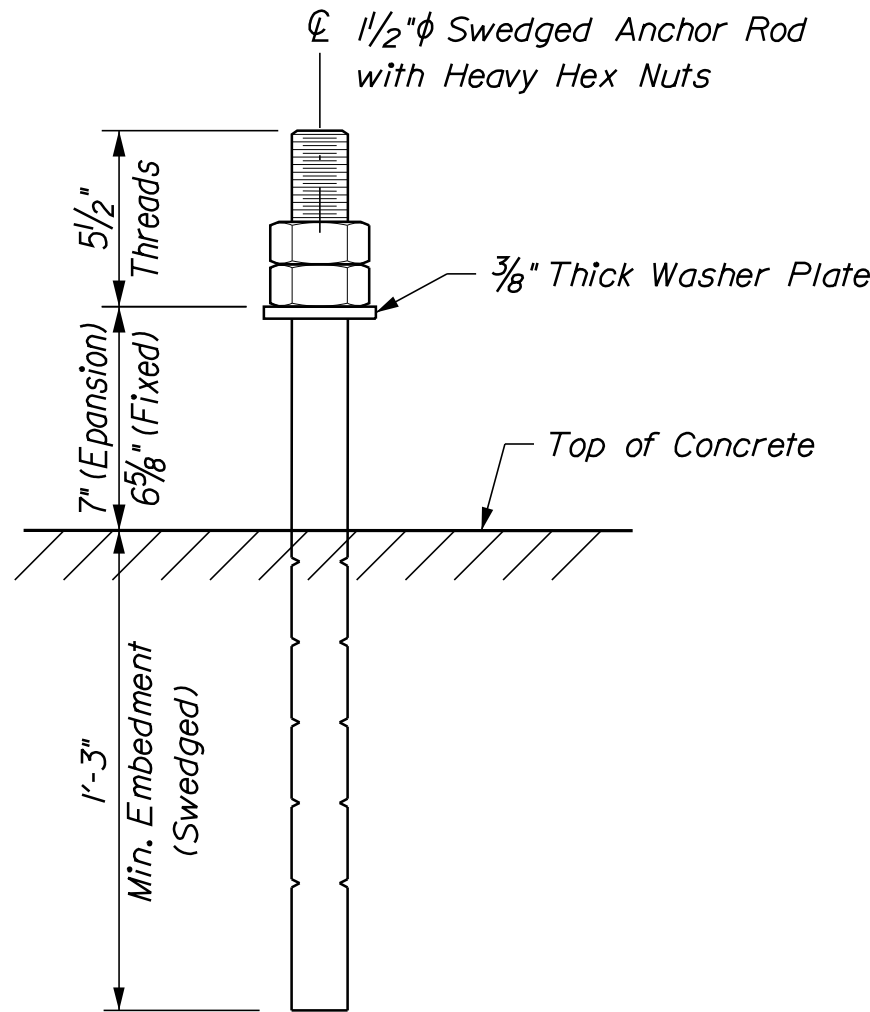


Elevation

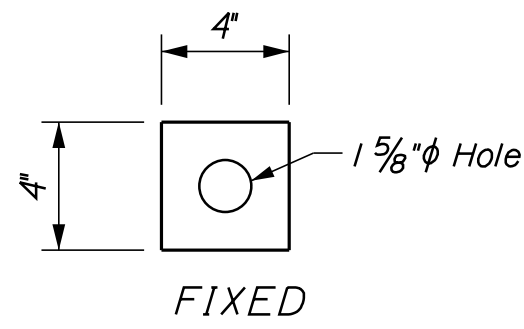


Section A-A

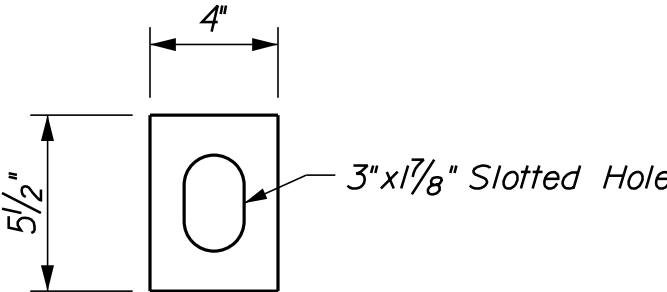
ABUTMENT 2 BEARING ASSEMBLY (EXPANSION)



ANCHOR ROD DETAIL



FIXED



EXPANSION

WASHER PLATE DETAIL

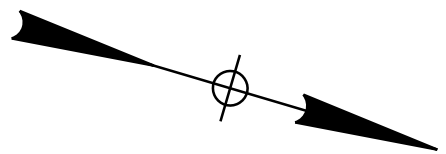
ELASTOMERIC BEARING NOTES

1. The shear modulus of the elastomer shall be 112 psi.
2. Vulcanization of the elastomer to the sole plates shall be done during the primary mold process. Sole plate shall be vulcanized to the elastomer.
3. Masonry plates, sole plates and shear blocks shall meet the requirements of ASTM A709, Grade 50 or 50W. Anchor rods shall meet the requirements of ASTM F1554, Grade 105 and shall be swedged on the embedded portion of the rod.
4. All steel located below the elastomeric pad shall be coated in accordance with Standard Specifications Section 506, Protective Coating - Steel (Thermal Spray Coating). All remaining steel shall be coated in accordance with Standard Specifications Section 506, Protective Coating - Steel (Zinc Rich Coating System). Payment for coatings for Elastomeric Bearings will be considered incidental to Item 523.5401 Laminated Elastomeric Bearings, Fixed or Item 523.5402 Laminated Elastomeric Bearings, Expansion as applicable.
5. All bearings shall be marked prior to shipping. The marks shall include the bearing location on the bridge and a direction arrow that points upstation. All marks shall be permanent and shall be visible after the bearing is installed.
6. Bearings shall be covered during shipping and at any time prior to installation that the bearings may be exposed to sunlight.
7. The superstructure may be erected when the ambient air temperature is within the range of 65 °F and 90 °F. If the ambient air temperature is outside this range, the bearings shall be reset as directed by the Resident.
8. All necessary precautions shall be taken to protect bearing components from field weld flash and spatter. Heat from welding operations shall be controlled such that steel adjacent to the elastomer does not exceed 200 °F. The temperature shall be verified by the use of temperature indicating crayons or other suitable means.
9. Upset the threads on the anchor rods after assembly of the bearing.
10. The Contractor shall not weld the girders to the sole plate until after all adjustments have been made in accordance with Standard Specification Section 523.094.
11. The "Bearing Design Load" for each bearing, as noted in Standard Specifications Subsection 523.23.4, is 467 kips. This is the total load for the Service I load combination, without impact.

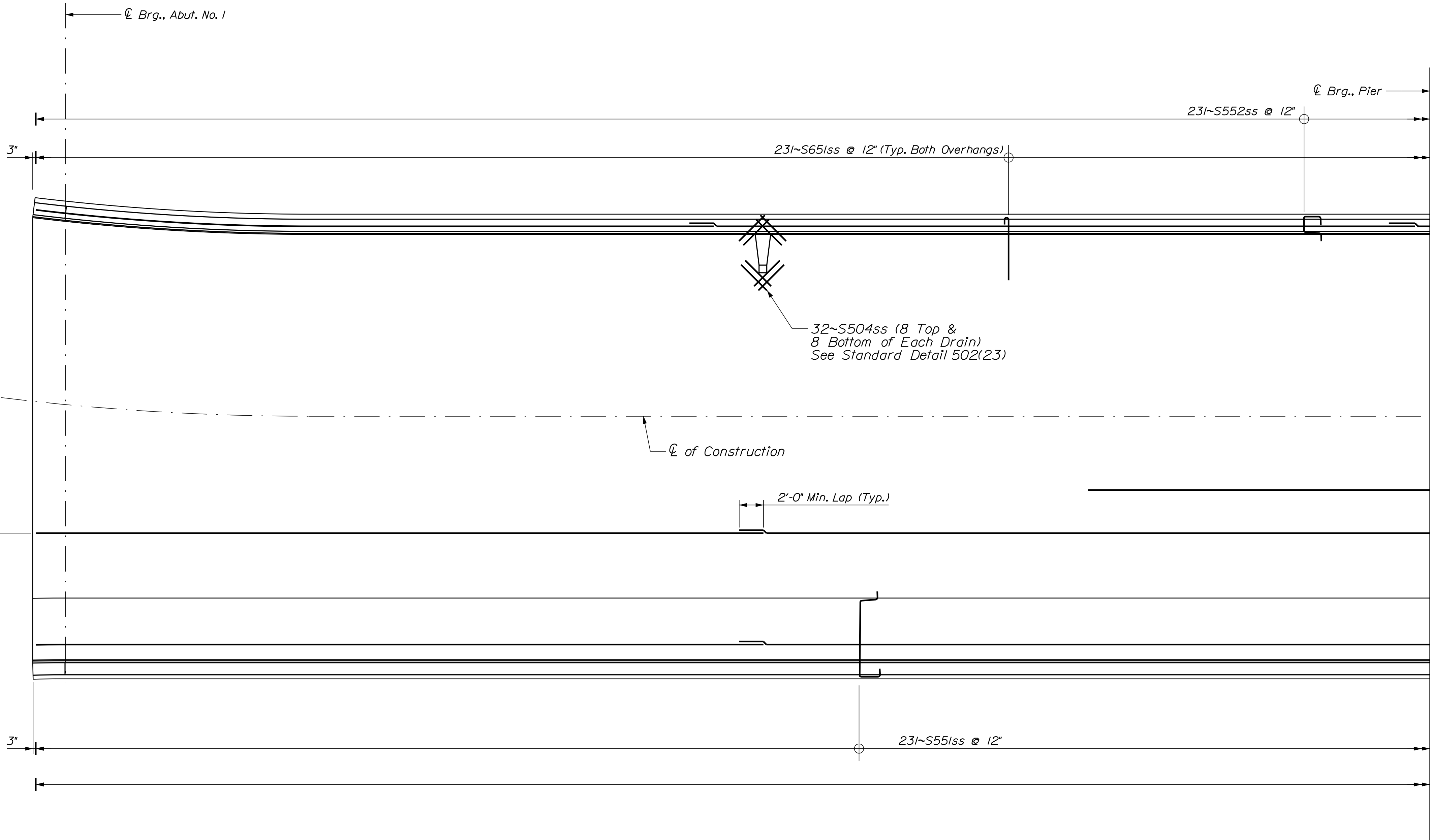


SUPERSTRUCTURE NOTES

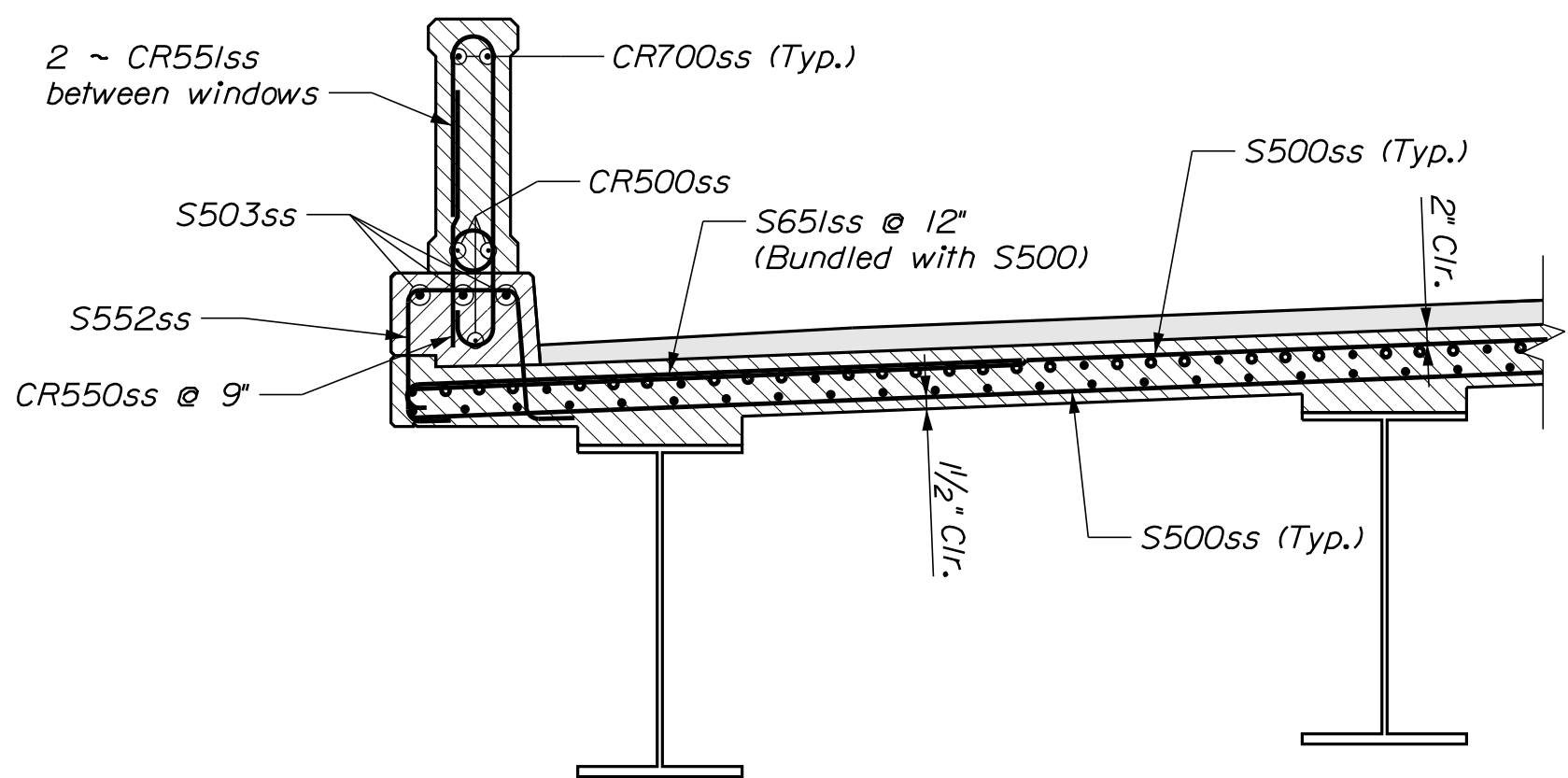
1. *The theoretical blocking used for design of the structure is 2.50 inches at the centerline of bearing of the pier and 3.625 inches at the centerline of bearing of the abutments. Refer to Standard Detail 502(03) for blocking details.*
2. *Reinforcing steel shall have a minimum concrete cover of 2 inches unless otherwise noted.*
3. *Form a one inch V-groove on the fascias at the horizontal joint between the curb and slab.*
4. *Adjust reinforcing steel to fit around the bridge drains in a manner approved by the Resident. Do not cut transverse reinforcing bars.*
5. *Unless the superstructure slab concrete is placed in one continuous operation, the initial placement shall begin at a simply supported end of the deck slab and shall terminate at the completion of a positive moment section. Successive placements shall proceed from the end of the previous placement, terminate at the completion of a positive moment section, and include two or more spans. Concrete in a placement shall be kept plastic one complete span behind the span being placed. A minimum of 5 days shall elapse between successive partial placements. The superstructure slab concrete placement sequence shall be approved by the Resident.*



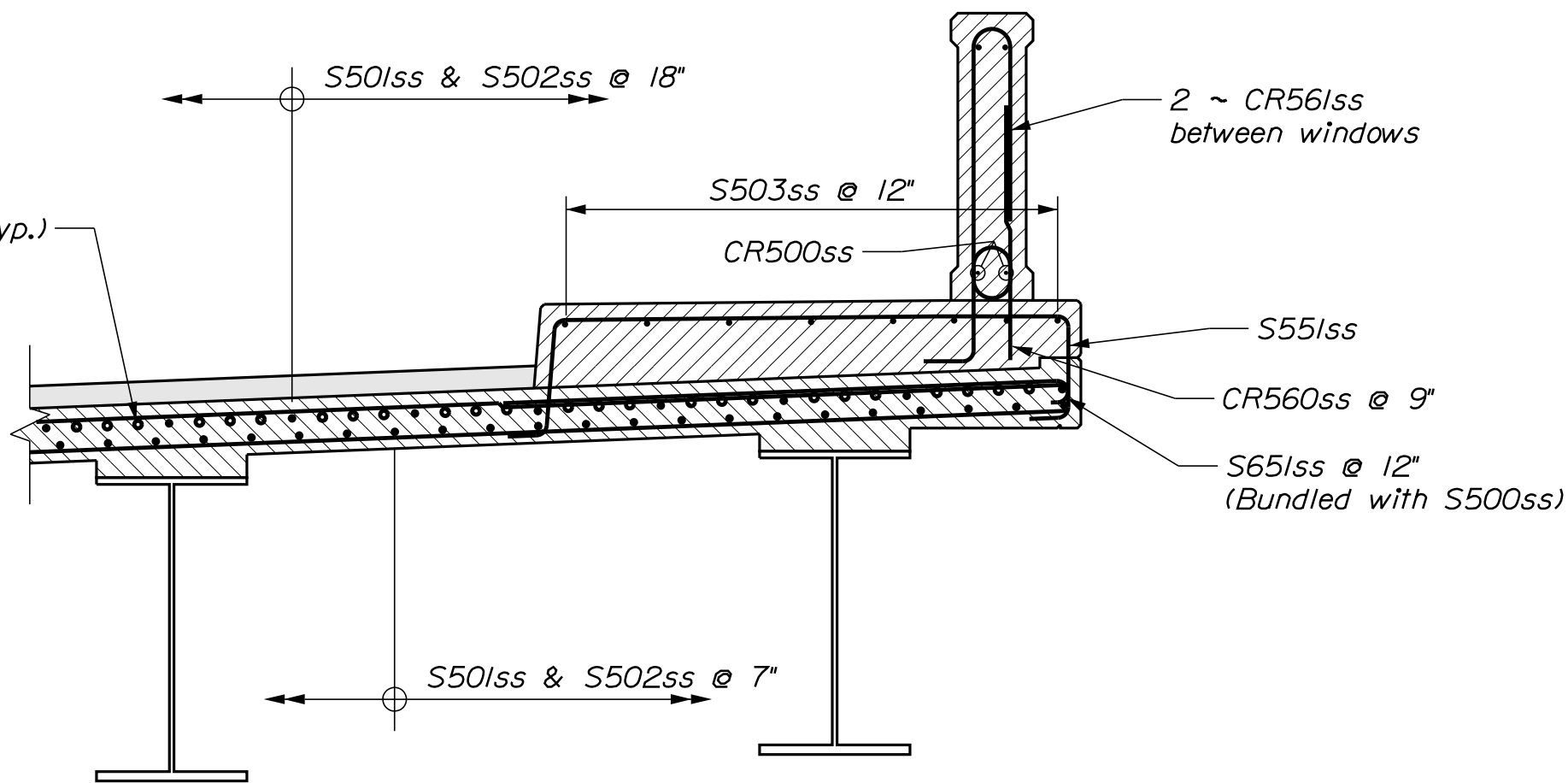
81~S501 & 27~S502 @ 18" (Top Mat)
198~S501 & 66~S502 @ 7" (Bottom Mat)



SUPERSTRUCTURE REINFORCING PLAN



S601ss (Over Pier) (Typ.)

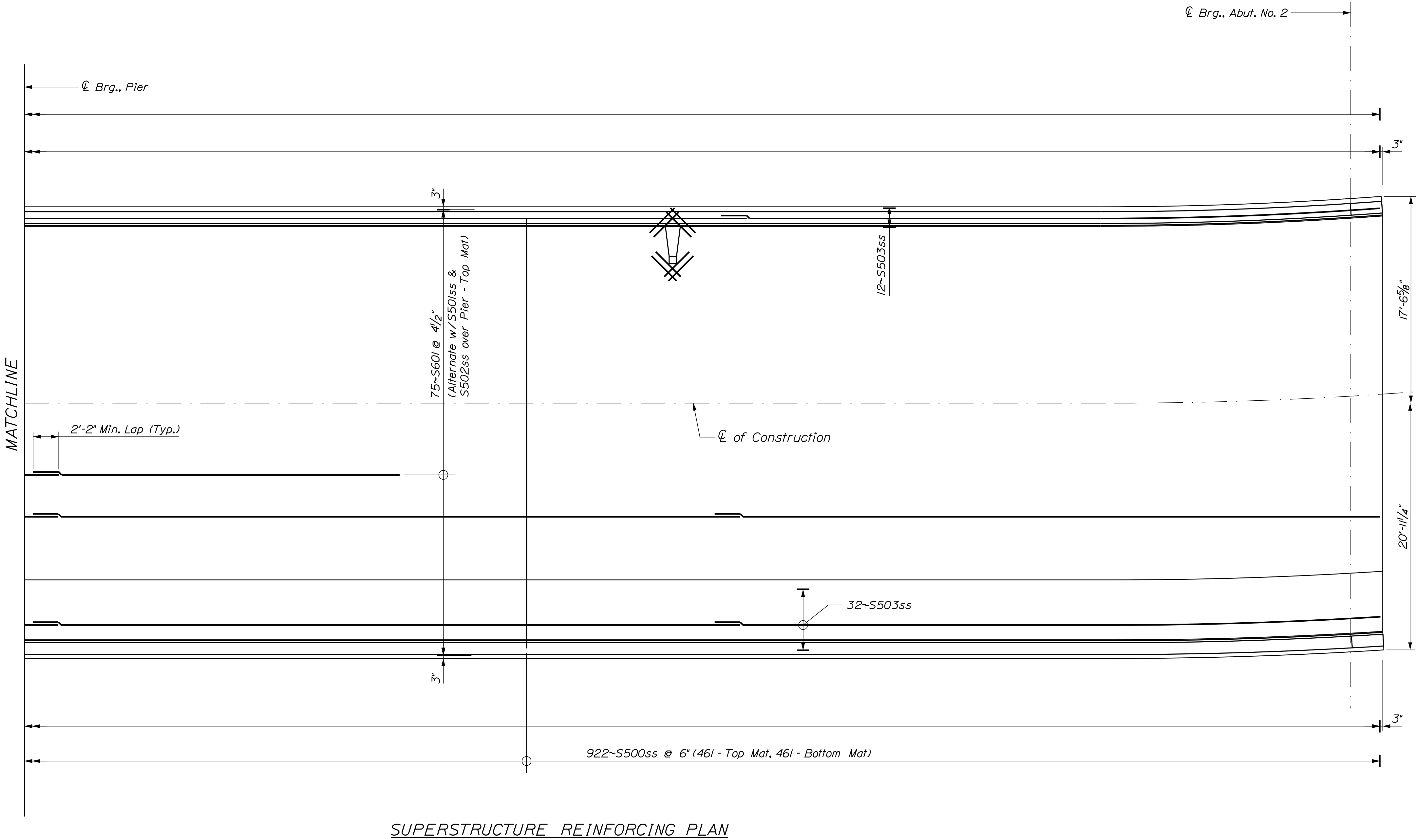
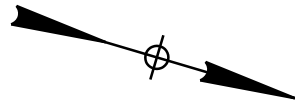


TRANSVERSE REINFORCEMENT SECTIONS



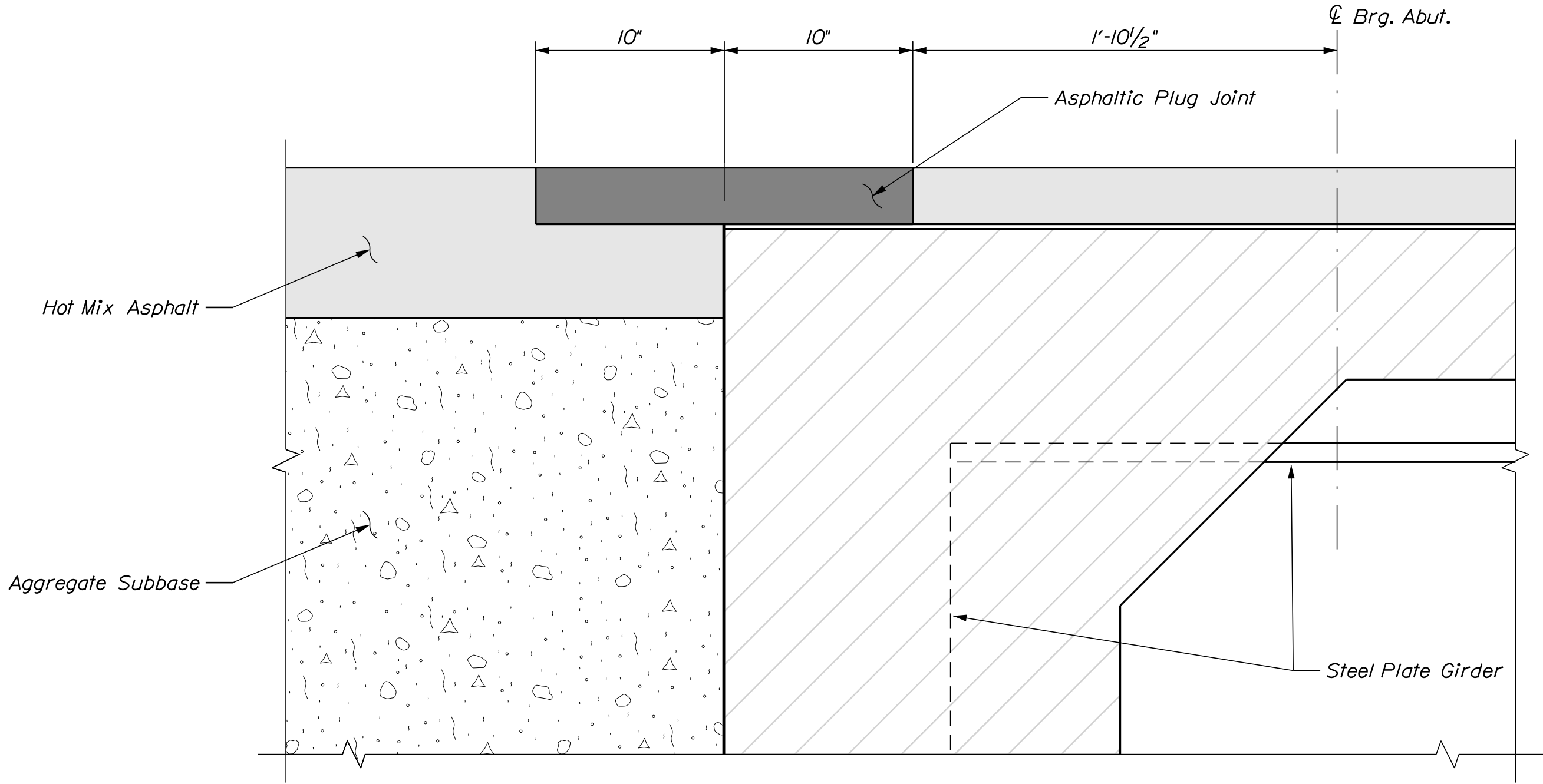
| PROJ. MANAGER | MICHAEL WIGHT | BY | DATE |
|-------------------|---------------|-----------|--------|
| DESIGNED-DETAILED | MYLENE | R. PARKER | 6/2021 |
| CHECKED-REVIEWED | C. SICHAK | C. SICHAK | 6/2021 |
| DESIGNED-DETAILED | | | |
| REVISIONS 1 | | | |
| REVISIONS 2 | | | |
| REVISIONS 3 | | | |
| REVISIONS 4 | | | |
| FIELD CHANGES | | | |

| SIGNATURE | P.E. NUMBER | DATE |
|-----------|-------------|------|
| | | |
| | | |
| | | |

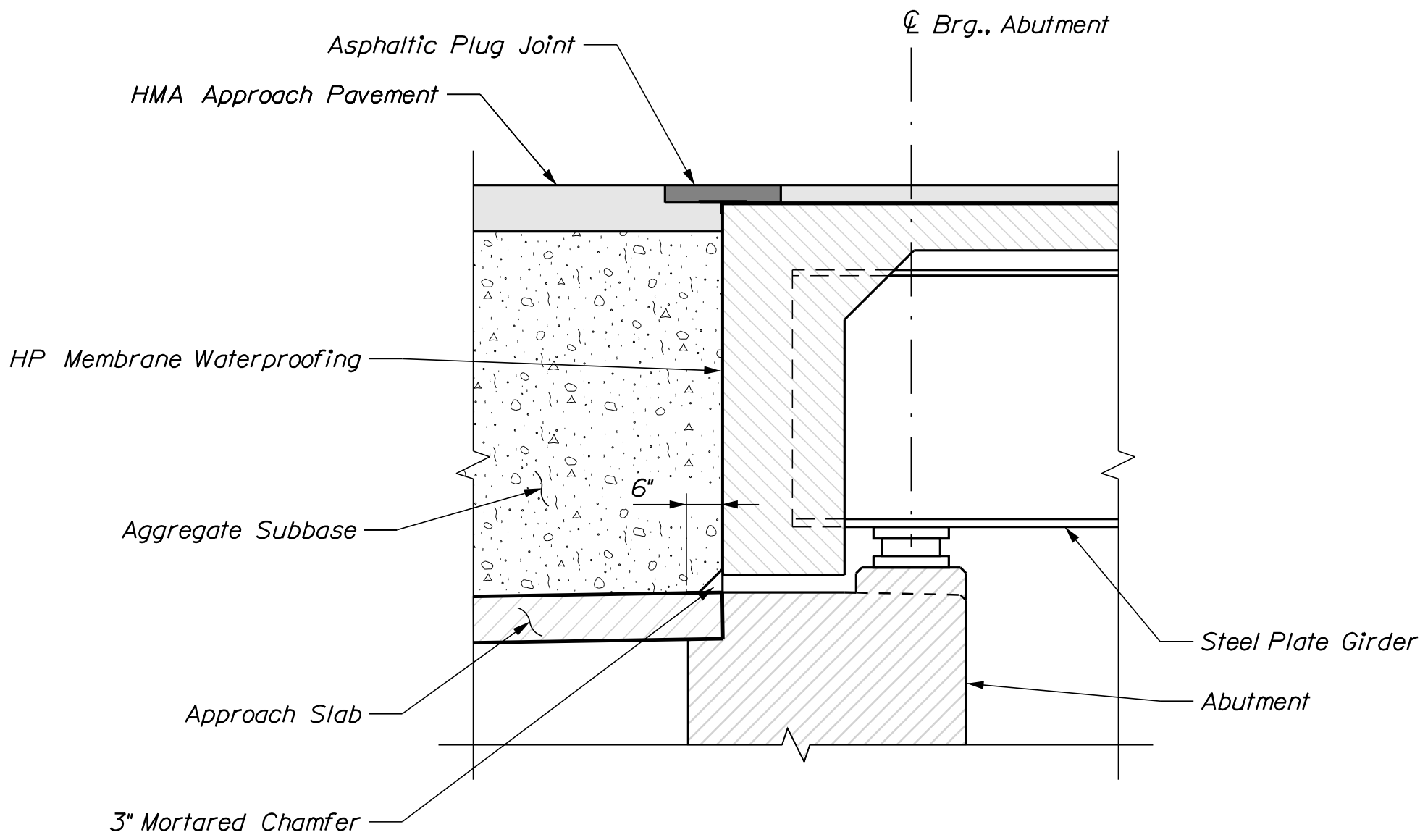


SUPERSTRUCTURE REINFORCING PLAN

| | | | | |
|---|------------------|---------------|-----------|--------|
| FARMINGTON FALLS BRIDGE SANDY RIVER CHESTERVILLE-FARMINGTON FRANKLIN COUNTY | PROJ. MANAGER | MICHAEL WIGHT | BY | DATE |
| | DESIGN DETAIL | MYLENE | R. PARKER | 6/2021 |
| | CHECKED-REVIEWED | C. SCHAK | C. SCHAK | 6/2021 |
| | DESIGN DETAIL | ED2 | | |
| | DESIGN DETAIL | ED3 | | |
| | REVISIONS 1 | | | |
| | REVISIONS 2 | | | |
| | REVISIONS 3 | | | |
| | REVISIONS 4 | | | |
| | FIELD CHANGES | | | |
| STATE OF MAINE DEPARTMENT OF TRANSPORTATION | | | | |
| 2229600 | | | | |
| BRIDGE NO. 2273 | | | | |
| WIN | | | | |
| 22296.00 | | | | |
| BRIDGE PLANS | | | | |



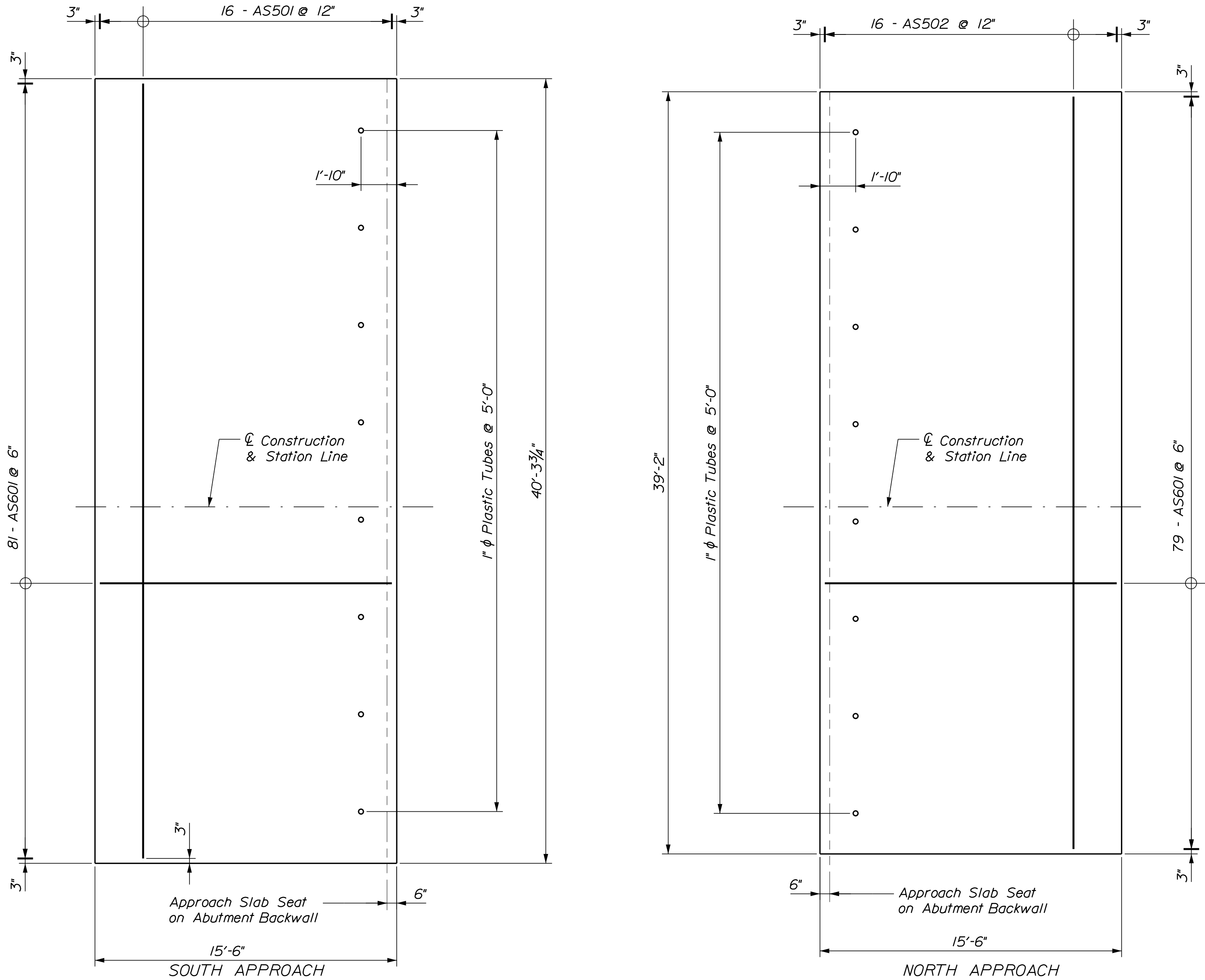
ASPHALTIC PLUG JOINT DETAIL



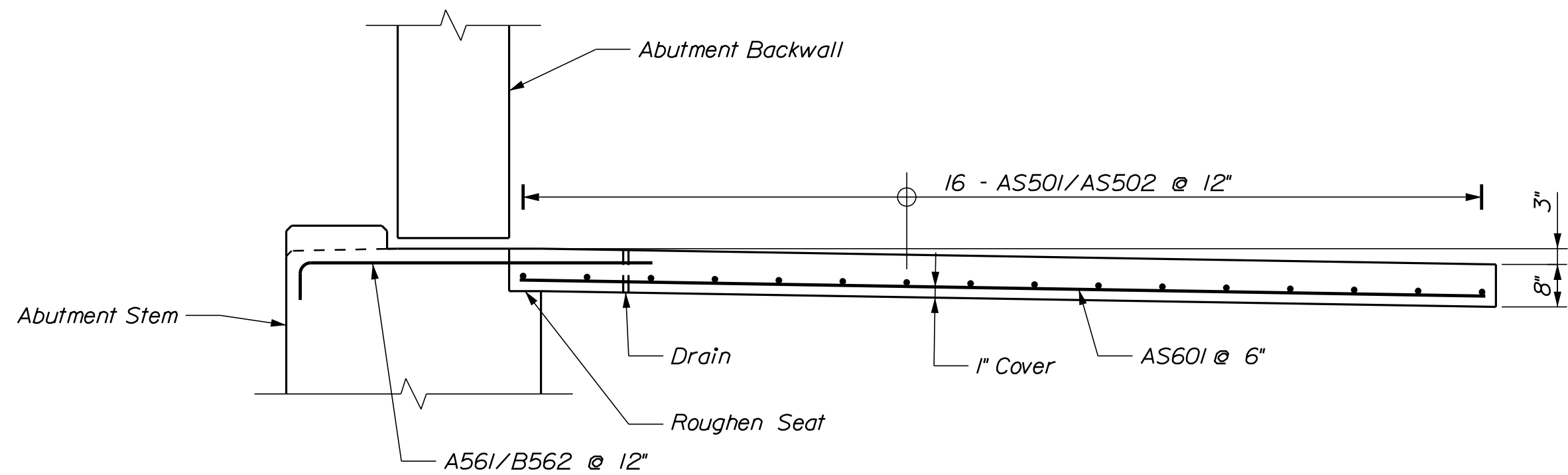
HP MEMBRANE WATERPROOFING DETAIL



| | | | | | | |
|---|--|--|--|--|--|--|
| <div> <div>FARMINGTON FALLS BRIDGE</div> <div>SANDY RIVER</div> <div>CHESTERVILLE-FARMINGTON FRANKLIN COUNTY</div> <div>SUPERSTRUCTURE DETAILS</div> </div> | <div> <div>STATE OF MAINE</div> <div>DEPARTMENT OF TRANSPORTATION</div> </div> | | <div> <div>2229600</div> <div>WIN</div> <div>22296.00</div> </div> | | <div> <div>BRIDGE NO. 2273</div> <div>BRIDGE PLANS</div> </div> | |
| | <div> <div>DESIGNED-Detailed</div> <div>CHECKED-Reviewed</div> <div>DESIGNED-Detailed</div> <div>REVISIONS 1</div> <div>REVISIONS 2</div> <div>REVISIONS 3</div> <div>REVISIONS 4</div> <div>FIELD CHANGES</div> </div> | | <div> <div>DATE</div> <div>6/2021</div> <div>6/2021</div> <div></div> <div></div> <div></div> <div></div> <div></div> </div> | | <div> <div>SIGNATURE</div> <div>P.E. NUMBER</div> <div>DATE</div> </div> | |
| | <div> <div>PROJECT MANAGER</div> <div>DESIGNED-Detailed</div> <div>CHECKED-Reviewed</div> <div>DESIGNED-Detailed</div> <div>REVISIONS 1</div> <div>REVISIONS 2</div> <div>REVISIONS 3</div> <div>REVISIONS 4</div> <div>FIELD CHANGES</div> </div> | | <div> <div>BY</div> <div>R. PARKER</div> <div>C. SICHAK</div> <div></div> <div></div> <div></div> <div></div> <div></div> </div> | | <div> <div>DATE</div> <div>6/2021</div> <div>6/2021</div> <div></div> <div></div> <div></div> <div></div> <div></div> </div> | |
| | <div> <div>SHEET NUMBER</div> <div>70</div> <div>OF 76</div> </div> | | <div> <div>2229600</div> <div>WIN</div> <div>22296.00</div> </div> | | <div> <div>BRIDGE NO. 2273</div> <div>BRIDGE PLANS</div> </div> | |



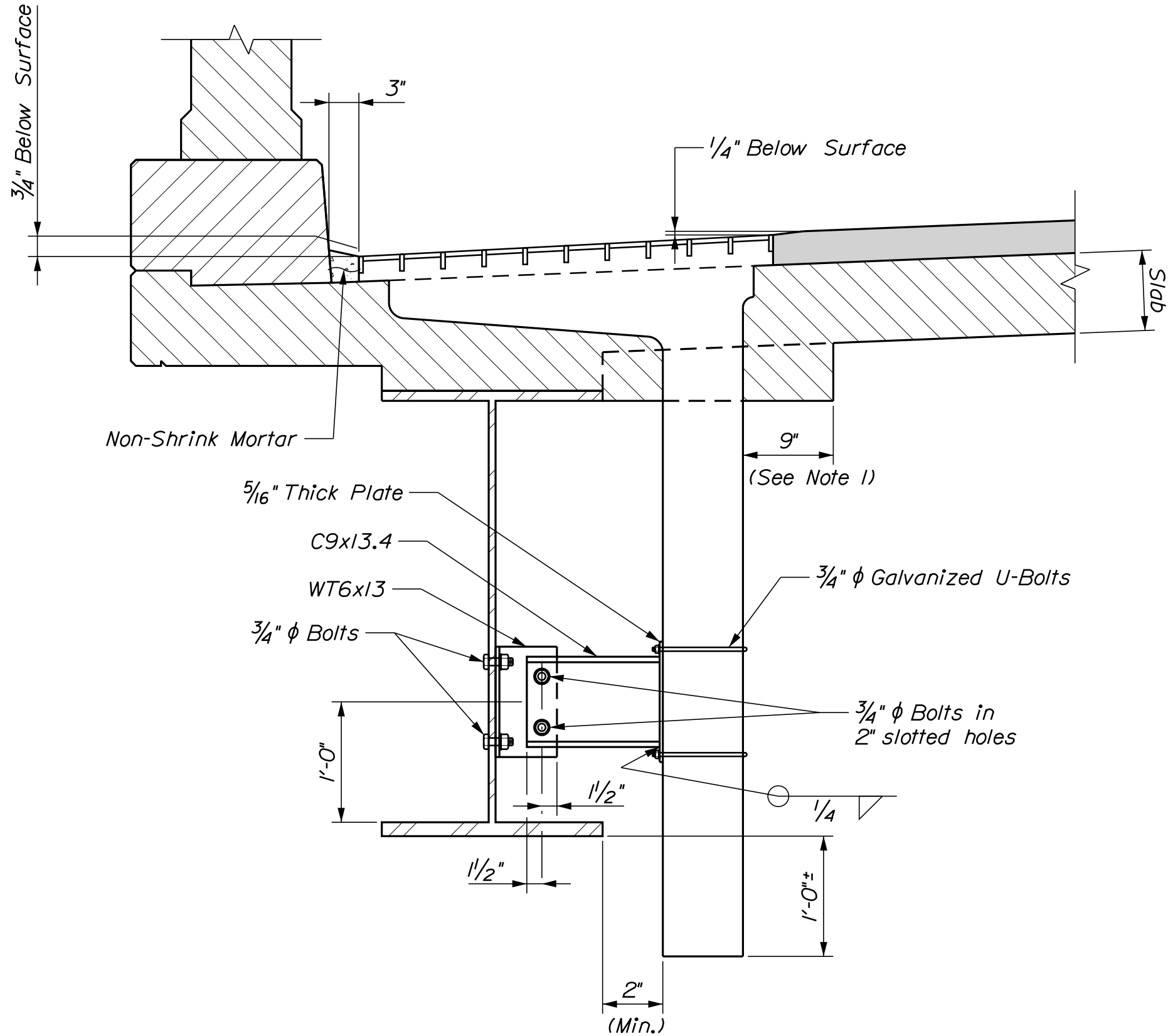
APPROACH SLAB REINFORCEMENT PLAN



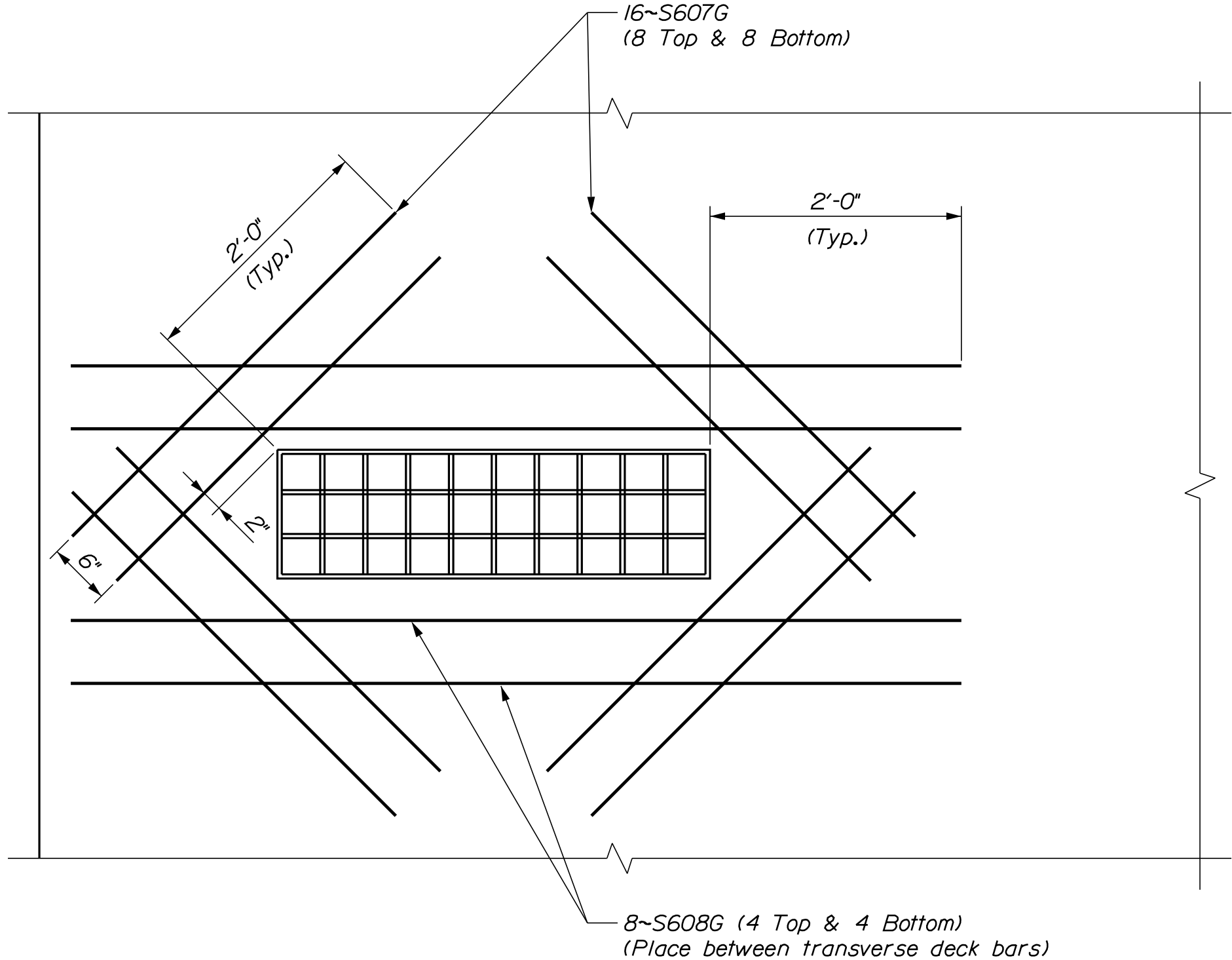
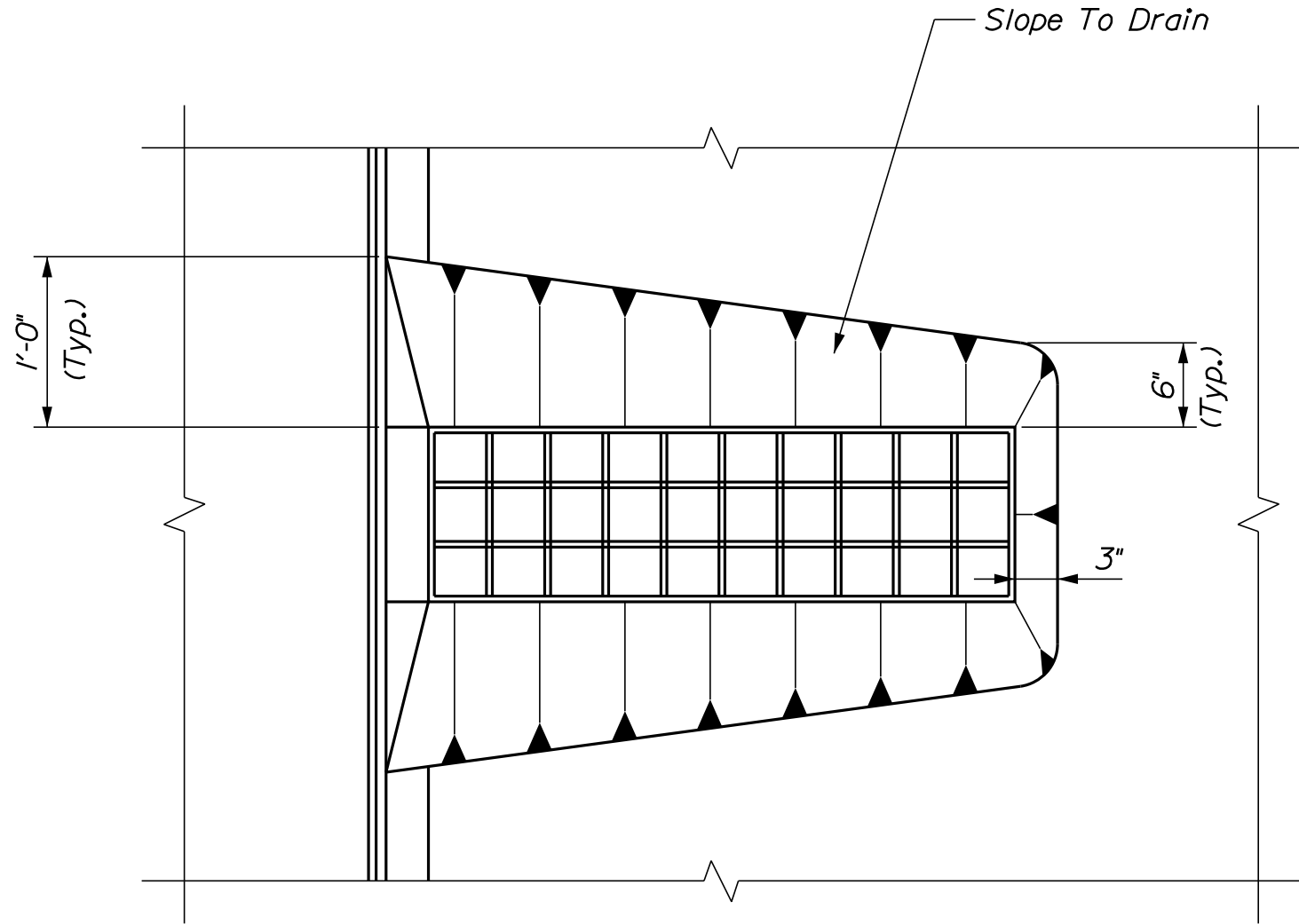
CONCRETE APPROACH SLAB SECTION



| | | | |
|---|-----------------|-----|--------------|
| STATE OF MAINE DEPARTMENT OF TRANSPORTATION 2229600 | SIGNATURE | | DATE |
| | P.E. NUMBER | | DATE |
| | BRIDGE NO. 2273 | | BRIDGE PLANS |
| 22296.00 | | WIN | 22296.00 |
| SHEET NUMBER | | 71 | |
| OF 76 | | | |



NOTE:
 1. If the minimum thickness of concrete below the drain pan is 2 inches or less, the concrete shall be thickened as shown.



DECK DRAIN REINFORCING PLAN

Deck reinforcement not shown for clarity
 Note: Cut Longitudinal Bars and Transverse Bars as Necessary (Top & Bottom)

| | | | | | | |
|---|--|--|----------------------------------|--|---------------------------------|--|
| FARMINGTON FALLS BRIDGE SANDY RIVER CHESTERVILLE-FARMINGTON FRANKLIN COUNTY | STATE OF MAINE DEPARTMENT OF TRANSPORTATION | | 2229600 WIN 22296.00 | | BRIDGE NO. 2273 BRIDGE PLANS | |
| | SHEET NUMBER 72 OF 76 | | SIGNATURE P.E. NUMBER DATE | | | |
| | BRIDGE DRAIN DETAILS | | FIELD CHANGES | | | |

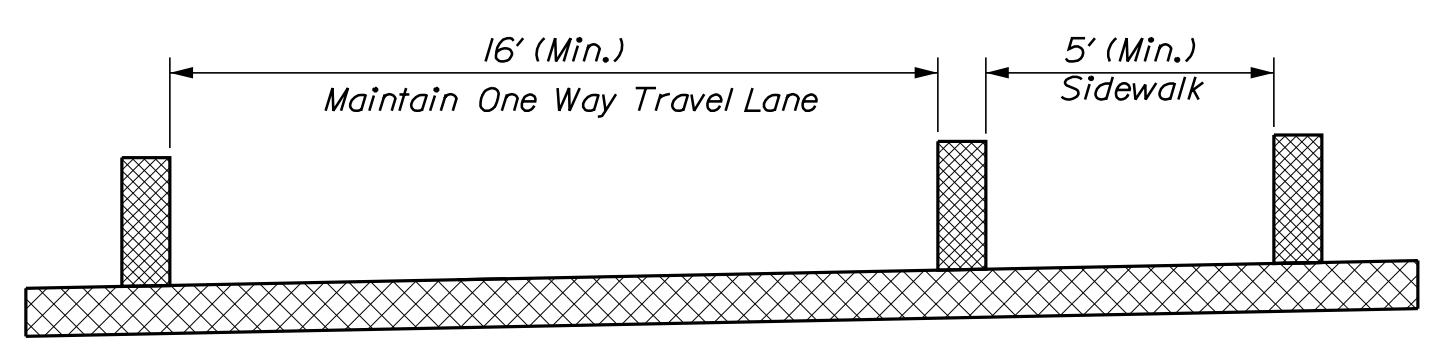
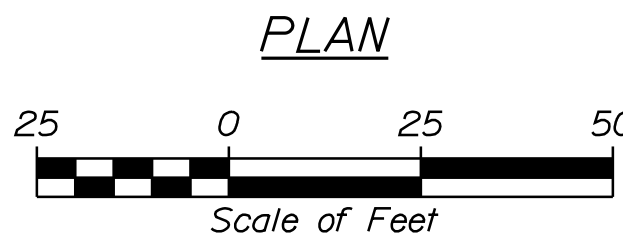
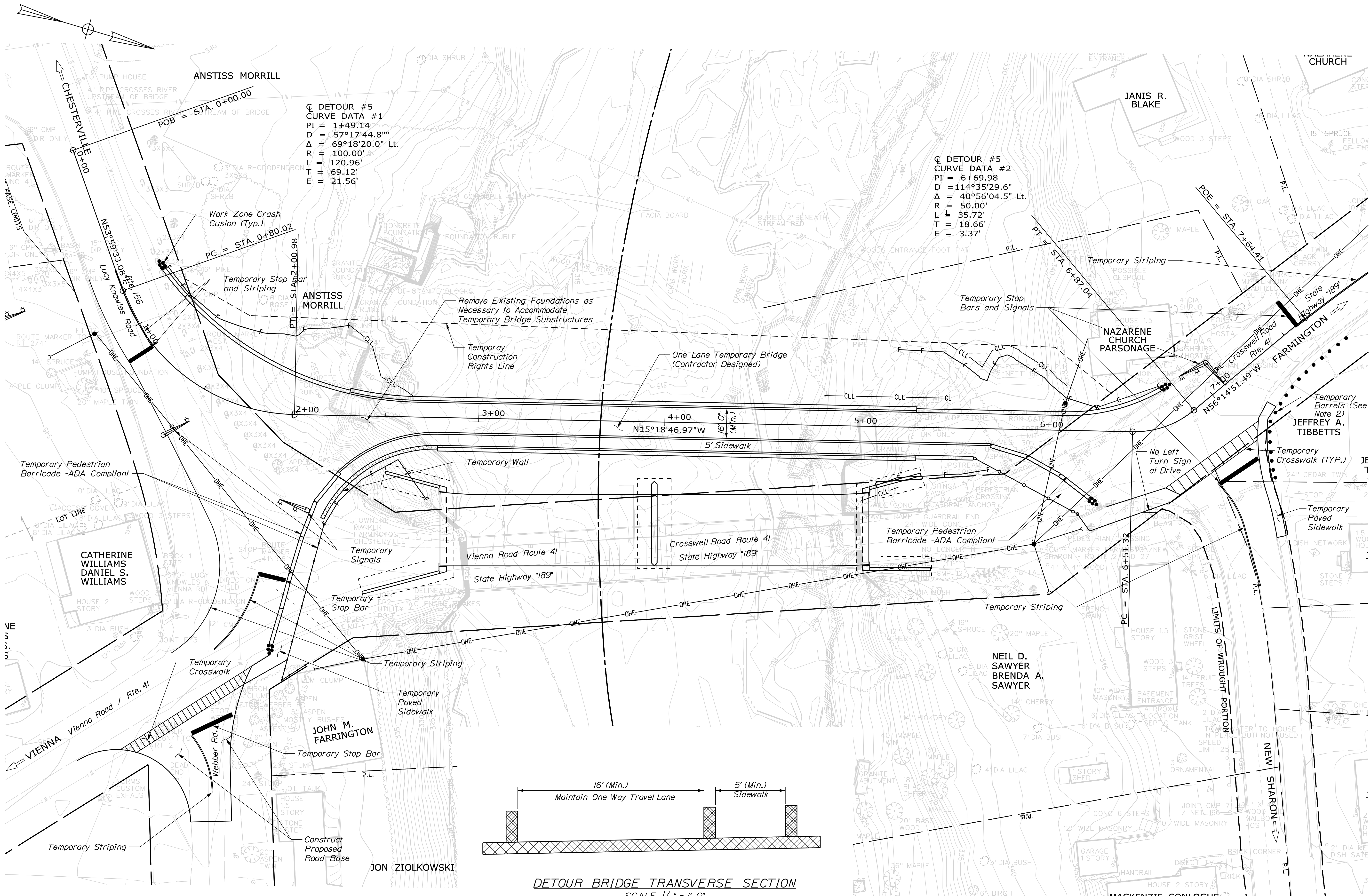
| STRAIGHT BARS | | | | | | | | BENT BARS | | | | | | | | | | | | | | | |
|----------------------------------|------|---------|-------------------------------|--------------------------------------|------|---------|---------------------------|----------------------------------|------|---------|------|--------|--------|-------|--------|-------|-------|--------|--------|-------|---|------------------------------|--|
| MARK | QTY. | LENGTH | LOCATION | MARK | QTY. | LENGTH | LOCATION | MARK | QTY. | LENGTH | TYPE | A | B | C | D | E | F | G | H | O | R | LOCATION | |
| ABUTMENT NO. 1 - PLAIN BAR | | | | ABUTMENT NO. 2 - PLAIN BAR | | | | ABUTMENT NO. 1 - PLAIN BAR | | | | | | | | | | | | | | | |
| A501 | 30 | 50'-1" | Footing | B501 | 26 | 36'-0" | Footing | A550 | 54 | 10'-1" | L | 0'-10" | 9'-3" | - | - | - | - | - | - | - | - | Footing Hook | |
| A502 | 38 | 52'-5" | Footing | B502 | 34 | 50'-4" | Footing | A551 | 89 | 12'-1" | L | 0'-10" | 11'-3" | - | - | - | - | - | - | - | - | Footing Hook | |
| A503 | 30 | 37'-7" | Footing | B503 | 26 | 27'-5" | Footing | A552 | 41 | 4'-10" | S | 0'-0" | 0'-10" | 3'-2" | 0'-10" | - | - | 0'-0" | - | - | - | Bridge Seat | |
| A504 | 46 | 15'-11" | Breastwall Vert. | B504 | 45 | 12'-0" | Breastwall Vert. | A553 | 89 | 3'-4" | S | 0'-0" | 0'-10" | 1'-8" | 0'-10" | - | - | 0'-0" | - | - | - | Wing U-bars | |
| A505 | 42 | 24'-5" | Breastwall Horiz. | B505 | 34 | 19'-5" | Breastwall Horiz. | A557 | 8 | 6'-0" | S | 0'-0" | 0'-10" | 3'-8" | 1'-6" | - | - | 0'-0" | - | - | - | Parapet U-bars | |
| A506 | 42 | 20'-2" | Breastwall Horiz. | B506 | 34 | 23'-9" | Breastwall Horiz. | A558 | 55 | 5'-6" | V | - | - | - | 2'-3" | 3'-3" | - | - | 3'-1" | - | - | Parapet Horiz. | |
| A507 | 41 | 20'-6" | Wing Vert. - SW | B507 | 44 | 18'-7" | Wing Vert. - NW | A559 | 9 | 6'-6" | S | 0'-0" | 0'-10" | 3'-8" | 2'-0" | - | - | 0'-0" | - | - | - | Parapet U-bars | |
| A508 | 54 | 29'-9" | Wing Horiz. - SW | B508 | 46 | 28'-3" | Wing Horiz. - NW | A561 | 41 | 5'-8" | L | 0'-7" | 5'-1" | - | - | - | - | - | - | - | - | Stem to Approach Slab | |
| A510 | 56 | 21'-10" | Wing Vert. - SE | B510 | 50 | 19'-10" | Wing Vert. - NE | A562 | 163 | 5'-4" | S | 0'-0" | 1'-2" | 3'-0" | 1'-2" | - | - | 0'-0" | - | - | - | Footing U-bars | |
| A511 | 56 | 20'-11" | Wing. Horiz. - SE | B511 | 48 | 32'-5" | Wing. Horiz. - NE | | | | | | | | | | | | | | | | |
| A512 | 56 | 21'-0" | Wing. Horiz. - SE | B512 | 10 | 9'-11" | Parapet Vertical | A950 | 61 | 10'-10" | L | 1'-7" | 9'-3" | - | - | - | - | - | - | - | - | Footing Hook | |
| A513 | 12 | 10'-0" | Parapet Vertical | | | | | A951 | 111 | 12'-10" | L | 1'-7" | 11'-3" | - | - | - | - | - | - | - | - | Footing Hook | |
| | | | | B601 | 84 | 11'-6" | Footing | | | | | | | | | | | | | | | | |
| A701 | 109 | 13'-6" | Footing | B602 | 60 | 11'-4" | Breastwall Vert. | ABUTMENT NO. 1 - STAINLESS STEEL | | | | | | | | | | | | | | | |
| A702 | 54 | 17'-0" | Footing | B603 | 84 | 19'-10" | Wing Vert. NE & NW | A554ss | 15 | 9'-4" | S | 0'-0" | 2'-10" | 3'-8" | 2'-10" | - | - | 0'-0" | - | - | - | Pedestal | |
| A703 | 61 | 15'-3" | Breastwall Vert. | | | | | A555ss | 15 | 6'-11" | S | 0'-0" | 2'-10" | 1'-3" | 2'-10" | - | - | 0'-0" | - | - | - | Pedestal | |
| | | | | B701 | 121 | 15'-6" | Footing | A556ss | 12 | 10'-4" | H | 0'-3" | 1'-3" | 3'-8" | 1'-3" | 3'-8" | - | 0'-3" | - | - | - | Pedestal | |
| A901 | 135 | 13'-6" | Footing | | | | | ABUTMENT NO. 2 - PLAIN BAR | | | | | | | | | | | | | | | |
| A902 | 71 | 17'-0" | Footing | B801 | 84 | 11'-6" | Footing | B551 | 139 | 10'-1" | L | 0'-10" | 9'-3" | - | - | - | - | - | - | - | - | Footing Hook | |
| A903 | 24 | 20'-6" | Wing Vert. - SW | | | | | B552 | 40 | 4'-10" | S | 0'-0" | 0'-10" | 3'-2" | 0'-10" | - | - | 0'-0" | - | - | - | Bridge Seat | |
| A904 | 33 | 21'-10" | Wing Vert. - SE | PIER - PLAIN BAR | | | | B553 | 84 | 3'-4" | S | 0'-0" | 0'-10" | 1'-8" | 0'-10" | - | - | 0'-0" | - | - | - | Wing U-bars | |
| SUPERSTRUCTURE - STAINLESS STEEL | | | | P502 | 6 | 40'-4" | Breastwall Horiz. | B557 | 8 | 5'-9" | S | 0'-0" | 0'-10" | 3'-8" | 1'-3" | - | - | 0'-0" | - | - | - | Parapet U-bars | |
| S500ss | 922 | 37'-10" | Transv. Deck & End Diaphragm | P503 | 6 | 39'-4" | Breastwall Horiz. | B558 | 23 | 5'-1" | V | - | - | - | 1'-10" | 3'-3" | - | - | 3'-1" | - | - | Parapet Horiz. | |
| S501ss | 279 | 60'-0" | Longitudinal Deck | P504 | 6 | 38'-4" | Breastwall Horiz. | B559 | 9 | 5'-4" | S | 0'-0" | 0'-10" | 3'-8" | 0'-10" | - | - | 0'-0" | - | - | - | Parapet U-bars | |
| S502ss | 93 | 58'-0" | Longitudinal Deck | P505 | 6 | 37'-4" | Breastwall Horiz. | B560 | 24 | 8'-4" | S | 0'-0" | 2'-5" | 3'-8" | 2'-3" | - | - | 0'-0" | - | - | - | Parapet Horiz. | |
| S503ss | 44 | 60'-0" | Longitudinal Sidewalk Curb | P506 | 6 | 36'-4" | Breastwall Horiz. | B562 | 40 | 5'-8" | L | 0'-7" | 5'-1" | - | - | - | - | - | - | - | - | Stem to Approach Slab | |
| S504ss | 32 | 3'-0" | Bridge Drain | P507 | 4 | 35'-4" | Breastwall Horiz. | B563 | 157 | 5'-4" | S | 0'-0" | 1'-2" | 3'-0" | 1'-2" | - | - | 0'-0" | - | - | - | Footing U-bars | |
| S505ss | 50 | 5'-2" | Vertical Backwall | P508 | 4 | 34'-9" | Breastwall Horiz. | | | | | | | | | | | | | | | | |
| S506ss | 50 | 5'-10" | Vertical Backwall | P509 | 4 | 34'-1" | Breastwall Horiz. | B851 | 144 | 10'-7" | L | 1'-4" | 9'-3" | - | - | - | - | - | - | - | - | Footing Hook | |
| S507ss | 42 | 4'-3" | Vertical Backwall | | | | | | | | | | | | | | | | | | | | |
| S508ss | 42 | 4'-10" | Vertical Backwall | P801 | 46 | 49'-0" | Top, Mid., Bot. - Footing | ABUTMENT NO. 2 - STAINLESS STEEL | | | | | | | | | | | | | | | |
| S509ss | 14 | 39'-6" | Horizontal Backwall (Abut. 1) | | | | | B554ss | 15 | 9'-4" | S | 0'-0" | 2'-10" | 3'-8" | 2'-10" | - | - | 0'-0" | - | - | - | Pedestal | |
| S510ss | 14 | 38'-9" | Horizontal Backwall (Abut. 2) | P901 | 120 | 19'-6" | Transverse Footing | B555ss | 15 | 6'-11" | S | 0'-0" | 2'-10" | 1'-3" | 2'-10" | - | - | 0'-0" | - | - | - | Pedestal | |
| | | | | P902 | 193 | 21'-6" | Breastwall Vert. | B556ss | 12 | 10'-1" | H | 0'-3" | 1'-3" | 3'-8" | 1'-3" | 3'-8" | - | 0'-3" | - | - | - | Pedestal | |
| S601ss | 75 | 60'-0" | Longitudinal Deck over Pier | | | | | SUPERSTRUCTURE - STAINLESS STEEL | | | | | | | | | | | | | | | |
| S602ss | 75 | 60'-0" | Longitudinal Deck over Pier | TEXAS CLASSIC RAIL - STAINLESS STEEL | | | | S551ss | 231 | 10'-11" | SC | 0'-10" | 1'-7" | 6'-3" | 1'-3" | - | - | 0'-10" | - | 6'-4" | - | Sidewalk StIRRUP | |
| APPROACH SLAB - PLAIN BAR | | | | CR500ss | 32 | 30'-0" | Rail Bot. & Curb | S552ss | 231 | 5'-7" | SC | 0'-10" | 1'-4" | 1'-3" | 1'-4" | - | - | 0'-10" | - | 1'-4" | - | Curb StIRRUP | |
| AS501 | 16 | 39'-9" | Approach Slab | CR501ss | 40 | 5'-3" | Nose/Post | S553ss | 81 | 5'-0" | W | - | - | - | 1'-8" | 1'-8" | 1'-8" | - | 1'-2" | - | - | Backwall | |
| AS502 | 16 | 38'-8" | Approach Slab | CR502ss | 4 | 2'-2" | Nose | | | | | | | | | | | | | | | | |
| | | | | CR503ss | 4 | 2'-0" | Nose | S651ss | 462 | 5'-10" | C | 0'-8" | 5'-2" | 0'-0" | - | - | - | - | - | - | - | Deck Overhang | |
| AS601 | 160 | 15'-0" | Approach Slab | CR504ss | 4 | 1'-10" | Nose | | | | | | | | | | | | | | | | |
| | | | | CR505ss | 4 | 1'-8" | Nose | PIER - PLAIN BAR | | | | | | | | | | | | | | | |
| | | | | CR506ss | 4 | 1'-6" | Nose | P551 | 162 | 4'-2" | S | 0'-0" | 0'-7" | 3'-0" | 0'-7" | - | - | 0'-0" | - | - | - | Footing StIRRUP | |
| | | | | CR512ss | 4 | 2'-11" | Nose | P552 | 2 | 14'-4" | PA | 4'-0" | 3'-2" | 4'-0" | - | - | - | - | 2'-3" | 4'-6" | - | Upstream Pier Nose StIRRUP | |
| | | | | CR513ss | 4 | 2'-9" | Nose | P553 | 3 | 13'-10" | PA | 4'-0" | 2'-11" | 4'-0" | - | - | - | - | 2'-1" | 4'-3" | - | Upstream Pier Nose StIRRUP | |
| | | | | CR514ss | 4 | 2'-7" | Nose | P554 | 3 | 13'-6" | PA | 4'-0" | 2'-9" | 4'-0" | - | - | - | - | 2'-0" | 4'-0" | - | Upstream Pier Nose StIRRUP | |
| | | | | CR515ss | 4 | 2'-5" | Nose | P555 | 3 | 13'-2" | PA | 4'-0" | 2'-7" | 4'-0" | - | - | - | - | 1'-10" | 3'-9" | - | Upstream Pier Nose StIRRUP | |
| | | | | CR516ss | 4 | 2'-3" | Nose | P556 | 3 | 12'-10" | PA | 4'-0" | 2'-5" | 4'-0" | - | - | - | - | 1'-9" | 3'-6" | - | Upstream Pier Nose StIRRUP | |
| | | | | | | | | P557 | 3 | 12'-4" | PA | 4'-0" | 2'-2" | 4'-0" | - | - | - | - | 1'-7" | 3'-3" | - | Upstream Pier Nose StIRRUP | |
| | | | | CR550ss | 303 | 3'-9" | Rail & Post | P558 | 2 | 12'-2" | PA | 4'-0" | 2'-1" | 4'-0" | - | - | - | - | 1'-6" | 3'-0" | - | Upstream Pier Nose StIRRUP | |
| | | | | CR551ss | 302 | 7'-1" | Rail & Post | P559 | 2 | 11'-8" | PA | 4'-0" | 1'-10" | 4'-0" | - | - | - | - | 1'-4" | 2'-9" | - | Upstream Pier Nose StIRRUP | |
| | | | | CR560ss | 303 | 4'-0" | Rail & Post | P560 | 2 | 11'-6" | PA | 4'-0" | 1'-9" | 4'-0" | - | - | - | - | 1'-3" | 2'-6" | - | Upstream Pier Nose StIRRUP | |
| | | | | CR561ss | 302 | 8'-9" | Rail & Post | P561 | 3 | 12'-6" | S | 0'-0" | 4'-6" | 4'-0" | - | - | - | 0'-0" | - | - | - | Downstream Pier Nose StIRRUP | |
| | | | | | | | | P562 | 3 | 12'-3" | S | 0'-0" | 4'-0" | 4'-3" | 4'-0" | - | - | 0'-0" | - | - | - | Downstream Pier Nose StIRRUP | |
| | | | | CR750ss | 8 | 5'-4" | Nose | P563 | 3 | 12'-0" | S | 0'-0" | 4'-0" | 4'-0" | 4'-0" | - | - | 0'-0" | - | - | - | Downstream Pier Nose StIRRUP | |
| | | | | | | | | P564 | 3 | 11'-9" | S | 0'-0" | 4'-0" | 3'-9" | 4'-0" | - | - | 0'-0" | - | - | - | Downstream Pier Nose StIRRUP | |
| | | | | | | | | P565 | 3 | 11'-6" | S | 0'-0" | 4'-0" | 3'-6" | 4'-0" | - | - | 0'-0" | - | - | - | Downstream Pier Nose StIRRUP | |
| | | | | | | | | P566 | 3 | 11'-3" | S | 0'-0" | 4'-0" | 3'-3" | 4'-0" | - | - | 0'-0" | - | - | - | Downstream Pier Nose StIRRUP | |
| | | | | | | | | P567 | 2 | 11'-0" | S | 0'-0" | 4'-0" | 3'-0" | 4'-0" | - | - | 0'-0" | - | - | - | Downstream Pier Nose StIRRUP | |
| | | | | | | | | P568 | 2 | 10'-9" | S | 0'-0" | 4'-0" | 2'-9" | 4'-0" | - | - | 0'-0" | - | - | - | Downstream Pier Nose StIRRUP | |
| | | | | | | | | P569 | 2 | 10'-6" | S | 0'-0" | 4'-0" | 2'-6" | 4'-0" | - | - | 0'-0" | - | - | - | Downstream Pier Nose StIRRUP | |
| | | | | | | | | P570 | 220 | 5'-11" | T9 | - | 0'-10" | 4'-6" | 0'-7" | - | - | - | - | - | - | Stem StIRRUP | |
| | | | | | | | | P571 | 215 | 5'-8" | T9 | - | 0'-10" | 4'-3" | 0'-7" | - | - | - | | | | | |

Date:6/29/2021

Username: LindoT

Division: BRIDGE

Filename: ... \Bridge\MS1A\074_DetourPlan.dgn



NOTES:

1. The intent of this drawing is to show a conceptual layout for the placement of the temporary detour features. Actual location for placement of the various features shall be determined by the Contractor and be in conformance with MaineDOT requirements.
2. Temporary barrels shall consolidate access during construction by facilitating an approximate 25' wide opening adjacent to the post office property.



STATE OF MAINE
DEPARTMENT OF TRANSPORTATION

2229600

BRIDGE NO. 2273

WIN
22296.00

BRIDGE PLANS

FARMINGTON FALLS BRIDGE
SANDY RIVER
CHESTERVILLE-FARMINGTON FRANKLIN COUNTY

DETOUR PLAN

| PROJ. MANAGER | MICHAEL WRIGHT | BY | DATE | SIGNATURE | P.E. NUMBER | DATE |
|------------------|----------------|-----------|--------|-----------|-------------|------|
| DESIGN-DETAILED | MYLENE | R. PARKER | 6/2021 | | | |
| CHECKED-REVIEWED | C. SICHAK | C. SICHAK | 6/2021 | | | |
| DESIGNS-DETAILED | | | | | | |
| REVISIONS 1 | | | | | | |
| REVISIONS 2 | | | | | | |
| REVISIONS 3 | | | | | | |
| REVISIONS 4 | | | | | | |
| FIELD CHANGES | | | | | | |

SHEET NUMBER

74

OF 76

Filename: ...\\00\\ROW\\MSTA001_RWP\\PLAN1.dgn Division: ROW Username: Perry, Silverman Date: 6/30/2021

